

A partnership of



Indiana Mineral  
Aggregates Association

# TRAINING MANUAL

For C.A.P.P. Technicians

REVISED NOV. 2025

Prepared for the  
CERTIFIED AGGREGATE  
PRODUCER PROGRAM



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# **CERTIFIED AGGREGATE TECHNICIAN PROGRAM**



## **PROCEDURES and POLICIES**

### **MANUAL**

**UPDATED  
MAY, 2019**

## **INDOT CERTIFIED AGGREGATE TECHNICIAN PROGRAM**

### **Objectives**

The Indiana Department of Transportation (INDOT) has established a Quality Control/Quality Assurance (QC/QA) program for aggregates for the purpose of properly assigning the responsibility of manufacturing and overall improving the consistency of aggregates. The QC/QA program for aggregates requires that all aggregates supplied for INDOT use be supplied by a Certified Aggregate Producer. The Certified Aggregate Producer Program (CAPP) is a program whereby a qualified mineral aggregate Producer requesting to supply material to INDOT assumes all of the Plant site controls, and INDOT monitors the Producers production, sampling, and testing procedures. The CAPP requires that a Certified Aggregate Technician supervise all sampling and testing for process control.

The principal objective of the Certified Aggregate Technician Program is to provide the necessary training to Producer personnel so that they may administer the Quality Control requirements of the CAPP. Knowledge of aggregate production, materials, sampling, testing, statistics, and documentation are provided.

### **Administration**

The training program is administered by INDOT and the Indiana Mineral Aggregates Association (IMAA). Administration duties of include:

1. Writing and Maintenance of the Training Manual
2. Proctoring Examination
3. Notification to Students of Examination Results
4. Mailing Certificates
5. Maintenance of Certified Aggregate Technician List
6. Retesting
7. Recertification
8. Course Announcement
9. Student Registration
10. Manual Printing
11. Training Facility Arrangements
12. Meal and Refreshment Arrangements
13. Providing Training Course Materials
14. Certificate Preparation
15. Miscellaneous Administrative Tasks
16. Grading the Examination



## **Program Committee**

The Program Committee acts as the steering committee which establishes the needs for the certification program and provides technical assistance for course materials and examinations. The committee is composed of representatives from INDOT, FHWA, IMAA.

## **Certification Committee**

The Certification Committee is responsible for revocation or suspension of certifications for technicians. Their tasks include reviewing the violations of standard policies, rendering judgement of the seriousness of the violation, and hearing any subsequent appeal. The committee is composed of the following members:

State Materials Engineer, INDOT

1 Aggregate Producer representative appointed by the IMAA Technical Committee

## **Certification Requirements**

A technician is required to pass a written examination to become certified. Participation in the certification training course is required for the technician to take the examination.

## **Training Course Announcement**

The announcement for the training course will be made on August 1 of each year that the course is offered.

## **Certification Examination**

The certification examination is given upon completion of the training course. The examination time is limited to a maximum duration of four hours and the examination is open book/open note. There are two parts of the examination. Part I consists of multiple choice, and fill in the blank questions, and Part II consists of word problems. A minimum score of 70 percent is required on each part to pass the examination.

A technician that has failed the certification examination will be allowed one retake of the exam. Only the part(s) failed are required to be retaken. A duration of 1 ½ hours for Part I and 2 ½ hours for Part II are allowed. The retake examination will be open book/open note and consist of a format similar to the original examination. The retake examination will be given at the INDOT Division of Materials & Tests within 30 days of notification of the technician's results of the original examination. A minimum score of 70 percent on each part is required to pass the retake examination. Technicians failing either part of the retake examination will be required to participate in the training course and pass the examination to become certified.

The examinations will be retained by INDOT for a period of one year after such time the examinations will be destroyed. Technicians may review their examinations in the presence of an INDOT representative within one year of the examination date. Arrangements for review of the examination shall be made with INDOT.

### **Recertification Requirements**

Certification for all Technicians expires annually on July 1. Recertification will extend the certification to July 1 of the following year. Recertification can be accomplished by completing one of the following:

- Attend a Regional Spring Meeting
- Watch a video recording of the meeting and pass a quiz

The Regional Spring Meetings are held annually as a joint effort of INDOT and IMAA, and provide updates to CAPP and aggregate related topics. Meetings are held in the northern, central, and southern portions of the state. Information on date and location will be distributed in advance of the meetings, as well as posted on the IMAA website.

Initial certification is good for a year and a half from the date of initial issuance.

Renewal of the certification may be made within the subsequent 6 months after expiration by watching the video recording of the regional meeting and passing the associated quiz.

If a technician requests to be recertified after six months but before 18 months of certification expiration, renewal of the certification may be made by passing both parts of the certification exam, as well as watching the video recording of the regional meeting and passing the associated quiz.

If a technician requests to be recertified after 18 months of certification expiration, the Certification Committee will review the request and render a decision on the certification requirements.

The certified technician will be notified of the recertification procedures prior to the expiration of the certification. The technician is responsible for completing the steps for certification renewal. A current email address is required to be on file with INDOT. Address revisions are required to be sent to:

Melissa Ehrhart  
Statewide Geologist  
Indiana Department of Transportation  
Office of Materials Management  
120 S. Shortridge Rd.  
Indianapolis, IN 46219  
MEhrhart@indot.in.gov  
317-522-9666

## **Technician List**

The Department will maintain a list of certified technicians on the Department website.

## **Fees**

The fee for attending the certification training course will be established by the Program Committee. The fee will cover a training manual, course materials, refreshments, and several meals.

The refund policy for the certification course fees is as follows:

1. An administration fee of \$100 will be charged for cancellation by the technician within 7 days of the course.
2. Lack of attendance of the course will result in no refund of fees.
3. Unforeseen emergencies that result in absences during the course will result in a refund of the course fee.

There is no fee to attend the Regional Spring Meeting, or to watch the video and take the quiz prior to July 1 of each year. If a technician lets their certification expire, and wishes to become recertified within six months of expiration, a \$75 fee will be charged to take the quiz. If a technician lets their certification lapse, and wishes to become recertified after six months, a \$250 fee will be charged to take the examination.

Failure to pay the training course or examination fees will result in suspension of the certification.

## **Cancellation Policy**

If a scheduled certification is cancelled because of insufficient class size, the technicians will be notified one week prior to the start of the course. The technicians will be reimbursed the course fee.

## **Revocation or Suspension of Certification**

Certifications awarded may be revoked or suspended at any time by the Certification Committee for just cause. The procedure that will be taken to revoke or suspend a technician's certification is as follows:

1. The technician will be sent written notification of the revocation or suspension of certification by a registered letter. A copy of the written notification will be sent to the technician's employer. The letter will state the grounds for the revocation or suspension, request a written response, and establish a hearing date.
2. The technician will be allowed 60 days from the date of the notification to respond by letter. The response shall include an explanation of why the technician disagrees with the decision to revoke or suspend the certification.
3. After the 60 day time period has elapsed or upon receipt of the response, the case will be reviewed by the Certification Committee on the hearing date. The technician's response letter will be considered and the technician may appear before the Certification Committee.
4. The Certification Committee will issue a decision within one week of the hearing.
5. If the technician does not send a response letter, or fails to appear before the Certification Committee, a default judgement will be issued by the Certification Committee based on the evidence available. The revocation or suspension may be affirmed, modified, or vacated following the hearing.

The reasons that a technician's certification may be revoked or suspended include:

1. Cheating on recertification examinations
2. Falsification of quality control test results and/or records

The Certification Committee may decide to revoke or suspend the certification depending upon the seriousness of the violation. Violations deemed as unintentional will result in a penalty of a letter of reprimand to the technician and the technician's employer. Subsequent violations will result in suspension of certification for a designated period as determined by the Certification Committee. The certification will return to good standing after the period of suspension expires.

Intentional violations will result in a one year suspension of the certification. Subsequent violations will result in permanent revocation of the certification. If the technician wishes to become recertified after the period of suspension, the technician will be required to participate in the certification training course and pass the certification examination.



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# CERTIFIED AGGREGATE PRODUCER PROGRAM C.A.P.P.

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Quality Control/Quality Assurance (QC/QA) is often used synonymously with the term Quality Assurance (QA). The American Association of State Highway Traffic Officials (AASHTO) defines Quality Assurance as “All those planned and systematic actions necessary to provide confidence that a product will perform satisfactorily in service.” This definition considers QA to be an all-encompassing concept which includes quality control (QC), acceptance, and independent assurance (IA). A Better understanding of the QC/QA concept may be made if the characteristics of the specifications are considered. These include:

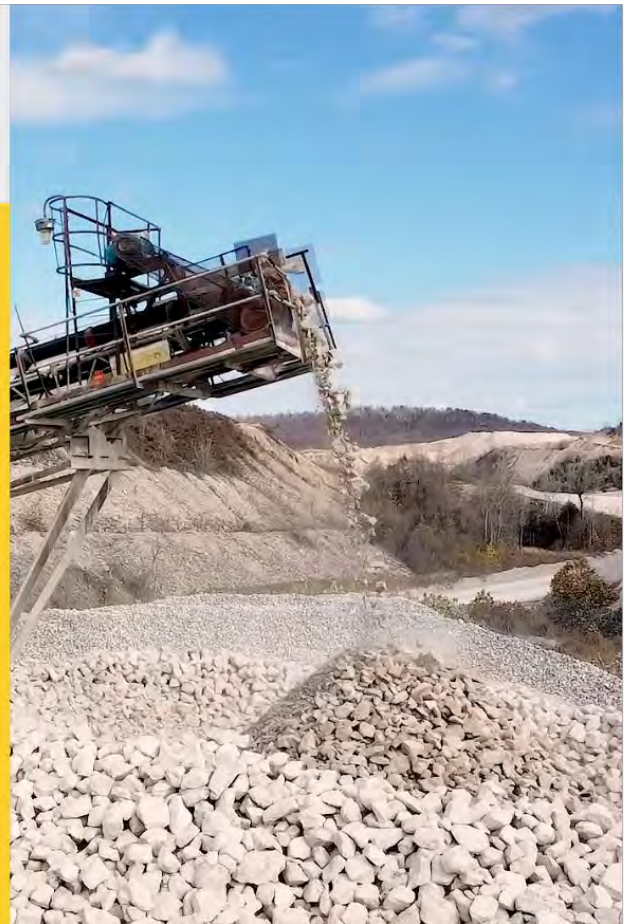
1. QC/QA recognizes the variation in materials and test methods.
2. QC/QA uses a statistical basis that is applied and modified with experience and sound engineering judgement.
3. QC/QA places primary responsibility on the Producer for the production control.

The procedure used by INDOT in the past to accept aggregates required that a stockpile of aggregates be tested to verify compliance with specifications, and the stockpile subsequently approved or disapproved prior to shipment. The pass/fail specification became very confrontational with Producers when failing tests were obtained and shipments delayed or stopped to active contracts. Even when eventually resolved, project delays were inevitable in many cases. A QC/QA procedure whereby Producer’s tests could be used for acceptance and shipments of aggregates made on demand was needed. The Certified Aggregate Producer Program (CAPP) was introduced as the procedure to accomplish both needs.

The CAPP designates specific quantities of material to be tested, material test values, test equipment calibrations, and statistical concepts to be applied to control aggregate products. As such, a standard method for rounding values is required to be established and basic statistical rules be presented. The chapter discusses the procedures for rounding numbers, and the basic statistical calculations.

# AGGREGATES IN INDIANA

## CHAPTER 1





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# AGGREGATES IN INDIANA

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## **INTRODUCTION OF AGGREGATES**

What is an Aggregate?

Uses of Aggregate

## **ORIGIN OF AGGREGATES**

Natural Sands

Gravel

Crushed Stone

Slag

## **DISTRIBUTION OF AGGREGATES**

Glacial Deposits

Bedrock Deposits

## **AGGREGATE TYPES**

Natural Aggregates

Artificial Aggregates

## **CLASSIFICATIONS OF AGGREGATES**

Fine Aggregates

Coarse Aggregates

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# INTRODUCTION OF AGGREGATES

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Aggregates play an important role in highway construction. Without aggregates, concrete bridges and structures, Hot Mix Asphalt (HMA) pavements and concrete pavements could not be constructed and very few roads could sustain the current loads. The use of aggregates in highway construction has literally brought the transportation industry out of the mud.

Aggregates are not used indiscriminately in construction, as not all aggregates are appropriate for every application. Some aggregates do not have the correct chemical or physical properties or the correct size or shape for the job. This chapter includes the requirements that aggregates are required to meet, the tests of INDOT, and the documentation that is required to be completed for the test results.

## **WHAT IS AN AGGREGATE?**

Aggregates are granular materials that have been mined for use as a building material in the construction industry. They include aggregates from natural and artificial origins, such as stone, sand and gravel, and slag.

## **USES OF AGGREGATES**

Streets, bridges, roads and sidewalks are made of concrete or asphalt, which is mostly made up of aggregates combined with a binder that acts like glue. Concrete is used in foundations and basements for houses. Other buildings sometimes use concrete throughout their structures.

But did you know that many items you use every day have aggregates in them? Your toothpaste has aggregates in it. Glass is made from sand, which is an aggregate. Minerals and aggregates are in plates, dishes, pots and pans, baby powder, household cleaners, makeup, medicines, paints, pencils, fertilizers, wallboard, and more including some of the foods you eat! How many times have you heard that your cereal, for instance, is fortified with vitamins and minerals?

Any object that hasn't been grown had to be mined! Aggregates, minerals and metals all come from the ground. You can look at objects all around you and know which were grown and which were mined. If it's metal, it was mined. If it is wood, it was grown. If it is cotton or wool, it was

grown. Paper comes from trees, which are grown. Pencils are grown and mined because wood is grown, and graphite is mined. The ink on this paper came from mining. But what about plastic? Is it grown or mined? Plastic comes from petroleum products, which are mined.

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# ORIGIN OF AGGREGATES

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The four main types of mineral aggregates in Indiana are gravel, natural sand, crushed stone, and slag. All have different origins.

## **NATURAL SANDS AND GRAVEL**

### Natural Sand

Natural sand is a mineral particle or rock fragment that is larger than coarse silt but smaller than gravel. It is a widespread but highly variable resource in Indiana that was formed mostly by glacial actions of large ice sheets and then sorted by running water. Sand can be used as fill; or more often the coarser parts find use as components of concrete or asphalt pavement. Sand, a finer granular material, is important in concrete and in making mortar and in snow and ice control. Very fine-grained sand finds use in foundries to make molds. It is also used in glass-making, or even as golf-course sand. The size of sand ranges from .0625 mm (.002 in) to less than 2mm (.08 in).

### Gravel

The term gravel applies to a range of particle sizes, rather than a specific rock or mineral type. Gravel is colored by the rock types present. It is a collection of rock particles that are at least .08 inches in diameter sizes but may also include boulders over 10 inches in diameter. Gravel is loose rock that is often rounded in shape from being worn by water at some point. Gravel can be used alone as fill for gravel roads or residential driveways. Gravel can also be used as a component of concrete or asphalt pavement. Gravel is greater than or equal to 2mm (.08 inch) in size. It can include larger rocks such as boulders (over 10 inches).

Most of the gravels and natural sands used today are products of the Ice Ages (glaciation). Geologists concur that glaciers may have been up to 1 mile thick. As the glaciers advanced southward, rock was scraped beneath them. When the glaciers melted, the flowing water

carried the rock fragments and deposited them downstream. The scraping action of the ice and flowing waters gave the gravels and natural sands their rounded appearance.

In addition, minor amounts of gravels and sands are obtained from postglacial or modern stream deposits. This operation is called fluvial and is largely restricted to the river bars, bottom lands, and flood plains of the Ohio River and the lower reaches of the White and Wabash Rivers.

Gravels and sands are unconsolidated granular materials resulting from the natural disintegration of rocks. They disintegrated primarily from the abrading action of water or ice on rock material. Therefore, in Indiana, deposits are likely to be found in stream bottoms, in terraces adjacent to streams, and in outwash plains, all of which are areas beyond the physical limits of the original glaciers.

## **CRUSHED STONE**

Crushed stone produced within Indiana originates from sedimentary bedrock deposits. There are three general classes of rocks: igneous, sedimentary, and metamorphic. Igneous rocks were formed from hot volcanic magma--molten mineral material. Sedimentary rocks were formed from the disintegration of other rocks and organic materials. Metamorphic rocks were originally igneous or sedimentary rocks but were changed by pressure and/or heat. Across the United States, variations of the above noted rock types are utilized for crushed stone aggregate. Sedimentary rock types, limestone and dolostone, are primarily used as construction aggregates within Indiana. Sandstone from southern Illinois, a sedimentary rock type, is permitted for use in hot mix asphalt surface courses.

## **SLAG**

Slag is a man-made or synthetic aggregate recycled from the steel-making process. It is a dark-colored and usually rough-textured aggregate. There are four main types of slag used as construction aggregate in the state of Indiana:

1. Blast Furnace Slag -- a non-metallic material removed in the molten state of iron production. The further refinement of this blast furnace slag results in three aggregate variations: air-cooled slag, expanded slag, and granulated slag.
2. Steel furnace slag -- a material derived from the further refinement of iron to steel.
3. Wet-bottom boiler slag -- a material which is a by-product from coal combustion at electrical generating plants. A secondary product created at these power plants is a residue in the flue gases known as fly ash.

4. Lightweight aggregate -- a material which is created as a by-product of the manufacturing process of construction brick. The primary constituent is shale, sedimentary rock.

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# DISTRIBUTION OF AGGREGATES

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All types of aggregates are not found in every area of Indiana. The composition of each type of aggregate also varies.

## GLACIAL DEPOSITS



**Figure 1.1 Glacial Deposit Map**

Gravel and sand deposits are found along almost any river in Indiana, except the south-central part of the state. At one-time, glaciers covered five-sixths of Indiana. Figure 1.1 shows the southern boundaries of the two glaciers that moved into Indiana. The size of the gravel and the type of minerals and rocks found in the deposits varies from place to place. As shown in Figure 1.1, the size of the gravel, in general, tends to get smaller downstream within a drainageway. Statewide, the occurrence of gravel decreases from northeastern to southwestern Indiana.

The composition of a deposit also varies from place to place. In some deposits, 10 to 20 different types of rocks may be found. Granite, gneiss, and schist (igneous and metamorphic rocks) or limestone, dolostone, chert, sandstone, siltstone, and shale (sedimentary rocks) are typically found. Porous chert, siltstone,

sandstone, ocher, and shale are deleterious, meaning that the material does not perform well in certain applications in highway construction. The map in Figure 1.1 illustrates the distribution of deleterious materials around Indiana.

## BEDROCK DEPOSITS

As shown in the bedrock map of Indiana (Figure 1.2), the bedrock belongs to five geologic periods, which are listed from the oldest to youngest: Ordovician, Silurian, Devonian, Mississippian, and Pennsylvanian.

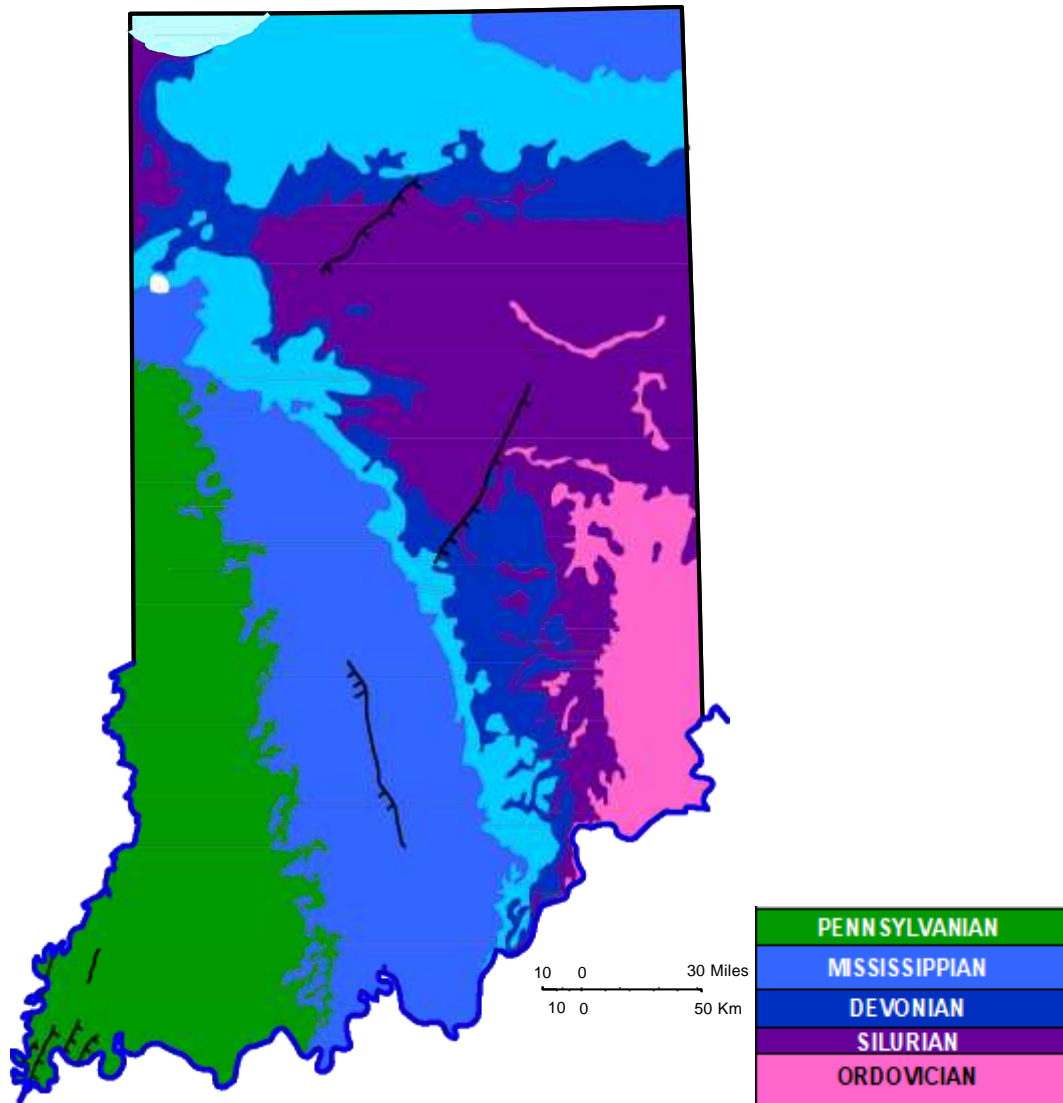


Figure 1.2 Bedrock Deposit Map

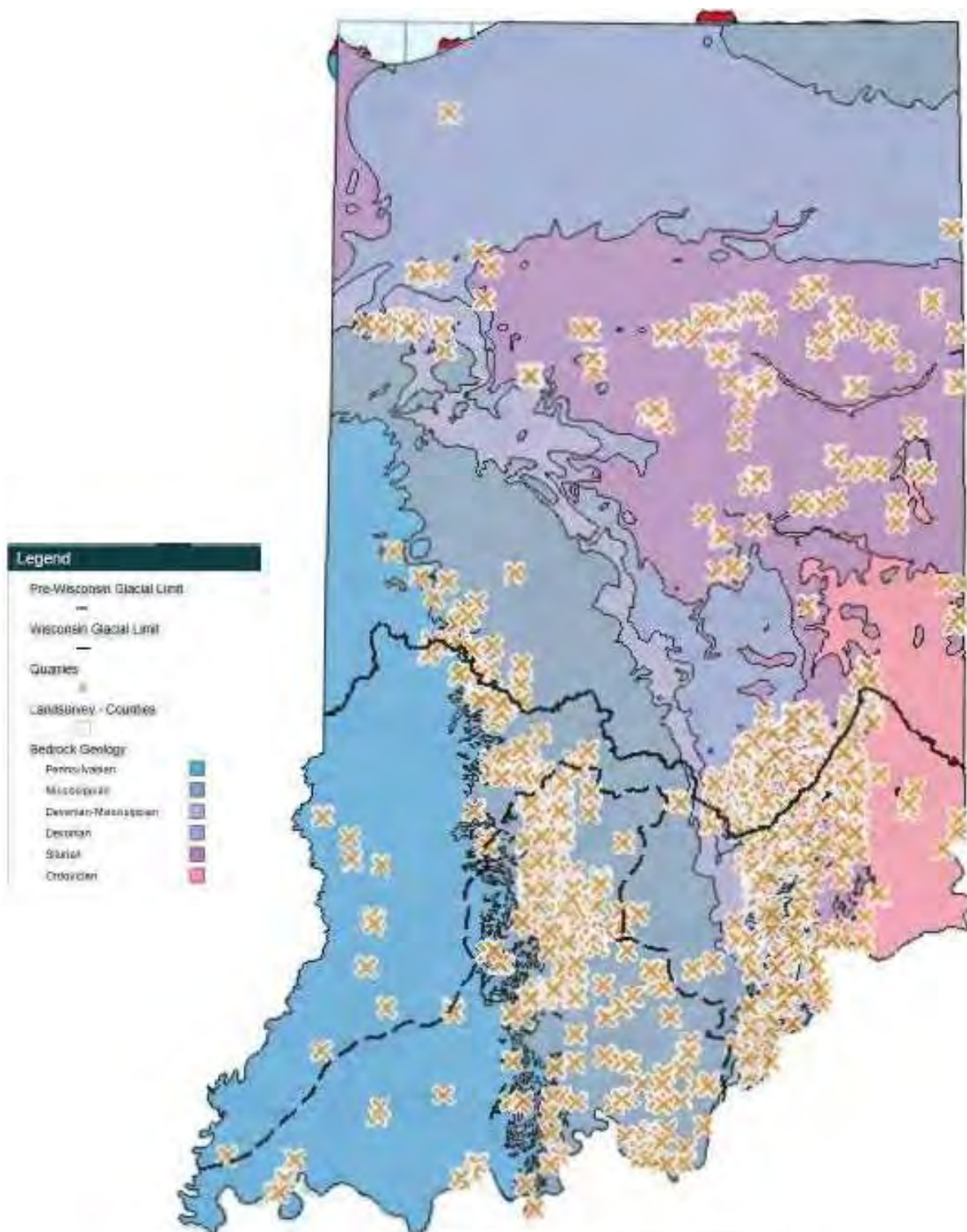
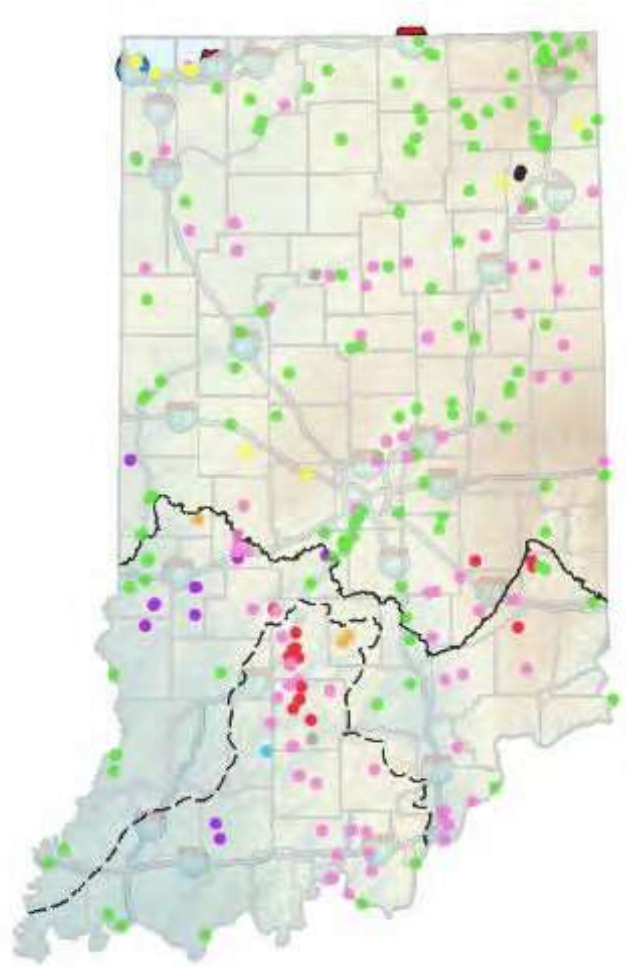


Figure 1.3 Quarry Distribution Map



Comparing the map of the quarry locations (Figure 1.3) to the bedrock map (Figure 1.2), almost all of Indiana's crushed stone quarries are in areas underlain by rock of Mississippian, Devonian, or Silurian Ages (Figure 1.2). During these periods, thick beds of high-grade limestone or dolostone were formed. Rock types formed during other geologic periods are either inaccessible or do not possess the minimum quality requirements needed for highway construction.

Since most of Indiana was once covered by glaciers, the deposits left by these glaciers have impacted the location of quarry sites in the state. Quarry sites are more easily developed in southern Indiana than in northern Indiana where the overburden may reach several hundred feet in depth. In the glaciated parts of Indiana, quarry sites are limited to areas where streams have eroded to bedrock or areas where bedrock was usually high in pre-glacial times, such as ancient coral reefs. Many quarries have been developed in areas where sand and gravel deposits were mined to the bedrock surface.



**Figure 1.4 Sand and Gravel Deposits**



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# AGGREGATE TYPES

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The aggregates used in highway construction are all mineral aggregates. Aggregates are composed of a naturally occurring solid chemical element or compound formed as a product of an inorganic process. There are two distinct types of aggregate: natural, and artificial.

## **NATURAL AGGREGATES**

Rock or stone (either term may be used) fragments which are used in their natural state are considered natural aggregates. Crushed stone, sand, and gravel are natural aggregates.

## **CRUSHED STONE**

Crushed stone is produced from quarries where the bedrock is blasted (shot) with explosives and further fragmented by mechanical crushing. All crushed stone fragments are angular in shape and all faces of the fragments are created by the crushing operation.

The most common sedimentary rock types found in Indiana are limestone, dolostone, sandstone, shale, and siltstone. Only limestone and dolostone are routinely used for highway construction, although some sandstone from southern Illinois is allowed for high-friction HMA surface.

## **SAND AND GRAVEL**

Sand and gravel are the result of the weathering and erosion of bedrock by natural forces. The two are generally found together, in pockets deposited by a stream or a glacier. These aggregates may be mined from a water-filled pit (a deposit below the water table) or from a cut-bank deposit (a deposit above the water table). If the aggregates come from a pit, the aggregate is referred to as "pit-run" material. A cut-bank deposit is termed "bank-run" material.

Sand from these deposits is often referred to as natural sand, while sand made by crushing stone, pieces of gravel, or slag are commonly called manufactured sand.

The sand and gravel found in the deposits have a variety of assorted sizes. Further processing is required including screening, washing, and some crushing. The crushing is done to produce aggregates of the proper size.

## **ARTIFICIAL AGGREGATES**

Artificial (synthetic) aggregates are manufactured aggregates or by-products of industrial processes. Of the artificial aggregates, INDOT most commonly uses the by-product aggregates. These aggregates are processed either from blast furnace slag, steel slag, or wet bottom boiler slag.

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# CLASSIFICATIONS OF AGGREGATES

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Aggregates are separated into two classifications: coarse aggregates, and fine aggregates. The No. 4 sieve generally determines the difference between coarse aggregate and fine aggregate for most highway construction work.

## **FINE AGGREGATE**

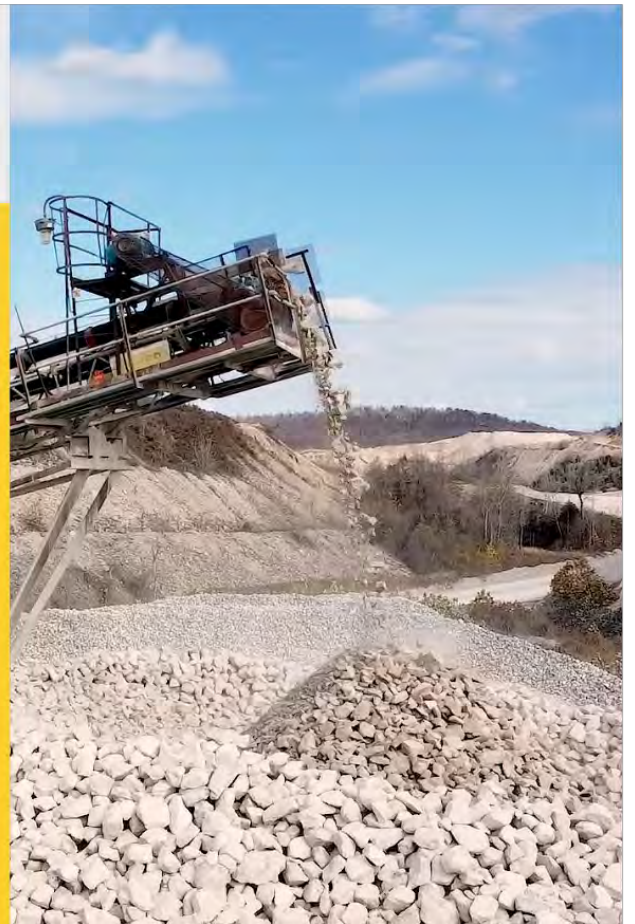
Fine aggregate is defined as aggregate that is 100 percent passing the 3/8 in. sieve and a minimum of 80 percent passing the No. 4 sieve. Natural sand and manufactured sand produced by crushing stone, steel furnace slag, air cooled blast furnace slag and wet-bottom slag are all fine aggregates.

## **COARSE AGGREGATE**

Coarse aggregate is defined as aggregate that has a minimum of 20 percent retained on the No. 4 sieve. Crushed stone, crushed or uncrushed gravel, and crushed blast-furnace and steel slag all fall within this category.

# AGGREGATE PROPERTIES

## CHAPTER 2



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# AGGREGATE PROPERTIES

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## **PHYSICAL PROPERTIES**

Absorption, Porosity, and Permeability

Surface Texture

Strength

Density and Specific Gravity

Soundness

Hardness

Particle Shape

## **CHEMICAL PROPERTIES**

Composition

Reactions with Asphalt and Cement Concrete

## **EFFECTS OF PHYSICAL AND CHEMICAL PROPERTIES ON AGGREGATE USE**

Compacted Aggregates

Aggregate for Hot Mix Asphalt

Aggregate for Portland Cement Concrete

## **UNDESIRABLE PROPERTIES**

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# AGGREGATE PROPERTIES

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The origin, distribution, and aggregate types found within Indiana were discussed in Chapter One. The intent of this chapter is to familiarize the personnel responsible for aggregate testing with:

1. Physical properties
2. Chemical properties
3. Effects of physical and chemical properties on aggregate use

Recognition of these properties and characteristics assists the Technician in evaluating the different aggregates used in highway construction.

Aggregate particles have certain physical and chemical properties which make the aggregate acceptable or unacceptable for specific uses and conditions. The following properties are of the greatest concern for road aggregate quality.

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## PHYSICAL PROPERTIES

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The physical properties of aggregates are those that refer to the physical structure of the particles that make up the aggregate.

### **ABSORPTION, POROSITY, AND PERMEABILITY**

The internal pore characteristics are very important properties of aggregates. The size, the number, and the continuity of the pores through an aggregate particle may affect the aggregate's strength, abrasion resistance, surface texture, specific gravity, bonding capabilities, and resistance to freezing and thawing action. Absorption relates to the particle's ability to take in a liquid. Porosity is a ratio of the volume of the pores to the total volume of the particle. Permeability refers to the particle's ability to allow liquids to pass through. If the rock pores are not connected, a rock may have high porosity and low permeability.

## **SURFACE TEXTURE**

Surface texture is the pattern and the relative roughness or smoothness of the aggregate particle. Surface texture plays a big role in developing the bond between an aggregate particle and a cementing material. A rough surface texture gives the cementing material something to grip, producing a stronger bond, and thus creating a stronger hot mix asphalt or portland cement concrete. Surface texture also affects the workability of hot mix asphalt, the asphalt requirements of hot mix asphalt, and the water requirements of portland cement concrete.

Some aggregates may initially have good surface texture but may polish smooth later under traffic. These aggregates are unacceptable for final wearing surfaces. Limestone usually falls into the unacceptable category. Dolomite generally is in the acceptable category when the magnesium content exceeds the minimum required in the INDOT Standard Specifications.

## **STRENGTH**

Strength is a measure of the ability of an aggregate particle to stand up to pulling or crushing forces. High strength is desirable in aggregate base and surface courses. These qualities minimize the rate of disintegration and maximize the stability of the compacted material. The best results for portland cement concrete may be obtained by compromising between high and low strength. This permits volumetric changes to take place more uniformly throughout the concrete.

## **DENSITY AND SPECIFIC GRAVITY**

Density is the weight per unit of volume of a substance. Specific gravity is the ratio of the density of the substance to the density of water. The following chart illustrates these relationships for some common substances.

Typical Specific Gravity Chart	
Substance	Specific Gravity
Wood (oak)	0.7
Water	1
Coal	1.6
Gypsum	2.3
Blast Furnace Slag	2.4
Limestone	2.6
Quartz	2.6
Shale	2.6
Gravel	2.7
Trap Rock	2.9
Steel Slag	3.5
Iron	7.9
Lead	11
Gold	19

**Figure 2.1 Typical Values**

The density and the specific gravity of an aggregate particle are dependent upon the density and specific gravity of the minerals making up the particle and upon the porosity of the particle. These may be defined as follows:

1. All of the pore space (bulk density or specific gravity)
2. Some of the pore space (effective density or specific gravity)
3. None of the pore space (apparent density or specific gravity)

Determining the porosity of aggregate is often necessary but difficult to achieve. In order to measure pore space, correlations may be made between porosity and the bulk apparent and effective specific gravities of the aggregate.

As an example, specific gravity information about a particular aggregate helps to determine the amount of asphalt needed in hot mix asphalt. If an aggregate is highly absorptive, the aggregate continues to absorb asphalt after initial mixing at the plant until the mix cools down completely. This process leaves less asphalt for bonding purposes; therefore, a more porous aggregate requires more asphalt than a less porous aggregate. The porosity of the aggregate may be taken

into consideration in determining the amount of asphalt required by applying the three types of specific gravity measurements.

## SOUNDNESS

Under real world applications aggregates are subjected to harsh conditions. Weather extremes create conditions conducive to aggregate breakdown. The soundness of an aggregate refers to the aggregate's inherent ability to withstand these extremes. This aggregate quality is verified through tests involving exposure to chemicals, such as sodium sulfate, and by exposure to rapid freezing and thawing cycles.

## HARDNESS

Hardness is a physical property of minerals that refers to the ease with which the mineral may be scratched by certain materials. Knowing the hardness of minerals contained within aggregates may prove useful for identification purposes. The Mohs Hardness Scale, shown in the table below, is the standard way to measure mineral hardness.

Limestone contains the mineral calcite, which is about a 3 on the hardness scale. Chert, a deleterious rock, is comprised of microcrystalline quartz, which is about a 7 on the hardness scale. A steel file or nail, or even a copper penny, may be used to differentiate between chert and limestone based on the known hardness of minerals contained in these rocks.



Mineral Name	Scale Number	Common Object
Diamond	10	
Corundum	9	Masonry Drill Bit (8.5)
Topaz	8	
Quartz	7	Steel Nail (6.5)
Orthoclase	6	
Apatite	5	Knife/Glass Plate (5.5)
Fluorite	4	
Calcite	3	Copper Penny (3.5)
Gypsum	2	
Talc	1	Fingernail (2.5)

**Figure 2.2 Mohs Hardness Scale**  
([www.nps.gov](http://www.nps.gov))



## PARTICLE SHAPE

Aggregates come in many shapes and sizes. For our purposes, aggregates are grouped into two broad categories, angular and rounded. Angular particles are newly broken. They have not been subjected to forces of nature such as stream polishing, weathering, and wind erosion. Crushed stone and crushed gravel are examples of products that contain angular particles. In stream sediments and glacial till in Indiana, rounded particles are common. Products such as natural sand and gravel contain rounded particles.

The shape of the aggregate particle can greatly affect workability and strength of both Portland cement concrete and asphalt pavements. Irregular angular particles in crushed stone and crushed gravel have a tendency to interlock and are therefore optimal for strength in concrete and asphalt mixtures. A percentage of natural sand is allowed to be added depending on the application. For requirements for crushed materials in certain applications, consult the current INDOT Standard Specifications book.

Another aspect to consider involving particle shape is the proportion of width to height. Particles which are flat and elongated have a tendency to be structurally weak. Generally, the optimal particle shape is a dice-like shape.

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# CHEMICAL PROPERTIES

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The chemical properties of aggregates have to do with the molecular structure of the minerals in the aggregate particles.

## COMPOSITION

The chemical composition of aggregate is significant in determining the difference between limestone and dolomite. Limestone is a rock consisting mainly or wholly of calcium carbonate and has a tendency to polish smooth under traffic. Therefore, when used in HMA surface mixtures, limestone is limited in use. Dolomite under traffic maintains a higher-friction, skid-resistant surface and is used on higher traffic volume HMA mixtures. The word dolomite is the mineral calcium magnesium carbonate  $\text{Ca Mg} (\text{CO}_3)_2$ . Technically, the term for aggregate that consists largely of calcium magnesium carbonate is dolostone. However, INDOT Standard Specs and

common use refers to the aggregate as dolomite. INDOT uses an elemental magnesium (Mg) content test to determine if a rock source is dolomitic. An elemental magnesium content of 10.3 percent or above is required for dolomite aggregates. Dolomite aggregates can cause errors in certain test methods when determining HMA mixture properties.

Some aggregates have minerals that are subject to oxidation, hydration, and carbonation. These properties are not particularly harmful, except when the aggregates are used in portland cement concrete. As might be expected, iron sulfides, ferric and ferrous oxides, free lime, and free magnesia in industrial products and wastes are some of the common substances. Any of these substances may cause distress in the portland cement concrete and give the concrete an unsightly appearance.

Some metallic minerals of the sulfite and hydroxide chemical families; particularly, pyrite ( $\text{FeS}_2$ ), marcasite ( $\text{FeS}_2$ ), hematite ( $\text{Fe}_2\text{O}_3$ ) and magnetite ( $\text{Fe}_3\text{O}_4$ ) may cause concerns. The latter three minerals, when they occur, can be traced to sand and gravel deposits. Pyrite, for the most part, is found in shales and limestones of crushed stone sources.

The primary effect of these minerals is a rust-like staining in exposed concrete and hot-mix asphalt pavements (HMA). Elevated percentages of pyrite may necessitate alternative test methods when determining HMA mixture properties.

## **REACTIONS WITH ASPHALT AND CEMENT CONCRETE**

There are several types of substances found in mineral aggregates that may have a negative effect on the cementing and overall performance qualities of asphalt and cement concrete. Most are rarely significant but various organic substances may retard hardening, reduce strength development, or cause excessive air entrainment in portland cement concrete. These organic substances include, but are not limited to, mica, iron oxide, lightweight chert, shale, coal, and lignite.

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# EFFECT OF PHYSICAL AND CHEMICAL PROPERTIES ON AGGREGATE USE

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Aggregates have three primary uses in highway construction:

1. As compacted aggregates in bases, subbases and shoulders
2. In hot mix asphalt
3. In portland cement concrete

Aggregates may also be used as structure backfill material, riprap, mineral filler, and other less significant uses.

## **COMPACTED AGGREGATES**

Compacted aggregates are aggregates that are spread and compacted, with optimum moisture, to Department specifications meeting either No. 2, 5, 8, 53, or 73 gradations as outlined in Standard Specifications Section 300. Compacted aggregates without the addition of a cementing material may be used as a base or subbase for hot mix asphalt and portland cement concrete pavements. Portland cement concrete pavements are rigid pavements. For these types of pavements, the purpose of the base may be to improve drainage, to prevent pumping, or to cover a material that is highly susceptible to frost. Consequently, gradation and soundness are the primary considerations in selecting or evaluating aggregates for bases under rigid pavements.

The load carrying capacity is a primary factor in the selection of aggregates for hot mix asphalt pavements. A hot mix asphalt pavement does not carry the load; help from the underlying base courses is required. In addition to gradation requirements, the aggregates are required to also possess the strength to carry and transmit the applied loads.

Aggregates are sometimes used to make up the entire pavement structure. In this type of pavement, aggregates are placed on the natural soil to serve as a base course and surface course. Again, the primary requirement is the gradation.

In many instances, compacted aggregates are also used to construct roadway shoulders and berms. In these applications, gradation and stability are very important. Granular materials used in building and construction; include natural and artificial aggregates.

## **AGGREGATE FOR HOT MIX ASPHALT**

INDOT uses hot mix asphalt in a number of different ways. In all cases the aggregates used should meet five requirements:

1. Strong, tough and durable
2. The ability to be crushed into bulky particles, without many flaky particles, slivers or pieces that are thin and elongated
3. Low porosity
4. Low permeability
5. Correct particle size and gradation for the type of pavement

## **AGGREGATES FOR PORTLAND CEMENT CONCRETE**

There are many uses of portland cement concrete in highway construction. Some of the major uses of aggregates are in rigid-pavement slabs, bridges, concrete barriers, sidewalks, curbs, slope walls, and other structures.

Aggregates in portland cement concrete are required to always be physically and chemically stable. Other factors to be considered include:

1. The size, distribution, and interconnection of voids within individual particles
2. The surface character and texture of the particles
3. The gradation of the coarse and fine aggregates
4. The mineral composition of the particles
5. The particle shapes
6. Soundness abrasion resistance
7. Water absorption

## **OTHER AGGREGATES**

There are other uses for aggregates in highway construction. The requirements are somewhat different from the ones already discussed. However, in most cases gradation as a controlling factor is common to all applications.

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# UNDESIRABLE PROPERTIES

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Particles with undesirable physical characteristics include but are not limited to the following:

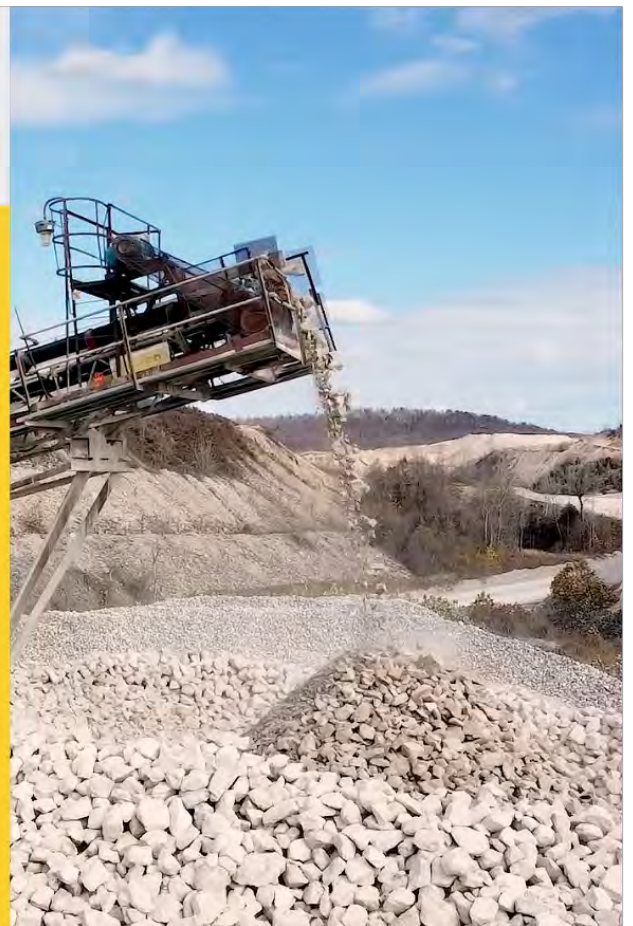
1. Non-durable soft or structurally weak particles
2. Clay lumps or clay balls
3. Flat or elongated particles
4. Organic matter contaminants
5. Lightweight chert

Some metallic minerals of the sulfite and hydroxide chemical families; particularly, pyrite ( $\text{FeS}_2$ ), marcasite ( $\text{FeS}_2$ ), hematite ( $\text{Fe}_2\text{O}_3$ ) and magnetite ( $\text{Fe}_3\text{O}_4$ ) may cause concerns. The latter three minerals, when they occur, can be traced to sand and gravel deposits. Pyrite, for the most part, is found in shales and limestones of crushed stone sources.

The primary effect of these minerals is a rust-like staining in exposed concrete and hot-mix asphalt pavements (HMA). Elevated percentages of pyrite may necessitate alternative tests methods when determining HMA mixture properties.

# AGGREGATE SPECIFICATIONS & RECOMMENDATIONS

## CHAPTER 3



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# AGGREGATE SPECIFICATIONS AND REQUIREMENTS

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## **AGGREGATE QUALITY CLASSIFICATION**

- Fine Aggregates
- Coarse Aggregates

## **PHYSICAL QUALITY TESTS**

- Absorption
- Abrasion Resistance
- Soundness
- Deleterious Materials
- Special Requirements

## **GENERAL USAGE REQUIREMENTS**

- Fine Aggregates
- Coarse Aggregates

## **GRADATION REQUIREMENTS**

- Fine Aggregates
- Coarse Aggregates
- B Borrow and Structure Backfill
- Riprap
- Aggregate Base
- Subbase
- Aggregate Pavements or Shoulders
- Summary of Gradation Requirements

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# AGGREGATE SPECIFICATIONS AND REQUIREMENTS

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The specifications for aggregates are detailed in Section 904 and other sections for the various types of construction located in the INDOT Standard Specifications manual. These specifications are to be followed when inspecting aggregates. There are two general types of requirements for aggregate: quality and gradation.

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## AGGREGATE QUALITY CLASSIFICATION

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Physical quality requirements are all specification provisions other than gradation or usage requirements. These requirements may be divided into five distinct groups as follow:

1. Bulk specific gravity and absorption
2. Abrasion resistance (LA Abrasion)
3. Soundness
4. Restriction on deleterious constituents
5. Special requirements

### **FINE AGGREGATES**

Section 904.02 defines the acceptable limits for all uses of fine aggregates. Fine aggregates are not divided into classes.



## COARSE AGGREGATES

Section 904.03 defines the acceptable limits for all uses of coarse aggregates.

Coarse aggregates are divided into classes based on quality requirements as noted in the Classification of Aggregates table. Class AP is the highest class and is assigned to aggregates which meet the requirements for all INDOT uses. Some INDOT contracts specify type AP aggregates for use in specific applications of portland cement concrete. Parameters concerning type AP aggregate are contained in ITM 210. Aggregates having restricted approval are rated Classes A, AS, B, C, D, E, and F.

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# PHYSICAL QUALITY TESTS

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Approval and use of aggregates is based upon meeting test requirements in the following physical tests.

## BULK SPECIFIC GRAVITY AND ABSORPTION

The absorption quality requirement applies only to coarse aggregates, but this data is necessary on fine aggregate for other purposes, such as mix design and water/cementitious ratios.

All aggregates are porous, but some are more porous than others. How porous an aggregate is determines how much liquid may be absorbed when soaked in water. AASHTO T 84/85 defines absorption as the increase in the weight of aggregate because of water in the pores of the material, but not including water adhering to the outside surface of the particles. Absorption is expressed as a percentage of the dry weight.

Absorption requirements are of concern only regarding aggregates used in hot mix asphalt and portland cement concrete. The intent is to avoid using highly porous, absorptive aggregates because extra water and cement or asphalt is needed to make a good mix. However, some aggregates, such as blast furnace slag, may be used despite their high absorptive capacity because of other characteristics that make them desirable, including skid resistance, economics, etc.

## **ABRASION RESISTANCE (LA ABRASION)**

### **(AASHTO T 96)**

Abrasion resistance applies only to coarse aggregates. Aggregates vary in their resistance to fracturing under impact (toughness) and breaking down into smaller pieces from abrasive action (hardness). The acceptable limits are set by the Los Angeles (LA) Abrasion Test AASHTO T 96. The limits vary from 30.0 to 50.0 percent, depending on the classification of the aggregate. The percentage is a measure of the degradation or loss of material as a result of impact and abrasive actions. Section 904.03 details the requirements. Abrasion requirements do not apply to blast furnace slag.

## **SOUNDNESS**

### **(AASHTO T 103/T 104, ITM 209)**

The quality of soundness applies to both fine and coarse aggregates. The durability of aggregates or their resistance to the forces of weathering is one of the most important considerations in the selection of a material for highway construction. Alternate freezing and thawing of the aggregates is the major concern.

INDOT uses three different test methods to evaluate soundness:

1. The water freeze and thaw test in accordance with AASHTO T 103, Procedure A
2. The sodium sulfate test in accordance with AASHTO T 104
3. The brine freeze and thaw test in accordance with ITM 209

The water freeze and thaw test requires the aggregate to be sealed and totally immersed in water and then be subjected to 50 cycles of freeze and thaw. The sodium sulfate test requires the aggregate to be immersed in a sodium sulfate solution and then be subjected to 5 cycles of alternate immersion and drying. The brine freeze and thaw test requires the aggregate to be enclosed in a bag containing a 3 percent sodium chloride solution and then be subjected to 25 cycles of freeze and thaw.

The freezing and thawing in water test is the method that most accurately simulates actual field conditions; however, the test requires a long period of time to conduct. The "quick" checks for soundness of the aggregate are the brine freeze and thaw and sodium sulfate tests. If the aggregate fails either the brine freeze and thaw or the sodium sulfate test, the aggregate is tested

using the freeze and thaw in water method. An aggregate that passes the freeze and thaw in water test is an acceptable material for use on INDOT contracts.

## **DELETERIOUS MATERIALS**

Certain substances in aggregates are undesirable for use in portland cement concrete. Therefore, the Specifications limit the amount of deleterious constituents to a level consistent with the quality sought in the final product.

### **FINE AGGREGATE**

(AASHTO T 21 and T 71)

Organic impurities are the only concern in fine aggregates. Section 904.02 places a restriction for fine aggregate for use in portland cement concrete and mortar. No restrictions are placed on organic impurities in fine aggregate for use in other types of construction.

The limitations on the amount of organic impurities allowed in fine aggregates are determined by the test method for organic impurities in AASHTO T 21 and the test method for Mortar Strength in AASHTO T 71. According to the Specifications, materials failing the organic impurities test are to be tested for the effect of organic impurities using the mortar strength test. The results of the test are the basis for acceptance or rejection of the fine aggregate.

### **COARSE AGGREGATE**

Section 904.03 includes a general statement regarding deleterious substances that applies to all classes of coarse aggregates. Section 904.03 also details more specific restrictions for other harmful substances as a maximum allowable percentage of the mass of each of the deleterious materials in a total sample of aggregates being tested. Figure 3.1 illustrates the materials which are classified as deleterious and the specification limits for each.

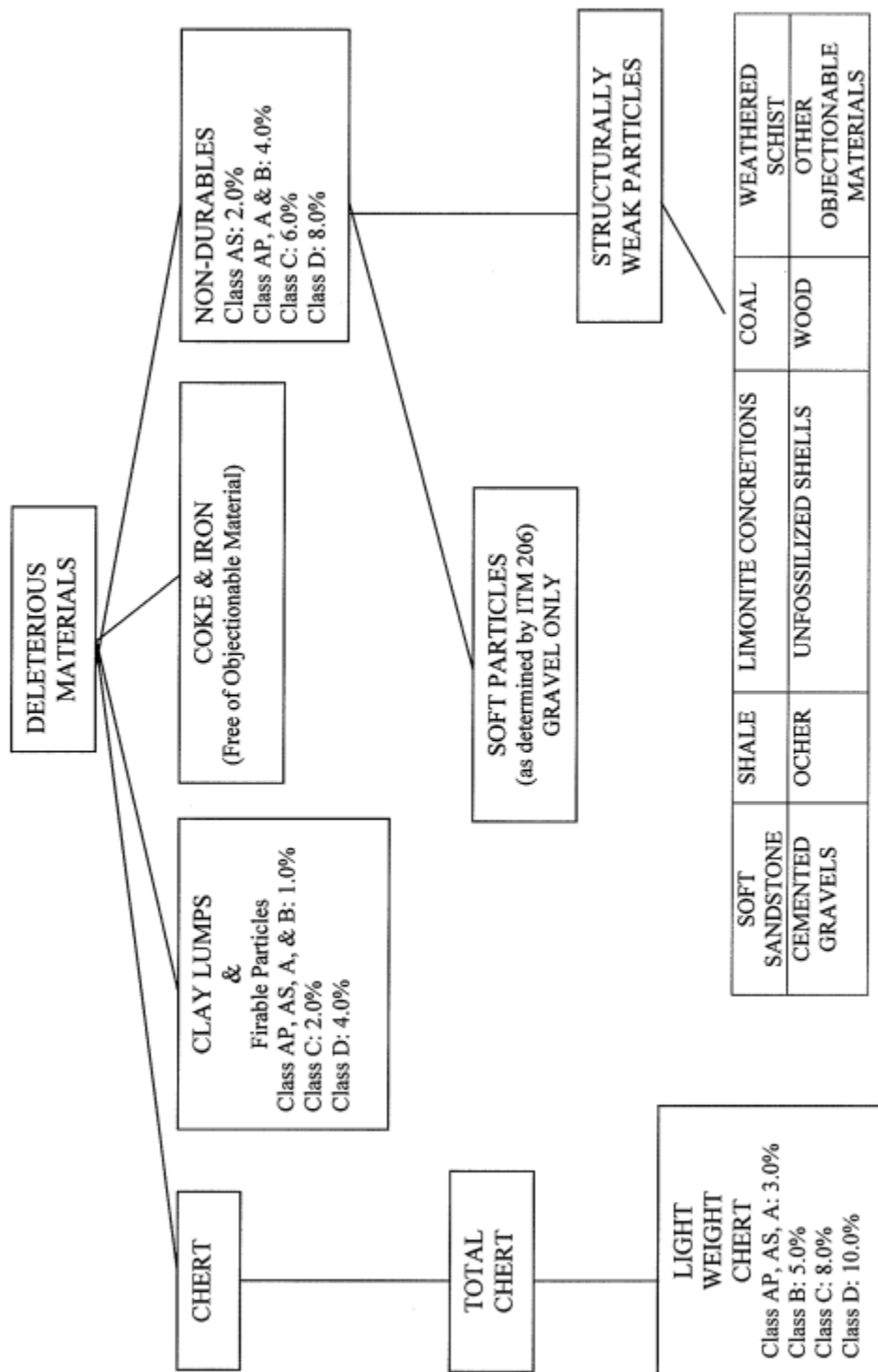


Figure 3.1 Deleterious Materials

## CLAY LUMPS AND FRIABLE PARTICLES:

(AASHTO T 112)

Clay lumps and friable particles are materials that are easily crumbled or mashed with the fingers. Testing for these particles is performed by AASHTO T 112, Clay Lumps and Friable Particles in Aggregates.

## NON-DURABLE PARTICLES

(ITM 206)

Non-durable particles are divided into two types: soft materials as determined by ITM 206, Scratch Hardness, and structurally weak material as determined by visual inspection. Structurally weak materials include the following:

1. Conglomerates-cemented gravels
2. Soft sandstone
3. Shale - laminated rock of clay size minerals
4. Limonite - iron oxide ranging in color from brown to black and is frequently a concretion around a soft core
5. Weathered schist-structurally weak
6. Ocher - soft rock clay to sand particles cemented with iron oxide which ranges in color from tan, through yellows, reds, and browns (looks and acts like chalk)
7. Shells - unfossilized shell of fresh water clams
8. Coal, wood, and other foreign materials
9. Materials with loosely cemented grains and/or a weathered coating
10. Other objectionable material

The technician determines the amount of these particles using procedures defined in other chapters.

## COKE AND IRON

Coke and iron are deleterious components found in blast furnace slag material. Limits are specified in section 904.03.

## LIGHTWEIGHT CHERT

(AASHTO T 113)

Chert is a rock of almost any color and is composed of glassy silica and very fine-grained quartz. Chert breaks into rounded surfaces with sharp edges. Unweathered chert appears hard, dense and brittle with a waxy or greasy texture. Weathered chert appears chalky or earthy and porous with a dull texture.

Lightweight chert is defined as aggregate that has a bulk specific gravity less than 2.45. The bulk specific gravity is determined using the saturated surface dry condition. Total chert is floated in a heavy liquid with a specific gravity of 2.45 so that lightweight chert will float and other chert will sink.

The Technician determines the amount of total chert using procedures in other chapters.

## SPECIAL REQUIREMENTS

In some cases, aggregates are required to meet special requirements for a particular use in construction as required by various Sections of 904. Some contracts may specify a unique gradation or aggregate. Details pertaining to this special requirement appear in the standard specifications and supporting documents.

## FINE AGGREGATES

(ITM 202)

The fine aggregate, including blended fine aggregate, used in HMA Surface 4.75 mm mixtures is required to have an acid insoluble content of not less than 40 percent. For air-cooled blast furnace slag or granulated blast furnace slag sand, the acid insoluble content is required to not be less than 25% for this application. Acid insoluble requirements do not apply to crushed gravel, limestone, or dolomite sands. The acid insoluble content is determined by ITM 202.

All fine aggregates used for HMA are required to be in accordance with 904.02 for soundness. If soundness testing cannot be conducted, the aggregate is required to originate from a Category I source in accordance with ITM 203.

The total blended aggregate from the fine and coarse aggregates, and recycled materials used in HMA are required to meet the fine aggregate angularity (FAA) requirements of Section 904.02(b). The procedure for determining the FAA value is described in Method A of AASHTO T 304.

## FLAT AND ELONGATED PARTICLES

(ASTM D 4791)

A special requirement placed on all coarse aggregates concerns the restriction on the number of flat and elongated pieces. Section 904.03 sets the limits for the number of flat and elongated pieces. A flat and elongated piece is defined as one having a ratio of length to thickness greater than five. The test method for determining the actual percentage of elongated pieces is ASTM D 4791.

## DOLOMITIC AGGREGATES

(ITM 205)

There is a special requirement to be met when dolomitic coarse aggregates are specified in HMA. These aggregates are specified for use under some conditions to obtain high friction, skid resistant HMA surface courses. ITM 205 is used to verify that the aggregate is carbonate rock containing at least 10.3 percent elemental magnesium.

## POLISH RESISTANT AGGREGATES

(ITM 214 and ITM 221)

Aggregates that meet the requirements of ITM 214 may be used in place of dolomitic aggregates in HMA surface mixtures. The procedure for approval requires initial British Pendulum testing, placement of a test section on an INDOT contract, and subsequent skid testing for two years. As an alternate approval process aggregates may be approved through testing with the Dynamic Friction Tester, Circular Track Meter/Circular Track Polishing Machine. Under this method the requirement for two-year skid testing may be waived.

Polish Resistant Aggregates in HMA Surface Course rated Category 4 ESAL can be evaluated per ITM 221.

## SANDSTONE AGGREGATES

Coarse sandstone is required to meet the Class B quality requirements and may only be used in HMA or SMA surface mixtures. Sandstone is defined as a sedimentary rock composed of siliceous sand grains containing quartz, chert, and quartzose rock fragments in a carbonate matrix or cemented with silica, calcite, or dolomite.

## SLAG AGGREGATES

When slag is furnished as an alternate to natural aggregate and payment is on a weight basis, adjustments are required to be made to compensate for the difference in specific gravity of the slag compared to the specific gravity of the natural aggregate. For any pay item less than 500 tons on a contract, no adjustment is made. The following typical values are used.

TYPICAL VALUES FOR SPECIFIC GRAVITY	
Natural aggregates (both fine and coarse)	<b>2.6</b>
Air cooled blast furnace slag coarse aggregate	<b>2.3</b>
Air cooled blast furnace slag fine aggregate	<b>2.6</b>
Granulated blast furnace slag fine aggregate	<b>2.1</b>
Steel furnace slag, both fine and coarse	<b>3.2</b>

**Figure 3.2 Typical Values for Specific Gravity**

## TYPE AS AGGREGATES

(AASHTO T 327 and ITM 220)

Aggregates used for stone matrix asphalt mixtures are required to meet the requirements of AS aggregates in accordance with Section 904.03 (a). These requirements include testing with the Micro-Deval abrasion apparatus (AASHTO T 327) and determination of the aggregate degradation in accordance with ITM 220. Additional requirements for control of the specific gravity of the steel furnace slag are included in Section 904.01.



## CRUSHED AGGREGATES

(ASTM D 5821)

There is a specific requirement for gravel coarse aggregates regarding crushed particles. This requirement applies, however, only when gravel coarse aggregates are used in HMA, compacted aggregates, and asphalt seal coats except asphalt seal coats used on shoulders. Crushed particles are defined as those particles having one or more sharp, angular, fractured faces. Fractured faces that have an area less than 25% of the maximum cross-sectional area of the particle are not considered crushed. ASTM D 5821 is used to determine the crushed particle content. Crushed gravel is required to comply with the requirements in Section 904.03.

## TYPE AP AGGREGATES

(ITM 210)

INDOT requires specific applications of portland cement concrete to be constructed with AP aggregate. Details and parameters concerning AP aggregate are described in ITM 210.

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# GENERAL USAGE REQUIREMENTS

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The general usage requirements describe the type of material which is considered acceptable for the type of construction, and the requirements which influence the acceptability of the material.

## **FINE AGGREGATES**

Section 904.02 states that fine aggregate is required to consist of natural sand or manufactured sand produced by crushing limestone, dolomite, steel furnace slag, air cooled blast furnace slag, granulated blast furnace slag, or wet bottom boiler slag. At the time of use, these materials are required to be free from lumps or crusts of hardened or frozen materials.

THE SPECIFIC REQUIREMENTS OF FINE AGGREGATES IN ACCORDANCE WITH SECTION 904.02:	
<i>TYPE OF CONSTRUCTION</i>	<i>ACCEPTABLE FINE AGGREGATE</i>
Portland cement concrete for pavement or bridge decks	Natural sand
Portland cement concrete for other construction	Natural sand or crushed limestone, dolomite, or air-cooled blast furnace slag
Hot mix asphalt	<ul style="list-style-type: none"> <li>Natural sand or manufactured sand.</li> <li>Steel furnace slag sand is permitted with steel furnace slag coarse aggregate.</li> <li>A combination of natural sand and manufactured sand is permitted. However, not more than 20 percent of the total aggregate used in HMA surface mixtures with ESAL equal to or greater than 3,000,000 may be crushed limestone sand if the limestone sand is from a source not on the Approved Polish Resistant Aggregate List.</li> </ul>
Pneumatic placement	Natural sand suitable for use with a pneumatic sand cement gun
Mortar	Natural sand
Mineral Filler	Dust produced by crushing stone, Portland cement, or other inert mineral matter
Snow and ice abrasives	Steel furnace slag, air-cooled blast furnace slag, granulated blast furnace slag, natural sand, crushed stone sand, or cinders

**Figure 3.3 Fine Aggregates for Construction**

## COARSE AGGREGATES

Section 904.03 includes the general requirements for coarse aggregate. This section lists several of the types of materials that may be used as coarse aggregate, and their applications and limitations.

CLASS OF COARSE AGGREGATES REQUIRED FOR VARIOUS TYPES OF CONSTRUCTION	
<i>TYPE OF CONSTRUCTION</i>	<i>REQUIRED QUALITY CLASSIFICATION</i>
Aggregate Base	Class A, B, C, or D
Subbase	Class A or B (No. 8) Class A, B, C, or D (No. 53)
Aggregate Pavements or Shoulders	Class A, B, C, or D

CLASS OF COARSE AGGREGATES REQUIRED FOR VARIOUS TYPES OF CONSTRUCTION	
HMA base coarse	Class A, B, C, or D
HMA intermediate coarse	Class A, B, or C
HMA surface coarse	Class A or B
SMA surface coarse	Class AS
Asphalt seal coat	Class A or B
Portland cement concrete pavement	Class AP
Portland cement concrete structural-- exposed	Class A or AP
Portland cement concrete structural -- non-exposed	Class A or B
Cover (choke) coarse aggregate	Class A or B

**Figure 3.4 Coarse Aggregates for Construction**

Where more than one aggregate classification is allowed, the Contractor has a choice, unless specified by provisions within a given contract. The class of aggregate may never be less than the lowest class for the designated use. For example, the highest class of aggregate for HMA surface course, Class A, may be used (with no additional payment to the Contractor or Producer). Class B aggregate may be used as the minimum requirement.

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## GRADATION REQUIREMENTS

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The gradation or particle size distribution of an aggregate is usually specified to be within certain limits for various types of construction. There is a great difference between what is considered an acceptable grading for aggregates for the various HMA mixes, for portland cement concrete, or for base layers. The gradation that aggregates are to meet for specific types of construction is contained in the contract plans, Special Provisions, or Standard Specifications and is usually designated by the aggregate size.

Sections 904.02 and 904.03 contain tables describing the acceptable particle size distribution for various sizes of both fine and coarse aggregates. Section 904.04 specifies the sizes for riprap and Section 904.05 lists the acceptable gradations for structure backfill.

## **FINE AGGREGATES**

The table found in Section 904.02(b) is used to accept six aggregates used for HMA mixes, portland cement concrete, pneumatic placement mortar, mortar sand, mineral filler, and snow and ice abrasives. The most common sizes of fine aggregates are No. 23, No. 24, No. 15, No. 16, PP, and S&I. No. 16 is the finest aggregate, because 100 percent of the fine aggregate is required to pass the No. 30 sieve. No. 23 is the coarsest of the six sizes. Quality assurance fine aggregate materials may be approved by the engineer. All fine aggregate particles are generally expected to pass the No. 4 sieve.

The aggregates for mortar sand are required to meet the gradation for size number 15 or an approved gradation from a CAPP source. The fine aggregates for pneumatic placement may meet size number 15, PP, or an approved gradation from a CAPP source. Mineral filler for SMA is required to meet size number 16.

Snow and ice abrasives are required to meet the gradation requirements of Section 904.02(g).

## **COARSE AGGREGATES**

The table found in Section 904.03 applies to coarse aggregates. The most common sizes of coarse aggregates are No. 2, No. 5, No. 8, No. 9, No. 11, No. 12, No. 43, No. 53, No. 73, and No. 91. No. 2 is the coarsest size and No. 12 is the finest. No. 53 and No. 73 are dense graded aggregates, and No. 91 is used for aggregates in precast concrete. Quality assurance coarse aggregate materials may be approved by the engineer. The majority of the coarse aggregate is retained on the No. 4 sieve and larger.

## **B BORROW AND STRUCTURE BACKFILL**

B Borrow and structure backfill requirements are listed in Section 211, 904.05 and 904.06 of the INDOT Standard Specifications.

Materials for B Borrow are required to contain no more than 10% passing the No. 200 sieve and be otherwise suitably graded as noted in Section 904.06. The use of an essentially one-size material is not permitted unless approved.

Materials for structure backfill are required to be of acceptable quality, free from large or frozen lumps, wood, or other extraneous matter. Structure backfill gradations are included in Section 904.05. Aggregate sizes No. 5, No. 8, No. 9, No. 11, No. 12, No. 53, and No. 73 crushed stone or

air-cooled blast furnace slag are allowed. Additional aggregate sizes permitted for structure backfill are listed in the table in section 904.05. The structure backfill types that allow aggregates and the specific uses of each type are defined in 211.03.01 and as follows.

Type 1 structure backfill is used in longitudinal or transverse structures placed under, or within 5 ft of, the back of paved shoulders or the back of sidewalks of a new facility. This type is also used for a structure of an existing facility where all existing pavement is to be replaced. Structure backfill in accordance with Section 904.05 may be used for Type 1 applications.

Type 2 structure backfill is used in longitudinal or transverse structures placed under, or within 5 ft of, the back of the paved shoulder or back of the sidewalk where undisturbed existing pavement is to remain. This type is also used for precast concrete three-sided or four-sided structures with a height of cover of 2 ft or more. Crushed stone or ACBF in accordance with Section 904.05 may be used for Type 2 applications (exception is No. 30, No. 4, and 2 in. nominal size aggregate).

Type 3 structure backfill is used behind mechanically-stabilized earth retaining walls. Structure backfill in accordance with Section 904.05 may be used for Type 3 applications, except only nominal size aggregates 1 in., 1/2 in. Structure Backfill or coarse aggregates No.5, No. 8, No. 11, or No. 12 may be used. No slag other than ACBF is permitted.

## **AGGREGATE BASE**

Section 301.02 includes the requirements for dense graded compacted aggregate material. No. 53 aggregate is used for this purpose.

## **SUBBASE**

Section 302.02 includes the requirements for subbase placed on a prepared subgrade for portland cement concrete pavement. Subbase consists of a No. 8 aggregate as the drainage layer over a No. 53 aggregate as the separation layer. Where a dense graded subbase is required, only No. 53 aggregate is used.

## **AGGREGATE PAVEMENTS OR SHOULDERS**

Section 303.02 includes the requirements for aggregates when used for pavements or shoulders. No. 53 and No. 73 aggregate are used for this purpose except that No. 73 aggregate is only used for surface courses.

## RIPRAP

Aggregate used for riprap is included in Section 904.04. These materials are typically large and are used as a protective coating as specified. Revetment, Class 1, Class 2, Uniform A, and Uniform B Riprap are required to meet the requirements of Section 904.04(f). The other ripraps listed have general size limitations.

## SUMMARY OF GRADATION REQUIREMENTS

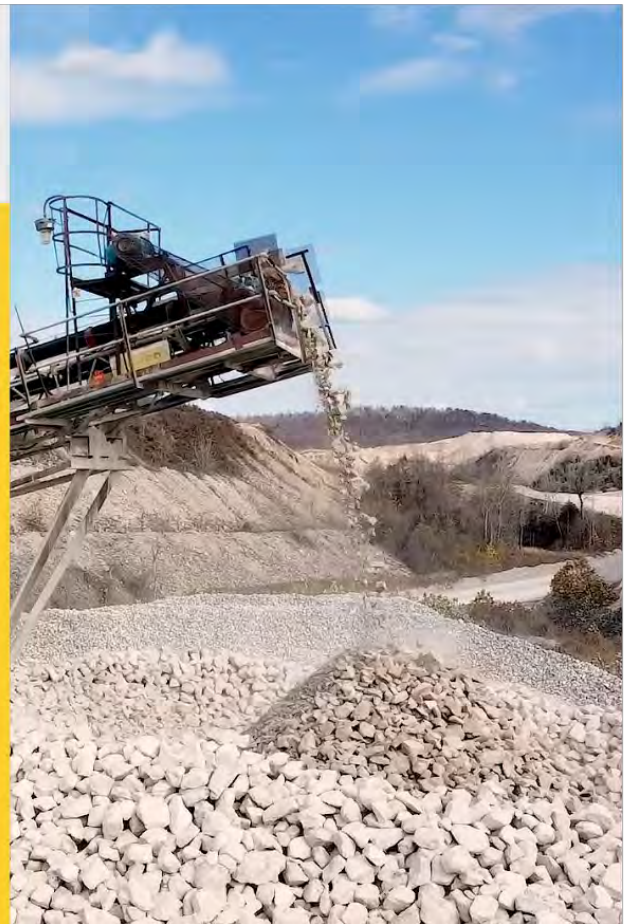
The gradation requirements for fine and coarse aggregates as specified in various sections of the Specifications for significantly different types of construction are summarized below. This listing is not all inclusive but covers the major uses of aggregates.

TYPE OF CONSTRUCTION REQUIREMENTS	GRADATION
Aggregate Base Coarse Aggregate	No. 53
Subbase Coarse Aggregate	No. 8, 53
Aggregate Pavements or Shoulders Coarse Aggregate	No. 53, 73
Asphalt Seal Coat Fine Aggregate Coarse Aggregate	Nos. 23 or 24 Nos. 8, 9, 11, 12 Nos. SC 11, 12, 16
Portland Cement Concrete Pavement (PCCP)/Structural Concrete Fine Aggregate Coarse Aggregate	No. 23 No. 8

**Figure 3.5 Gradation**

# AGGREGATE PRODUCTION

## CHAPTER 4



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# AGGREGATE PRODUCTION

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## **EXTRACTION**

- Stripping
- Drilling and Blasting
- Shot Rock or Gravel Bank

## **CRUSHING**

- Scalping
- Primary Crushing
- Secondary and Tertiary Crushing
- Impact Crushing

## **OTHER BENEFICIATION**

## **SCREENING**

- Product Quality
- Gradation Control

## **SAND PRODUCTION**

- Natural Sand
- Manufactured Sand
- Processing

## **SEGREGATION**

## **STOCKPILING AND HANDLING**

- Cone Stockpiles
- Radial Stockpiles
- Truck-Built Stockpiles
- Layered Stockpiles
- Stockpiling - General

## **DEGRADATION**

## **CONTAMINATION**

## **RETRIEVAL**



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# AGGREGATE PRODUCTION

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This chapter discusses the total process of aggregate production from extraction through processing. Also discussed is the handling, stockpiling, and shipping of the product up to the point where the material leaves the Producer's control. Processing influences mineral quality and integrity, aggregate physical properties, and in particular gradation (size control). Establishing a stable production process may reduce variability of the product.

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## EXTRACTION

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Most aggregate products are obtained from bedrock or unconsolidated deposits. Exceptions would include but not be limited to slag and other manufactured products. The majority of materials used in the mineral aggregate industry are obtained from surface-mined stone quarries or from sand and gravel pits. How materials are extracted influences their quality.

### **STRIPPING**

As a first step, a Producer is required to designate a detailed stripping procedure (Figure 4.1) for each and every deposit that is mined. This phase often is overlooked yet has a great influence on the quality and variability of the product. Inadequate removal of overburden from the mineral deposit often may be the source of excessive variation in minus No. 200 material and may even have a deleterious effect on nearby vegetation and other aspects of the mine.

For example, excessive knobs and depressions on the surface of a stone deposit may necessitate the use of smaller equipment or special techniques to clean the stone. Inexperienced equipment operators may easily corrupt good stripping practices (which are somewhat of an art and site specific). Spillage over the working face and other sloppy practices can also affect the cleaning process.



**Figure 4.1 Stripping**

## **DRILLING AND BLASTING**

Quarry operators commonly design fragmentation shots for safety, economy, ease of use at the primary crusher, and even public relations, but they often forget about quality. The shot layout is required to be properly engineered, documented, and adhered to for maximum consistency. Varying the shot pattern may mean changes in product size throughout the operation. Smaller shot rock, resulting in less crushing in the secondary and tertiary stages, may mean less improvement through crushing.



**Figure 4.2 Drilling**

Therefore, the mineral quality and/or changes in physical properties of the product may be affected.

Hole detonation-sequencing and blast intensity also are required to be properly engineered. Size changes resulting from inattention to detail can have the same effects as mentioned above. Also, an erratic blast that throws the shot rock over a large area tends to cause variations in size gradation that is delivered to the primary crusher. Any deviation from previously established shot patterns, sequencing, and intensity should be carefully thought out as to the effect on product quality.



**Figure 4.3 Blasting**

## **SHOT ROCK OR GRAVEL BANK**

A constant problem of gravel pit and quarry operators is the difficulty in maintaining uniform load-out from either the shot rock pile or the gravel bank. Even the best shot has some variation from side to side and from front to back. Only experienced and well-trained equipment operators may accomplish the mixing from around the shot for the most uniform feed to the processing plant.

Subsurface sampling and testing are helpful to gravel-pit managers where the size of the material changes. In many cases, for example, material from above the water level and below the water level may help in delivering uniform feed to the plant.

Changes in equipment, if done without thought as to how to maintain uniform sizing, also may have the same effect. Any change in equipment should be evaluated for effect. These changes may need to be placed as an addendum to the Producer's Quality Control Plan.



**Figure 4.4 Changes in Equipment**

Geologic variability in the deposit may sometimes affect sizing but more often causes a change in mineral integrity and physical properties. If a large variation exists, some products at later stages in the process may require separation.

Moisture variation in shot rock may also cause significant problems during processing. Shot-rock moisture should be monitored because significant changes in moisture almost always require changes in downstream processing.

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# CRUSHING

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The first step of processing begins after the extraction from quarry or pit. Many of these steps also are common to recycled materials, clay, and other manufactured aggregates. The first stage in most operations is the reduction and sizing by crushing. Some operations, however, provide a step prior to crushing called scalping.

## SCALPING

Scalping most often is used to divert fines at a jaw primary crusher in order to improve crusher efficiency. In this way the very coarse portion is crushed and then recombined with the portion of crusher-run material before further processing. This first step may, however, be an excellent time to improve a deleterious problem. If a deleterious or fines problem exists in the finer fraction of crusher-run material (namely, clay, shale, finely weathered material, etc.) the fall-through of the scalping operation may be totally or partially diverted and wasted or may be made into a product of lesser quality. In any case, only acceptable amounts, if any, should be returned back into the higher quality product. Consideration of process variables in this early stage may be very important.



**Figure 4.5 Scalping**



## PRIMARY CRUSHING

In stone quarries or in very "boney" gravel pits, large material usually is reduced in size by either a jaw (Figure 4.6) or a gyratory crusher. Both types are compression crushers. Although economical, they have the tendency to create thin, elongated particles. Particle shapes sometimes may be a problem for Producers of hot mix asphalt. In some operations impact crushers are used for primary crushing, but they may have a slightly higher cost per ton. Impact crushers may upgrade poor-quality aggregate and increase separation, such as removal of rebar from concrete in recycling operations.



**Figure 4.6 Jaw Crusher**

After primary crushing/reduction the resulting aggregate generally is placed in a large "surge" pile where the aggregate may be fed into the secondary operation whenever convenient.

Care is always taken when building up and loading out surge piles, as this step may be a major source of segregation of material going to the secondary plant. Variation at this point may affect both mineral quality and gradation. Drawing from an inverted cone over a load-out tunnel works well after material has been deposited and left undisturbed to form the walls of the draw-down cone. If the need ever arises to consume the entire pile, care is taken to thoroughly mix the older material a little at a time with fresh product to make the surge as uniform as possible as the aggregate is being pushed into the tunnel. If the operation relies on end loaders to feed the secondary plant from the surge pile (Figure 4.7), the same care is taken to mix coarse with fine material from the outside to the inside of the pile.



**Figure 4.7 Surge Pile**

## SECONDARY AND TERTIARY CRUSHING

Secondary and tertiary crushing, if necessary, are the final steps in reducing the material to a desired product size. Historically, cone and roll crushers were the most common choice crushers, but in recent years impact crushers are more widely used. These crushers also are sometimes used as primary crushers for fine-grained gravel deposits.

The cone crusher (a compression type) simply crushes the aggregate between the oscillating cone and the crusher wall (Figure 4.8). Clearance settings on this equipment are suggested to be checked and maintained as part of standard operating procedure.



**Figure 4.8 Cone Crusher**

As with other compression crushers, the cone crusher yields a somewhat elongated and slivery particle shape. This may be minimized, however, by "choke" feeding the crusher. This technique will also make the shape and size more uniform. One way to choke feed is with a surge hopper and a controlled belt-feed to the cone crusher (Figure 4.9). Automatic level controls measure the head of the material over the top of the cone.



**Figure 4.9 Choke Feed**

A roller crusher (Figure 4.10) is another compression type crusher that simply breaks the material by pinching the aggregates. These types of crushers are often found in gravel operations. Roller crushers have constant maintenance problems and are prone to excessive wear. The rollers are required to be checked frequently to insure proper clearance and uniformity across each roller. Rebuilding and re-milling the roller is a standard operating procedure.



**Figure 4.10 Roller Crusher**

## **IMPACT CRUSHING**

Impact crushers may be used as primary, secondary, or tertiary crushers. Despite having a somewhat higher operating cost than other crushers, they tend to produce a more uniform particle shape. Impact crushers usually will benefit the aggregate better than compression crushers, and they may generate more fines. Common types are the horizontal shaft (Figure 4.11), vertical shaft, and hammermill impactors.

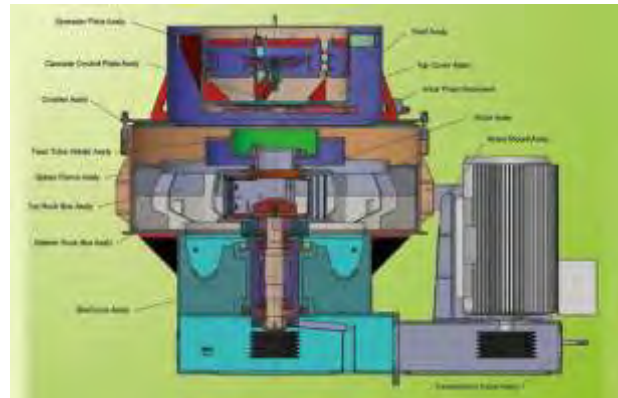


**Figure 4.11 Horizontal Shaft**



The horizontal shaft single or double rotor may aggressively handle large and odd-shaped material. Large horizontal impactors sometimes are used as primary crushers. Fracturing occurs at the same time by rock against rotor, rock against breaker bar, and rock on rock.

The vertical shaft impactor (Figure 4.12) is operated in rock against anvil, or rock against rock (through the installation of a rock shelf) modes. The Producer is required to decide carefully the mode best suited to the raw material.



**Figure 4.12 Vertical Shaft**

The hammermill impactor (Figure 4.13) provides excellent reduction and beneficiation through the impacting and shearing action of the hammers and grates; however, a large amount of fines is produced. This type of crusher is sometimes used to manufacture agricultural ground limestone.



**Figure 4.13 Hammermill Impactor**

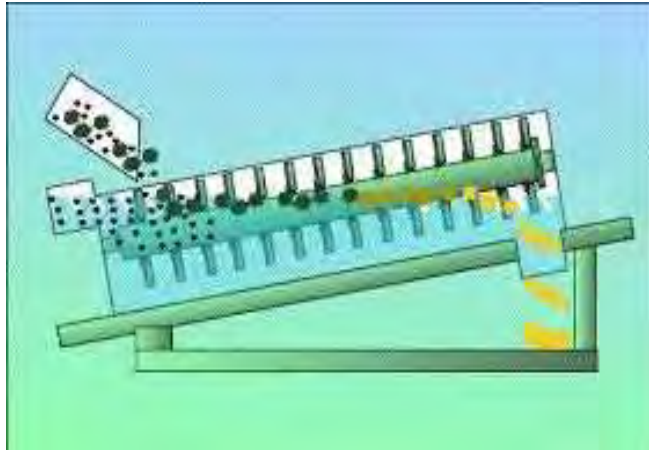
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# OTHER BENEFICIATION

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Other forms of beneficiation for quality are available to the Producer. These may include the log washer, heavy media separator, or attrition mill.

The log washer (Figure 4.14) commonly is used in wet operations to agitate and scrub clay and other objectionable fines from coarse aggregate. A Producer may need to use a log washer when rinsing screens do not remove these objectionable fines.



**Figure 4.14 Log Washer**

Heavy media separation is somewhat costly but may be the only practical way for a Producer to meet quality requirements. This method works only when the undesirable material has a different specific gravity than the desirable material. The deleterious material is discarded after the media is separated for recycling.

Attrition mills are seldom used but remain an option when the deleterious particles are uniformly softer than the non-deleterious particles. The attrition mill abrades the deleterious particles into fines that may be screened out of the system.

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# SCREENING

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Screening is another technique to control both quality and gradation of the aggregate product.

## PRODUCT QUALITY

If deleterious material exists at undesirable levels after crushing and may be identified as being predominantly in one size range that is not needed for product size, the material may be screened out (fines or top size). This step may occur between crushing so that an opportunity exists to recreate the same size downstream, if needed, to create a product. The screened-out lower-quality material may be used for a lower quality product or wasted if no use exists.

The rinse screen (Figure 4.15) is also commonly used. By processing the material over a screen that retains all of the product, the clay and deleterious fines may be rinsed away to make the product acceptable.

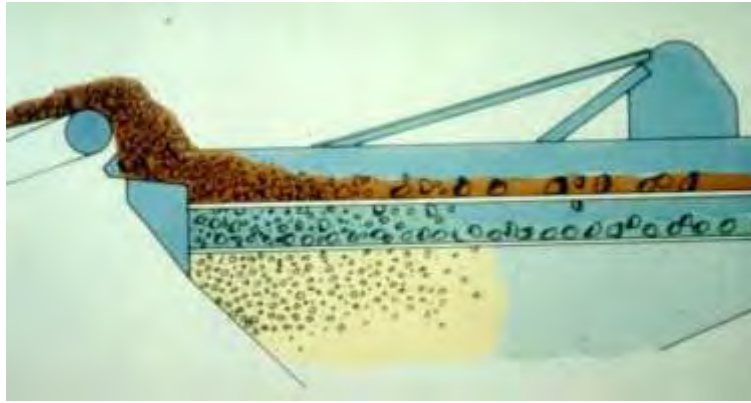


**Figure 4.15 Rinse Screen**

## GRADATION CONTROL

The best technique for gradation control is screening (Figure 4.16). Screening may be done wet or dry, depending on the type of aggregate being processed and the degree of consistency required for each product.

Washing, for example, may be necessary to clean a concrete aggregate, but may not be needed for hot mix asphalt products, which may contain more fines. For gradation control alone, however, consistency sometimes may only be maintained by using wet screening. Gradation consistency is usually an overriding factor for a hot mix asphalt customer. Water volume and flow direction are critical in wet screening. Frequent checking of the gradation is a standard operating procedure.



**Figure 4.16 Screen Deck (Double)**

Dry screening is a slight misnomer because the material passing over the screen decks is wet, ranging from slightly damp to very wet, depending on conditions such as rain or subsurface moisture. Non-washed screening is a more accurate description of this screening process. High moisture is a concern because the wet aggregates may cause some material to become sticky and bind together, making the aggregate harder to separate. Furthermore, high-moisture conditions may cause binding of lower screen decks, causing override of the material rather than separation. If these conditions are encountered, the Producer may need to establish a balance between the moisture content of the incoming material and the feed rate through the screens. This balance should be made for each hour of operation. If reduced feed rates do not solve the problem or are too costly, washing or an additional screen area may be needed.

Sometimes screening variation is too great even under the most favorable of conditions. When this occurs, the Producer should inspect and ensure that the equipment and the screen cloth are in good repair. The most common reason for high screening variability is the tendency to push too much material over a screen. The only way to maintain a bed of material thin enough for optimum efficiency is to provide enough screening to allow the desired rate of production. Standard operating procedures should reflect the maximum feed rate for the design of the plant.

For well-graded products having many sieves, (namely #53s), gradation control may not be done without first separating the material into fractions. Separating the material into numerous small fractions and then back-blending at a set rate for each fraction may be necessary to control the gradation.

Frequent sampling, testing, and control charting are necessary for monitoring because aggregate gradation is subject to many variables.

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# SAND PRODUCTION

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Sand plays a critical role as a construction aggregate and deserves special attention when considering the means of process control. Unlike coarse aggregate where various types of crushers may be used to upgrade mineral quality, sand basically relies on the same techniques to address both mineral quality and sizing. These techniques are called particle exclusion. Whichever size the Producer decides to eliminate for quality reasons obviously also affects the sizing.

## **NATURAL SAND**

Good quality natural sand is readily available in many areas and may be easy to obtain and process. As with the gravels that they often accompany, the sand deposits may not have been laid uniformly, meaning a potential change in quality and size is possible. In some deposits, sand found below the water table differs in fines content and quality from that found above the water table. Subsurface drilling, sampling, and testing is necessary to know to what degree and where these differences occur. Standard operating procedures in the Quality Control Plan should address the process if differences in size and quality are encountered, as a uniformly graded product of predictable quality is required to be maintained.

## **MANUFACTURED SAND**

Because of the angularity, manufactured sand is very beneficial for use in hot mix asphalt where stability is critical. Many Indiana quarries are high in clay content and often a large amount of dust ends up in the feed stock for manufactured sand. Care should be taken to select the appropriate classification equipment that removes the necessary amount of minus No. 200 yet retains other fractions of the sand gradation that are needed. For some uses, particle shape is important. Particle shape is set primarily by the crushing operation for the coarse aggregate. Any changes in crushers or crushing techniques may affect the properties of the manufactured sand product and therefore affect the customer's use of the product.

## PROCESSING

Very few sand products are produced by air classification or by direct non-washed screening. Most sands are produced with wash water and water classification. The key to all rinsing and water classifying systems is adequate delivery of water. Inadequate water supply and poor maintenance are the two most common reasons for inconsistent sand gradations.

The most common water classifier is a simple dewatering screw (Figure 4.17) which may make a single "cut" in gradation and float out a certain amount of fines. By altering the through-put and rate of water flow the cut point may be changed.



**Figure 4.17 Dewatering Screw**



**Figure 4.18 Dewatering Wheel**

A variation of the dewatering screw is the dewatering wheel (Figure 4.18). This device also is capable only of making a cut in the feed stock but may be more finely tuned and may be the better choice when trying to retain as much No. 50 and No. 100 material as possible.

An even more sensitive method of cutting out fines is the wet cyclone (Figure 4.19). The sand slurry in the cyclone is spun at a prescribed velocity. Centrifugal force separates the coarser fraction from the water and fines which exit to the pond.





**Figure 4.19 Wet Cyclone**

Any of these techniques could conceivably be used with others in tandem or in tandem with rinse screens. The material could then be back-blended to create a desired product. A simpler and probably more cost-effective way to control a sand gradation on multiple sieves is the rising current, multiple cell classifier (Figure 4.20). This equipment has numerous cells, each having varying water pressures that for different sizes of material. Any number of cells may then be combined to create the final product. With this type of system, a high degree of process control is possible.



**Figure 4.20 Multiple Cell Classifier**

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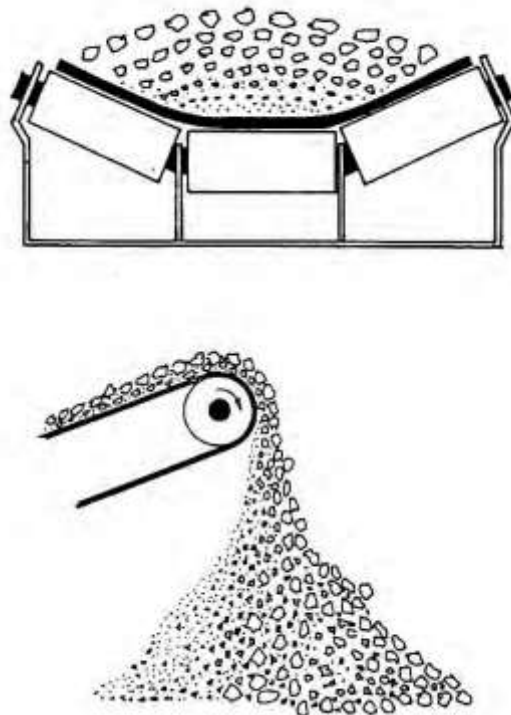
# SEGREGATION

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Segregation is the non-uniform distribution of coarse and fine aggregate components within a mixture of aggregates. Product conformity and uniformity may be predicted if all of the inputs into the plant are measured, evaluated, and controlled. Whenever one rock is placed upon another rock, segregation may reduce the uniformity that the Producer has carefully built into the product.

Segregation begins on the belt where fines vibrate to the bottom and coarse aggregate remains on the top as the material bounces across the idlers (Figure 4.21). At the end of the belt, if left un-deflected, the coarse particles are thrown out and away. Fine particles, on the other hand, tend to drop down or if wet even follow back underneath the conveyor. The greater the speed of the belt, the worse the segregation problem is. This is known as front-to-back segregation and may be addressed by the following methods:

1. Belt wipers underneath the head pulley that reduce carry back
2. Movable stackers kept near the top of the pile to reduce the spread
3. Mixing paddles or deflectors at the head pulley to keep the material together
4. Wider belts at lower velocities to prevent segregation



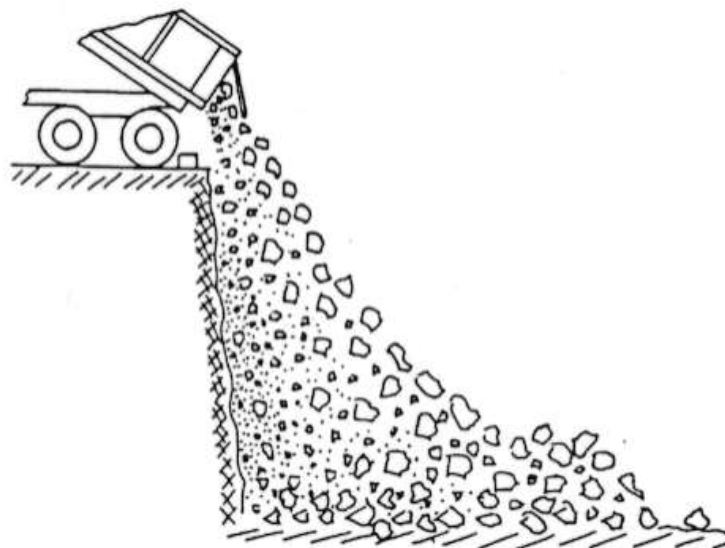
**Figure 4.21 Belt Segregation**





**Figure 4.22 Roll Down**

A second common type of segregation is "roll down," which occurs any time aggregate is piled so that large particles roll down the sloped side of the pile (Figure 4.22). The higher the pile, the worse this problem is. This type of segregation is very obvious in operations with high conical stockpiles, but also occurs in improperly loaded trucks. Keeping storage bins over half-full whenever possible improves the situation.



**Figure 4.23 Roll Down Segregation from Dump Truck**

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# STOCKPILING AND HANDLING

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Segregation is probably the greatest problem that occurs because of stockpiling and handling, but certainly other problems such as degradation and contamination may adversely affect product quality. Every possible precaution should be taken to protect the product quality from the point of manufacture to the point where the aggregate leaves the Producer's control.

## CONE STOCKPILES

Although the cone stockpile is very common in the aggregate industry, two stockpile procedures may easily reduce product integrity. Roll-down segregation obviously occurs in full circle around the pile, and very high piles are difficult to adequately remix before shipping. These piles usually are being replenished with fresh material as old and new material is being removed, which keeps the product size in a state of continual change (Figure 4.24).



**Figure 4.24 New Production Added to Old**

In some cases, the "front-to-back" segregation adds extra coarse material thrown forward and extra fines carried back for even greater variability. In addition, some piles are not fully retrieved for several years and the new product that is added to the old pile may even have

different production targets (Figure 4.24). Situations like these add up to serious problems for predicting gradation uniformity in the retrieved product.



**Figure 4.25 Front to Back Segregation**

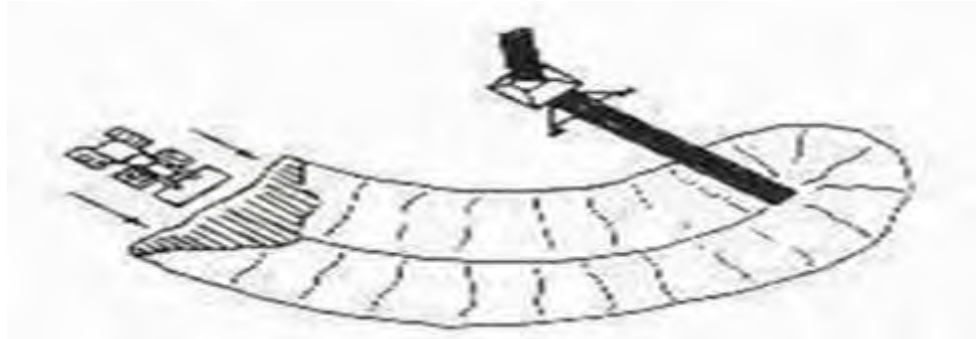
The final element of a cone pile that adds to the effects of both the roll-down and front-to-back phenomena is an excessively high drop from the end of a fixed conveyor to the top of the pile (Figure 4.26). This procedure should be avoided. Use of cone stockpiles should be kept to a minimum and used with extreme caution.



**Figure 4.26 High Drop**

## RADIAL STOCKPILES

A radial stacker (Figure 4.27) is a compromise solution for conveyor-built stockpiles, especially if kept to less than 20 ft. The proper technique is to keep the end of the movable conveyor less than a meter from the top of the pile and raise the conveyor with the pile to the full height. Then the conveyor is moved horizontally with the pile in small increments. In this manner the pile is constructed at one end while the products are retrieved at the other end.



**Figure 4.27 Radial Stacker**

Although roll-down segregation does occur from the sides of the pile, a continual remixing of coarse and fine material occurs longitudinally as the pile advances. Proper retrieval may take care of the edges.

## TRUCK BUILT STOCKPILES

If piles from the end of the product belts are thoroughly remixed then truck-built stockpiles (Figure 4.28) are capable of greatly minimizing segregation, if the trucks are loaded properly. The best truck-built stockpiles are those that are constructed one dump high with each dump placed against previously dumped material. This procedure, because of the low profile, reduces roll-down segregation and allows remixing during load out. However, these stockpiles require more space than the others mentioned. A technique that may help reduce segregation is to place truck dumps on top of other dumps with a large end loader operating from ground level. In this case, care should be taken to place the upper lift back from the edge far enough that a long-sloped face is not created that would cause the same kind of roll-down problem that this type of pile is meant to eliminate.



**Figure 4.28 Truck Built Stockpiles**

## **LAYERED STOCKPILES**

A layered stockpile, if built correctly, may also greatly minimize segregation. Unfortunately, these types of stockpiles are very difficult to build properly. Each layer is placed uniformly across the top of the pile in thin horizontal lifts. Care should be taken to keep the edge of each new lift set back from the edge of each previous lift so as not to create long sloped edges. This is best done with a large clam shell crane, which is slow and tedious, or with specially made equipment that may place the layers without being on the pile. A compromise is to allow hauling equipment on top of the pile; however, this procedure causes degradation of the product, and the pushing equipment may move the material over the edges causing severe segregation (Figure 4.29). Generally, these activities are poorly managed, and the stockpile takes on the shape of a ramp and spills over. These situations are very detrimental to product quality.



**Figure 4.29 Layered Stockpile**



## STOCKPILING - GENERAL

The Producer should write standard operating procedures on building stockpiles for each product to educate all those involved in their responsibilities in the procedure. Most stockpiling problems are created because of inconsistent management. Illustrations at the end of this chapter indicate the different techniques that may be used for stockpiling and retrieving. The Segregation Index (S.I.) indicated with each example is a numerical index where the numbers are associated only with the other techniques and indicate greater segregation severity as they become higher.

## DEGRADATION

Degradation or breakdown of the product is often caused by equipment running on top of the aggregate when the aggregate is being stockpiled (Figure 4.30). When this occurs, the degraded portion of the pile should be discarded before shipping. The difficulty lies in knowing where the "bad" material begins and ends. Extensive sampling and testing in these cases may be needed prior to shipping to determine what product is not good enough to ship. Degradation may also occur during retrieval where some of the lower portion of the pile is carelessly run over with equipment while loading out. A Producer should know which products tend to degrade with handling and make appropriate allowances. For example, many stone sands increase in minus No. 200 content each time they are loaded and moved. In some cases, old stockpiles may degrade through weathering. Piles two years old and older should be rechecked for gradation before shipping and possibly even for mineral quality.



**Figure 4.30 Typical Cause of Degradation**

## CONTAMINATION

Contamination (Figure 4.31) is usually the result of carelessness and poor housekeeping. To save space, stockpiles of different products are placed close together and as they grow in size they may grow together. Equipment also may track dirt or other foreign matter into the product pile area. Old piles are subject to wind-blown fines over time and are required to be checked for this before shipping.



**Figure 4.31 Contamination**

## RETRIEVAL

Retrieving material properly from a stockpile is just as important as building the stockpile properly (Figure 4.32). Truckers often force their way into the loading area, causing the loader operator to load from areas other than the working face. This practice should not be allowed. Strict procedures for load out should be written, adhered to, and become routine as part of standard operating procedures. Loading from the outside of an un-worked pile for the sake of convenience may very quickly result in an unsatisfactory product.

Cone-shaped stockpiles are the most difficult to approach. Once retrieval has begun, no new material should be added to the pile. To maintain a representative gradation, exactly one-half of the pile should be removed, the edges (coarse) folded into the center (fine), and the entire mass turned over and made into a level pad. The product is then ready for shipping. After shipping the first half of the pile, the procedure should be repeated for the second half. New material should be placed elsewhere in the meantime.

For radial or tent-shaped stockpiles, retrieval should begin at the oldest end while new material continues to be placed at the other end. The first entry into a new pile is handled as described above since the beginning of a radial pile is half-conical shaped. After a face has been established parallel to the stacking conveyor, continued mixing occurs in front of the load out face by pulling material from the center of the pile and mixing the material with the edges. The face is required to be kept as uniform as possible. At no time should new material be placed at the load out face.

For layered stockpiles more than one loader bucket high, remixing is necessary as the height of the pile and type of the product increases. For low-profile truck-built stockpiles, only minor remixing is necessary when encountering the edges.



**Figure 4.32 Retrieving Materials**



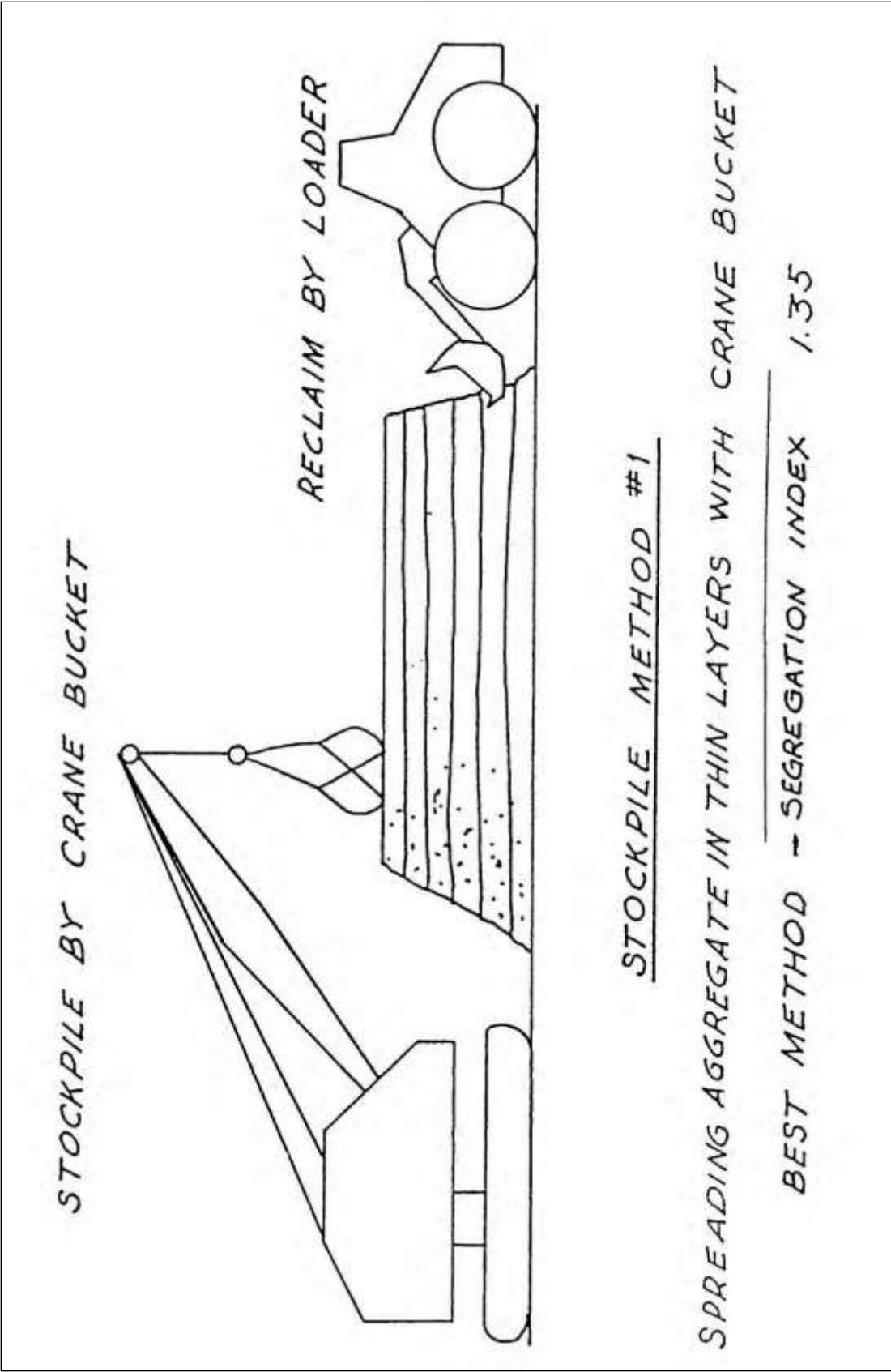


Figure 4.33

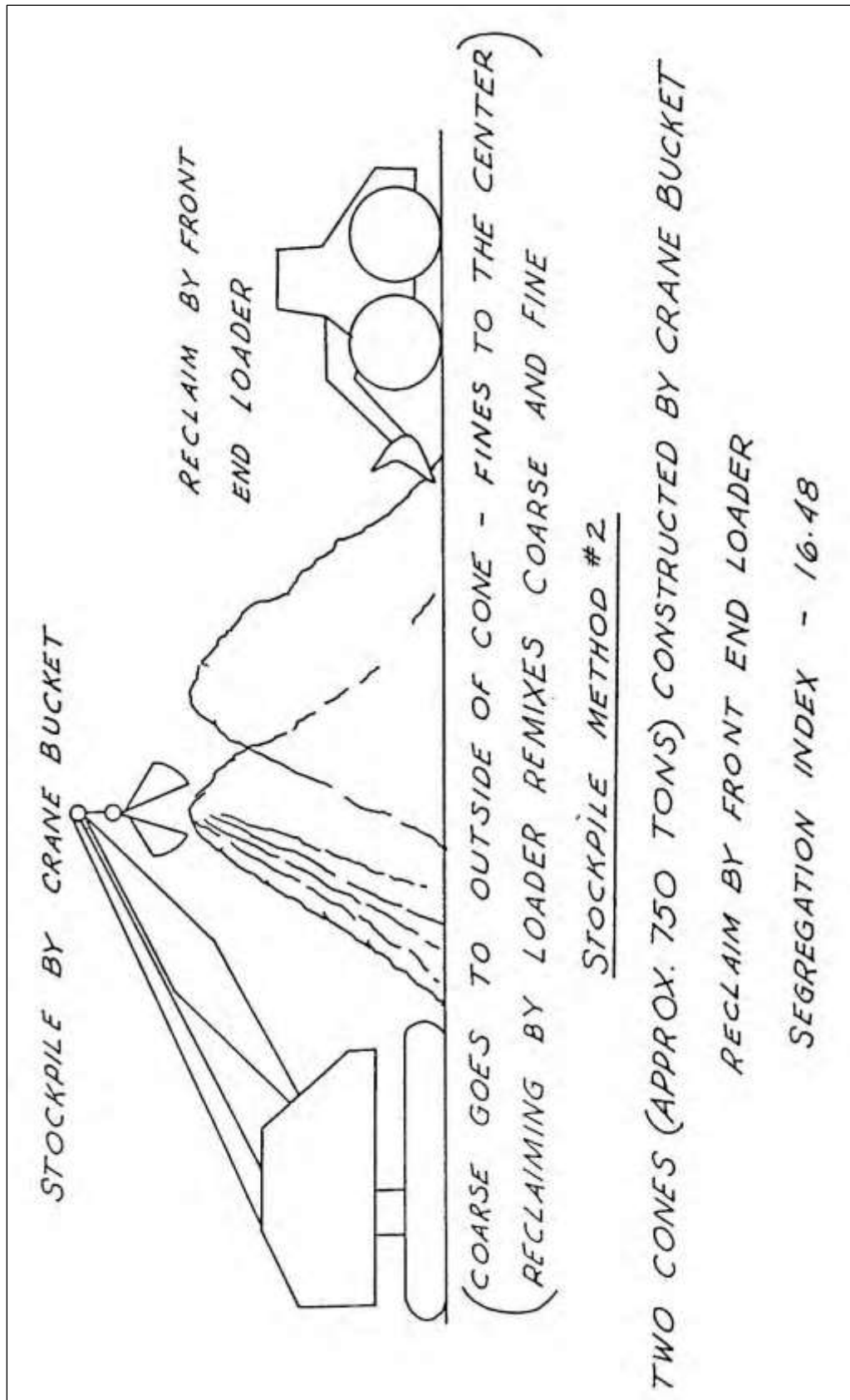


Figure 4.34

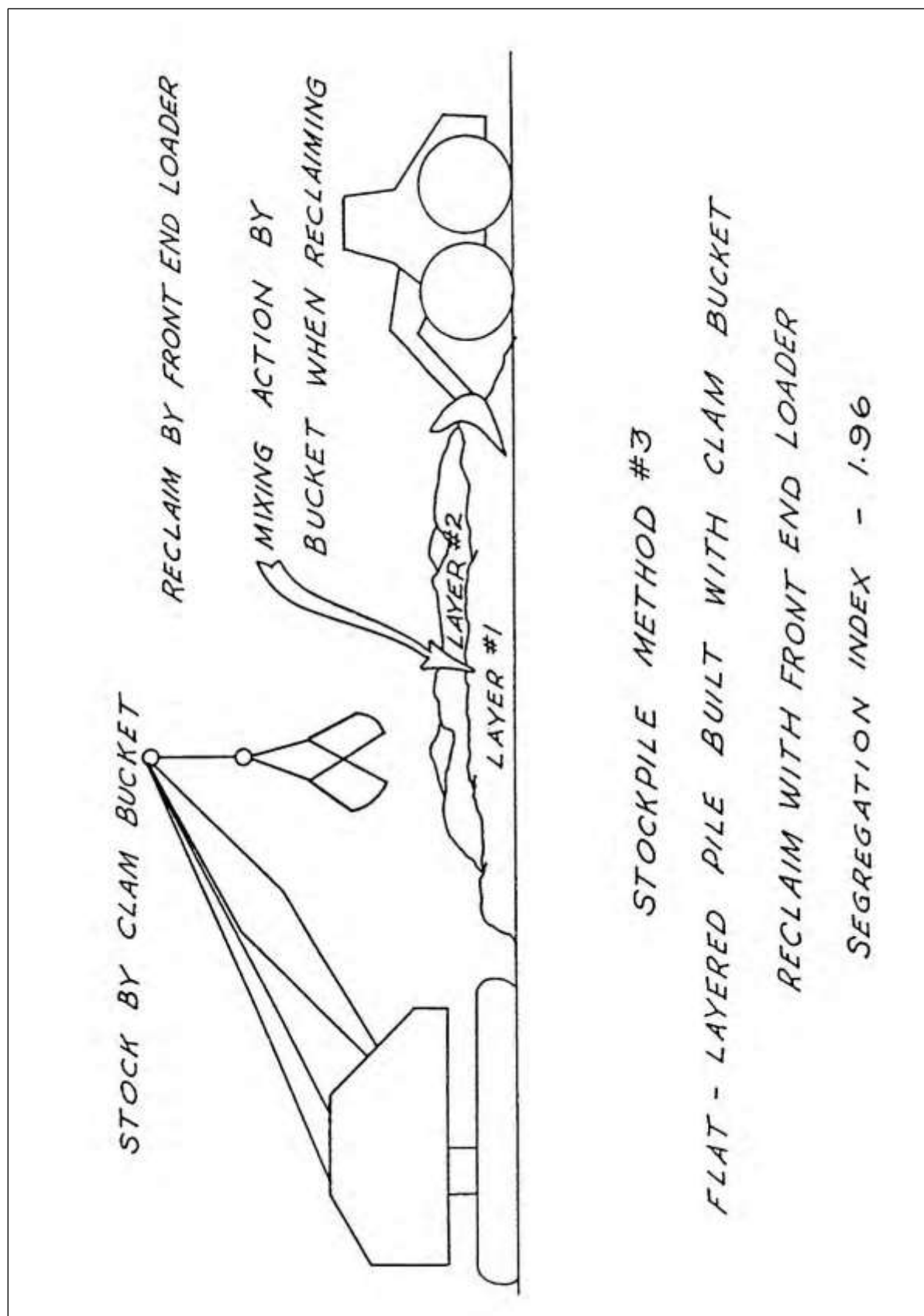
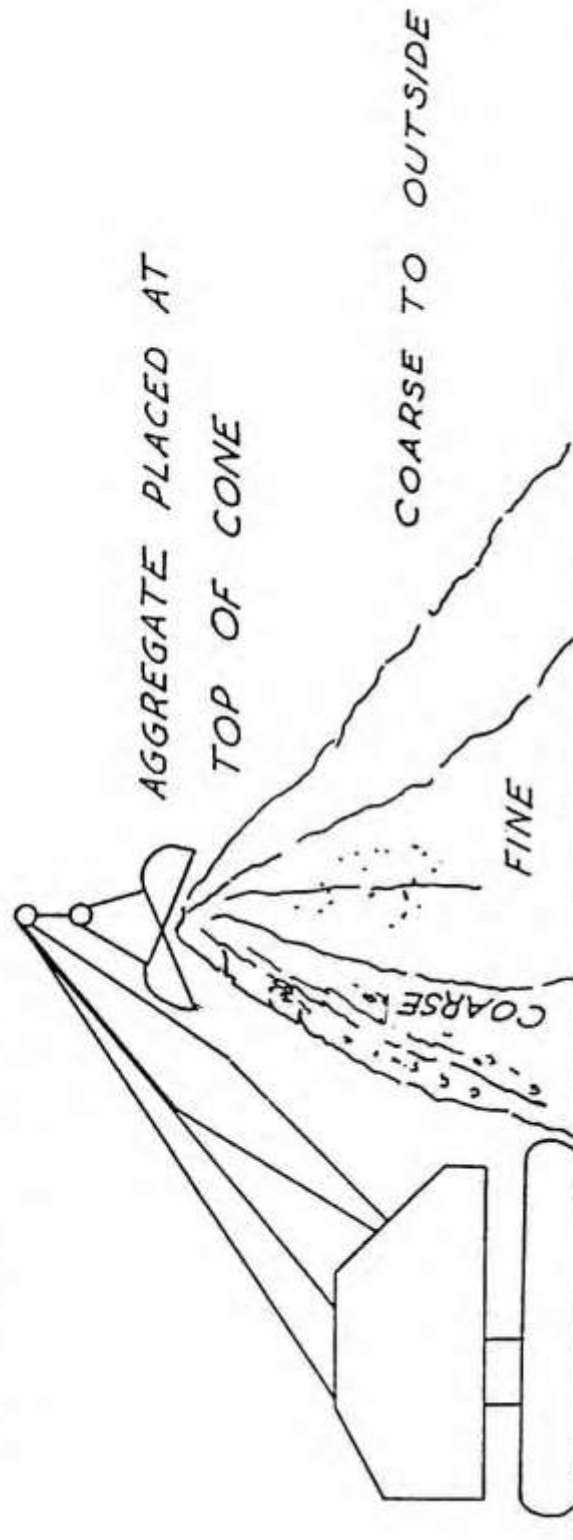


Figure 4.35

STORED AND RECLAIMED  
BY CLAM BUCKET



STOCKPILE METHOD #4

SINGLE CONE BUILT WITH CLAM BUCKET (APPROX 1500 TONS)

RECLAIMED BY CLAM BUCKET IN HORIZONTAL LAYERS

SEGREGATION INDEX - 16.86 (WORST METHOD)

Figure 4.36

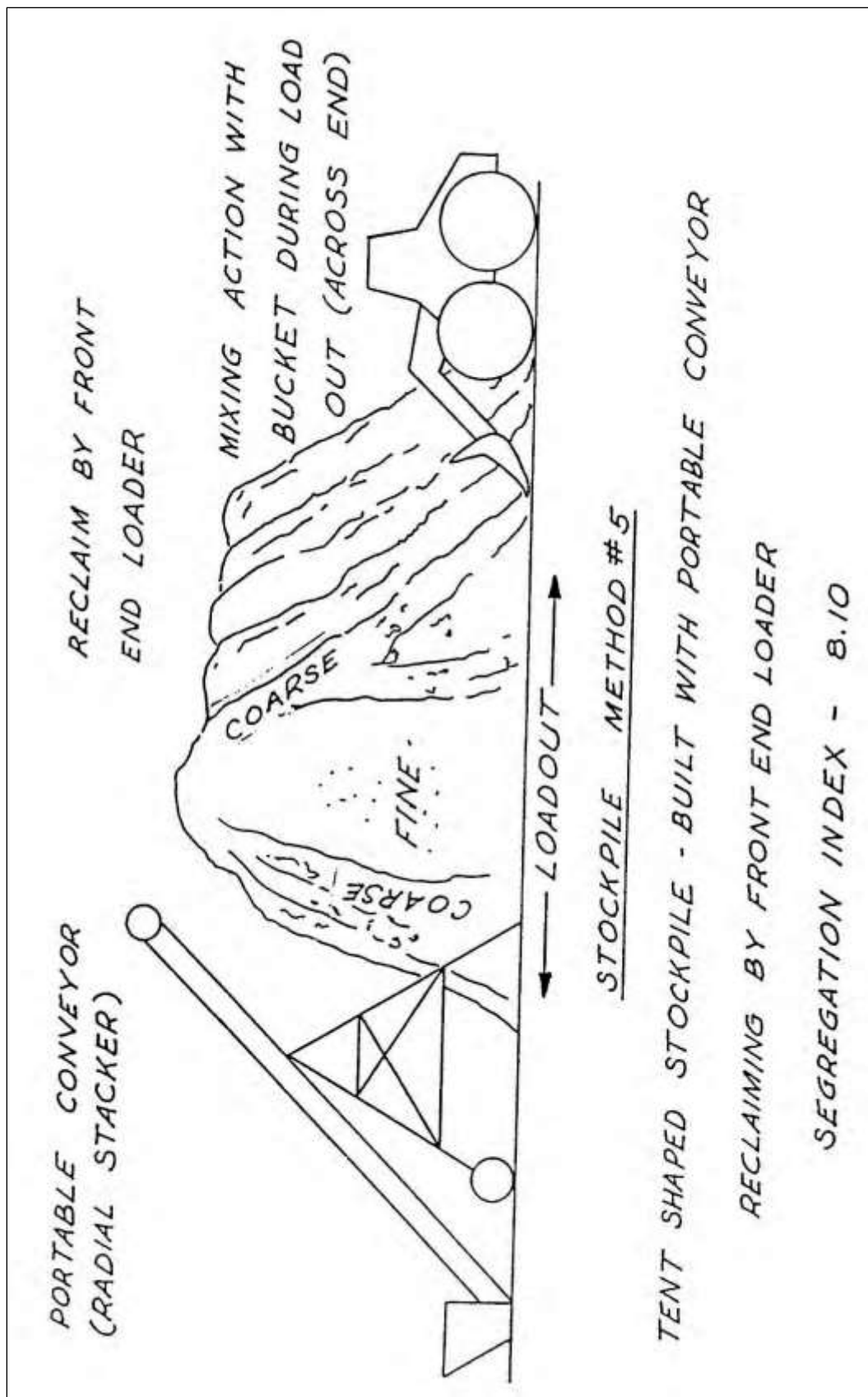
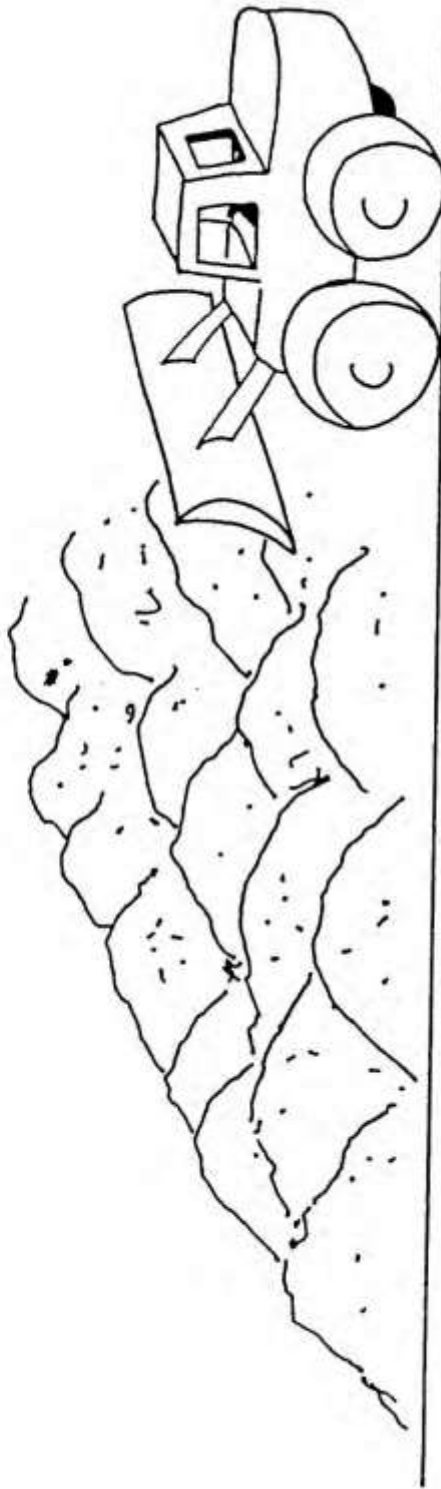


Figure 4.37

LIMITING FACTORS  
 HEIGHT OF LOADER BUCKET  
 STOCKED AND RECLAIMED  
 BY FRONT END LOADER  
 LARGE AREA REQUIRED



STOCKPILE METHOD #6  
 FLAT LAYERED CONSTRUCTED BY FRONT END LOADER  
 RECLAIMED BY FRONT END LOADER  
 SEGREGATION INDEX - 4.05

Figure 4.38

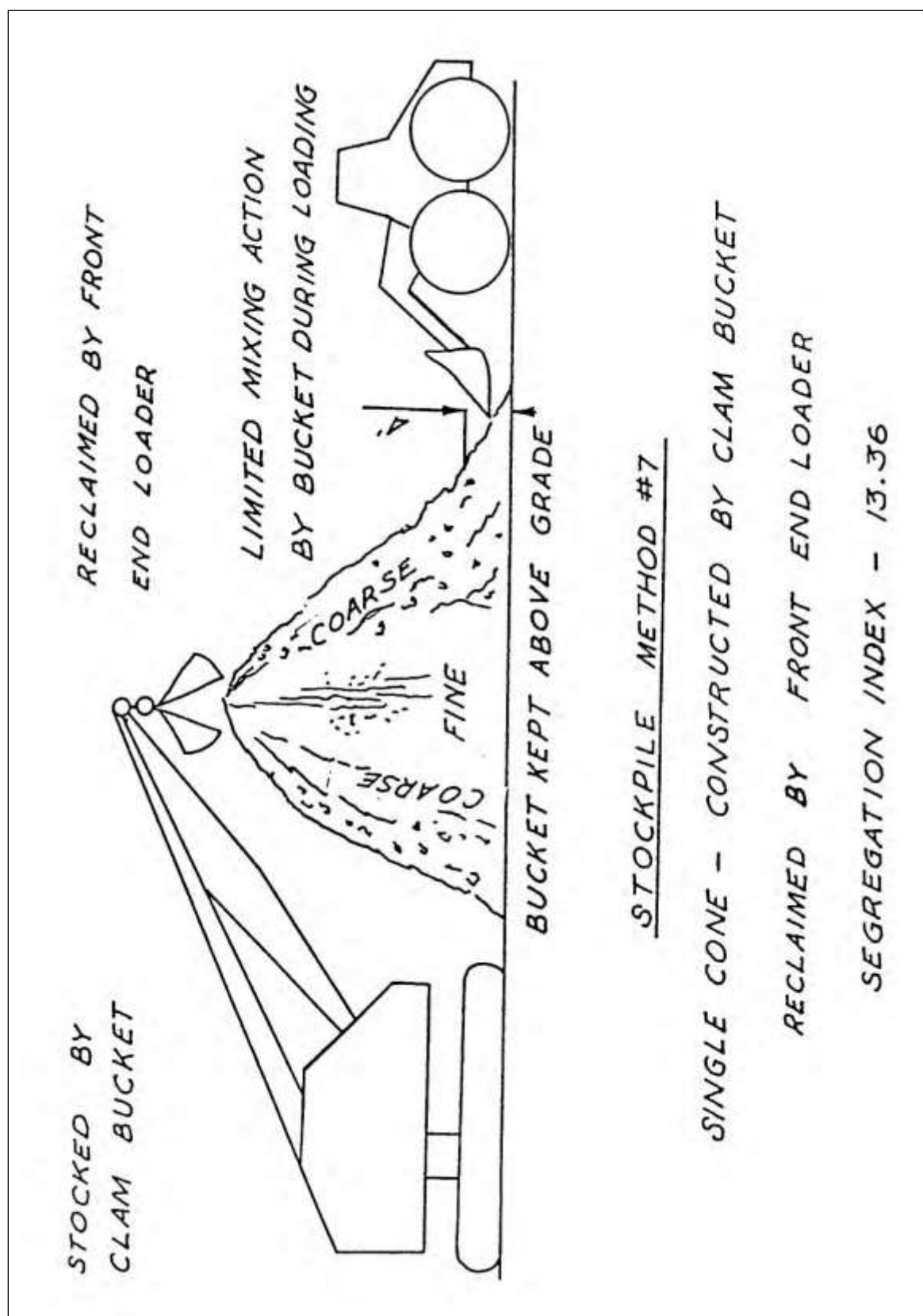
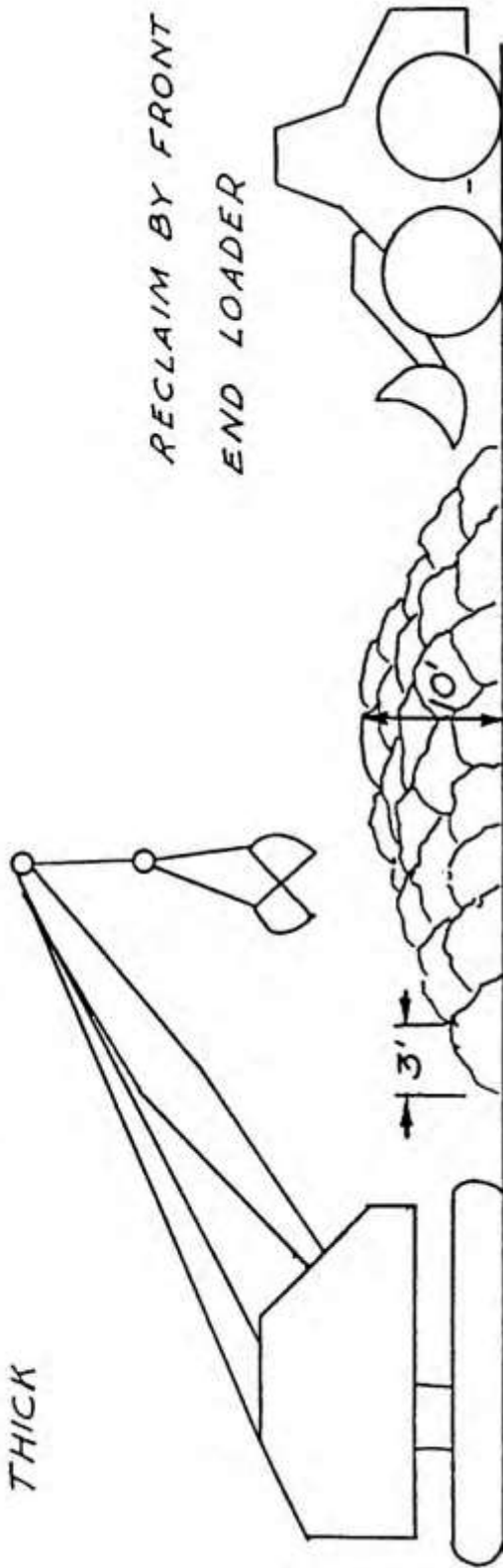


Figure 4.39

STOCK BY CLAM BUCKET  
EACH LAYER ONE BUCKET

THICK



RECLAIM BY FRONT  
END LOADER

METHOD VERY SLOW AND EXPENSIVE  
SET BACK OF 3 FT BETWEEN LAYERS

STOCKPILE METHOD #8

TIERED (BERMED) BY CLAM BUCKET

RECLAIMED BY FRONT END LOADER

SEGREGATION INDEX - 7.37

Figure 4.40

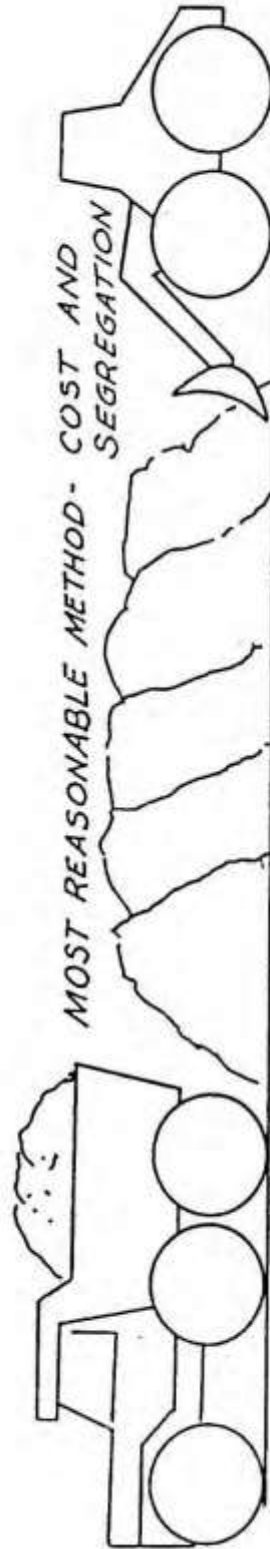


STOCKED BY TRUCK BACKED  
INTO SIDE OF PRECEDING PILE

RECLAIMED BY FRONT  
END LOADER

THIS METHOD CAN BE TIERED

MIXING ACTION BY  
BUCKET DURING LOADING



MOST REASONABLE METHOD - COST AND  
SEGREGATION

STOCKPILE METHOD #9

TRUCK DUMP SINGLE LOAD - BACKED AND DUMPED

RECLAIMED BY FRONT END LOADER

SEGREGATION INDEX 2.30

Figure 4.41

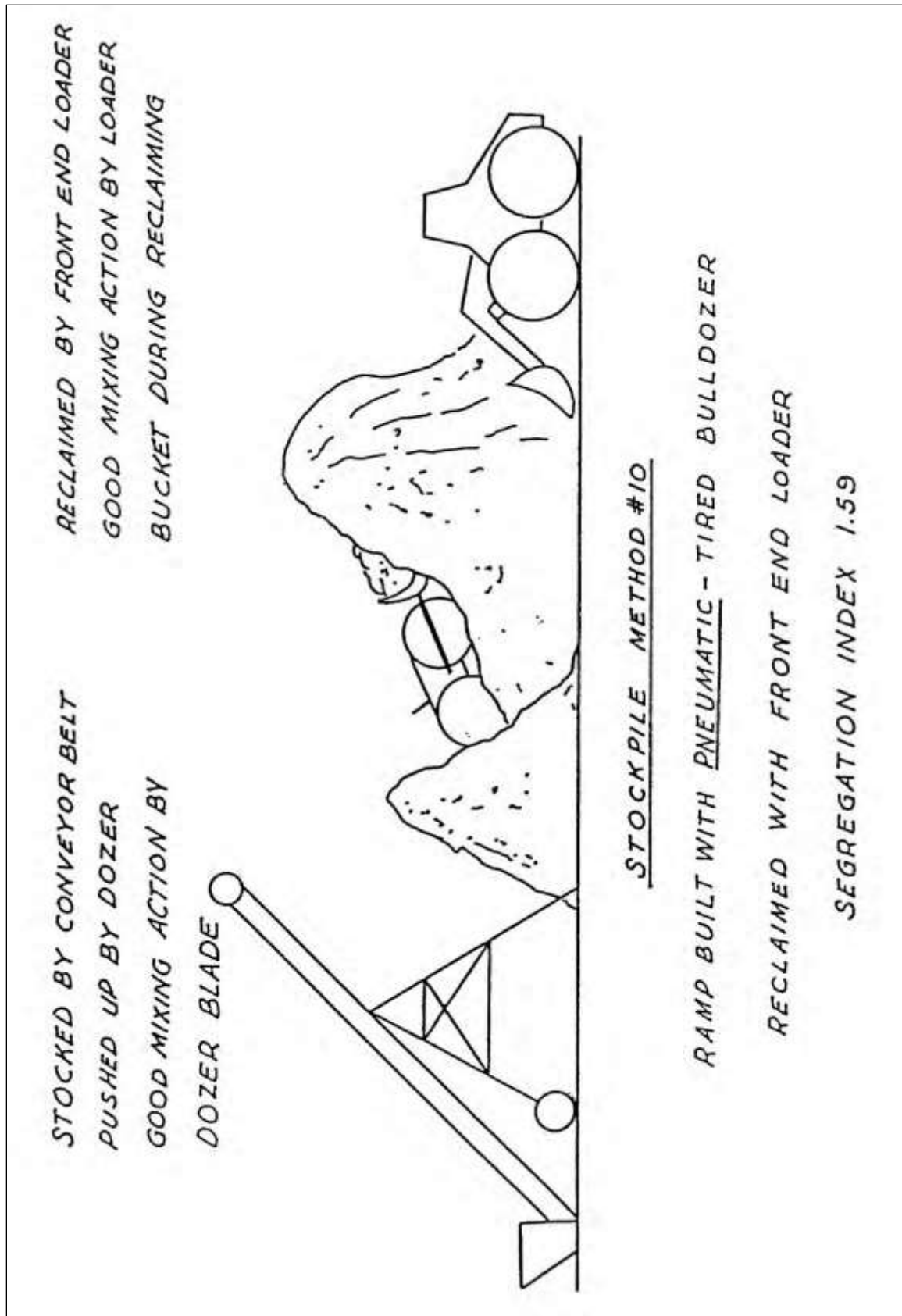
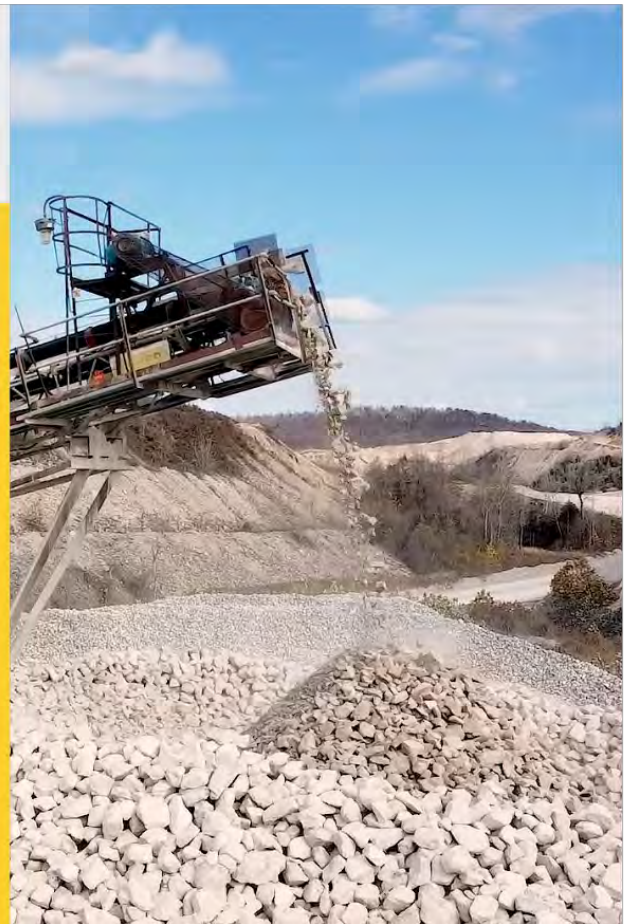


Figure 4.42

# LABORATORY OPERATION & MANAGEMENT

## CHAPTER 5



# LABORATORY OPERATION AND MANAGEMENT

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## **LABORATORY EQUIPMENT**

- General
- Sampling
- Sample Reduction
- Sieve Analysis
- Decantation
- Deleterious and Chert
- Test Equipment Verification

## **JOB RESPONSIBILITIES**

- Quality Control Plan
- Frequency of Sampling and Testing
- Diary Requirements
- Successful Lab Practices

# LABORATORY EQUIPMENT

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Before entering the Coordinated Testing Phase of the Certified Aggregate Producer Program, the Producer is required to have a suitable laboratory and testing equipment that has been verified. Laboratories are checked by an INDOT representative before start-up of the Coordinated Testing Phase and periodically to ensure compliance within the program. (ITM-211)

## EQUIPMENT

1. Electronic balance, Class G2, general purpose balance in accordance with **AASHTO M 231**. The balance is to be readable to 0.1 g and accurate to 0.2 g or 0.1 percent of the test load, whichever is greater, throughout the range of use
2. Laboratory oven, capable of maintaining a temperature of  $230 \pm 9^{\circ} \text{F}$ , and ample interior volume to handle the anticipated sample load
3. Metal pans for drying and storage
4. Utensils for washing and drying samples, such as trowels, spatulas, etc.
5. Appropriate data sheets, log books, etc.



Figure 5.1 Example Lab

## SAMPLING EQUIPMENT

Equipment for **AASHTO R 90 (ASTM D 75)** or **ITM 207**:

1. Square-nose shovel
2. Sampling tube or fire shovel for fine aggregate
3. Containers, such as 5-gallon buckets, plastic fiber bag, etc. Galvanized bushel tubs work well and will stand up to oven temperatures.
4. Labels of sufficient size to allow for proper identification of samples

## **SAMPLE REDUCTION EQUIPMENT**

Equipment for **AASHTO R 76**:

1. Mechanical splitters with proper fitting pans
2. 5-gallon buckets

## **SIEVE ANALYSIS EQUIPMENT**

Equipment for **AASHTO T 27**:

1. For coarse aggregates a 15 in x 23 in. or 14 in. x 14 in. sieves are recommended. The sieve sizes should be of appropriate sizes to accommodate the material being tested. Cutter sieves may be used to ensure overloading does not occur.
2. For fine aggregates; 8 in. or 12 in round sieves are recommended. The sieve sizes should be of appropriate sizes to accommodate the material being tested. Cutter sieves may be used to ensure overloading does not occur.
3. Sieve brushes (never use a wire brush on sieves smaller than the No. 50 sieve)

## **DECANTATION EQUIPMENT**

Equipment for **AASHTO T 11**:

1. Sieve sizes No. 8, No. 16, and No. 200. The No. 200 sieve is protected from punctures and tears by covering with the No. 8 or No. 16 sieve.
2. Container sufficient size to hold the sample covered with water and to permit vigorous agitation
3. Wetting agent, such as liquid detergent, etc. Some fine materials, especially limestone dust, require a wetting agent to break the surface tension of the particles. A drop or two of dishwashing liquid is usually sufficient.
4. Decant machine (optional) (may be used provided the results are consistent with those obtained using manual operations)

## DELETERIOUS AND CHERT EQUIPMENT

Equipment for deleterious and chert:

1. Scratch hardness tester – ITM-206 (Gravel only)
2. Hydrochloric acid and glass plate

## TEST EQUIPMENT VERIFICATION

The test equipment is required to be properly verified and maintained within the limits described in the applicable test method. Verification of the test equipment is to be prior to beginning testing in the Coordinated Testing Phase. The Producer is to verify the equipment at the minimum frequency as follows:

Equipment	Requirement	Min. Freq.	Procedure
Balances	Verification	12 mo.	ITM 910
Mechanical Shakers	Check sieving thoroughness	12 mo.	ITM 906
Sieves	Check physical thoroughness	12 mo.	ITM 902

**Figure 5.2 Test Equipment Minimum Frequency Chart**

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# JOB RESPONSIBILITIES

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All persons in the chain of command at the processing plant should be responsible for how they fit into the overall manufacturing process. As problems occur in the process, each individual in the system is responsible for performing a professional manner to ensure the final result is a quality product.

## QUALITY CONTROL PLAN

The Quality Control Plan (QCP) encompasses the total process from preliminary material quality approval through the point where the aggregate leaves the Producers' control. Before writing the QCP, consider the job responsibilities of Certified Aggregate Technician is not only related to sampling and testing, but also other responsibilities during the day-to-day operations. The activities below are examples of what may constitute a Certified Aggregate Technician's responsibilities.

- Meeting with management to receive production information
- Coordinating with personnel involved with the sampling process
- Sampling the material
- Performing tests on material (different sizes of material require varying amounts of time)
- Calculating test results and clarity
- Investigating issues that may arise from test results
- Checking the operation starting from the pit to when material leaves the property
- Completing diary
- Maintaining lab cleanliness

At minimum, each plant/redistribution terminal is required to establish sampling and testing frequency according to ITM 211. Each plan may determine an increased sampling and testing frequency may be needed and can adjust their schedule accordingly.

### **REMEMBER:**

*Day-to-day operations may be interrupted by unexpected occurrences, such as customer relations, special requests, writing reports, or working with INDOT personnel.*



## **FREQUENCY OF SAMPLING AND TESTING**

The most vital activity required by the CAPP is the sampling and testing of the aggregates. The term certified material is defined as a product produced under the CAP Program intended for INDOT use. A frequency is required to be established for each certified material in the Quality Control Plan. (ITM 211).

### **GRADATION (SECTION 11.3)**

The minimum frequencies of sampling and testing for gradation include three time periods: Start of Production, Normal Production, and Load-Out.

The minimum requirement for sampling and testing a certified material during Start of Production is:

1. One test per 1000 t for the first 5000 t produced
2. A maximum of two per calendar day

The minimum requirement for sampling and testing a certified material during Normal Production is:

1. One test per 2000 t
2. A maximum of two per calendar day

The minimum requirement for sampling and testing a certified material during Load-Out is:

1. One test per 8000 t shipped
2. A minimum of one test per month for any certified material shipped that exceeds 1000 t

### **DECANTATION (SECTION 11.4)**

All load-out samples are required to be decanted. Unless specific problems are encountered, start of production and normal production samples do not require a decant test.

### **CRUSHED PARTICLES (SECTION 11.5)**

The minimum requirement for determining the amount of crushed particles is one test per week for each size of material during start of production and normal production. No test is required if the week's production is less than 100 t.

## DELETERIOUS MATERIALS (SECTION 11.6)

The minimum requirement for determining the percentage of deleterious materials is one test per week for each size of material during the start of production and normal production. No test is required if the week's production is less than 100 t.

## DIARY REQUIREMENTS (ITM 211.10)

Each plant/redistribution terminal is required to maintain a diary. Test reports do not substitute for a diary. The diary is required to be an open-format book with at least one page devoted to each day that there is a material related operation. Entries into the diary are required to include:

1. General weather conditions
2. Area of extraction-location and ledges or pit area
3. Estimate quantity of materials produced
4. Time test samples obtained and tested, and corrective action if there were problem
5. Changes in key personnel, if any
6. Changes in equipment, plant, screens, etc. which may affect the current statistical results of aggregate materials
7. Any significant events or problems
8. Any nonconforming condition, as well as the action taken to correct the condition, if needed.

The diary entry is signed by the Certified Aggregate Technician or Management Representative. On occasion the diary may be signed by another person; however, the diary is required to then be counter-signed by the Certified Aggregate Technician or Management Representative.

## SUCCESSFUL LAB PRACTICES

Proper organization of the laboratory will maximize efficiency and minimize problems and erroneous results. The laboratory should be organized in the direction of a consistent flow. For example, the equipment may be arranged from left to right when conducting sieve analyses as follows:

1. Riffle splitter -- for reduction of incoming samples
2. Oven or other heat source -- for drying samples after reduction

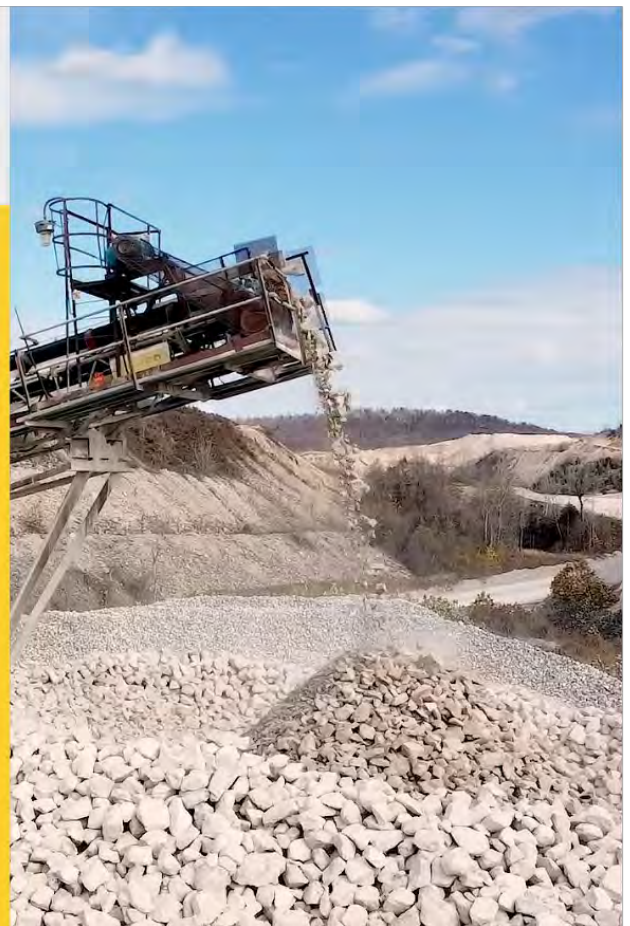
3. Cooling rack and fan -- (note: make sure that the fan does not blow towards the balance in the weighing area or disperse sample fines)
4. Coarse aggregate shaker
5. Fine aggregate shaker
6. Weighing area -- balance should be in an area free from vibration, dust, and air flow

**REMEMBER:**

*Every laboratory situation is different. Set up the lab to meet the flow requirements of routine tests. Minimize the need for back-tracking, especially if more than one Technician is working at a time. A little extra time and thought to the setup of the lab significantly increases productivity and decreases turn-around time of test results.*

# SAMPLING

## CHAPTER 6



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# SAMPLING

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## **SAMPLING PROTOCOL**

- Safety
- Sample References
- Size of Original Samples
- Sample Types

## **METHOD OF SAMPLING**

- Production Sampling
- Sampling Directly from Trucks, Rail Cars, or Barges
- Load-Out Sampling

## **REDUCING A SAMPLE TO TEST SIZE**

- Mechanical Splitter
- Sand Splitter
- Miniature Stockpile
- Quartering

## **SIZE OF TEST SAMPLE (AFTER SPLITTING)**

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# SAMPLING PROTOCOL

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Sampling is perhaps the most important step in assuring that good quality aggregates are being used on INDOT contracts. Since a sample is just a small portion of the total material, the importance that the sample be representative of the material being delivered cannot be overemphasized. Any test conducted on the sample, regardless of how carefully and accurately done, is worthless unless the sample is truly representative of the material used on the contract.

## **SAFETY**

The sampling of materials may expose the Technician to machinery, moving belts, large stockpiles, and other potential dangers. Proper safety practices are always the first concern. When an unsafe condition exists, instructions from the Supervisor on the safety procedures for sampling are required to be obtained.

## **SAMPLE REFERENCES**

### **(AASHTO R 90 and ITM 207)**

A representative sample may be obtained by following the standard procedures detailed in AASHTO R 90, or ITM 207, Method of Sampling Stockpile Aggregate. Any deviations from the aforementioned procedures will require a detailed description within the QCP (ITM 211 Section 14.2.12).

## **SIZES OF ORIGINAL SAMPLES**

The key to any sample program is to obtain a representative sample. A standard sampling method is required to be followed to obtain uniform samples. The following is a list of recommended minimum sizes of composite samples to be used as a guide when collecting samples.

MATERIAL	SAMPLE SIZE
No. 2 coarse aggregate	220 lb or 100100 g
No. 5 coarse aggregate	110 lb or 50050 g
No. 8 coarse aggregate	55 lb or 25025 g
No. 9 coarse aggregate	35 lb or 15925 g
No. 11, No. 12 & No. 16 coarse aggregate	25 lb 11375 g
No. 43 coarse aggregate	110 lb or 50050 g
No. 53 coarse aggregate	135 lb or 61425 g
No. 73 coarse aggregate	80 lb or 36400 g
2 in. Structure Backfill	245 lb 111475 g
1½ in. Structure Backfill	190 lb 86450 g
1 in. Structure Backfill	135 lb 61425 g
½ in. Structure Backfill	60 lb 27300 g
All fine aggregate	25 lb or 11375 g

**Figure 6.1 Sample Size Chart**

The weight of the sample depends on the maximum particle size of the material being inspected. As a rule, a larger top size material requires a larger sample. A 11375 g sample of No. 2 coarse aggregate would not be as representative of that material as a 11375 g sample of natural sand.

**REMBEMBER:**

*Top Size or Maximum Particle Size -- The sieve on which 100 percent of the material passes.*

*Nominal Maximum Particle Size -- Smallest sieve opening through which the entire amount of the aggregate is permitted to pass.*

*Although these two definitions are almost identical, the difference is important. An INDOT 53 aggregate, for example, is required to have 100 % of the material passing the 1 1/2 in. sieve. The*

*next smallest sieve by Specification is the 1 in. sieve which requires 80-100 % of the material to pass the sieve. The maximum particle size therefore is 1 1/2 in. since 100 % of the aggregate is required to pass the 1 1/2 in. sieve. The nominal maximum particle size is 1 in. since the 1 in. sieve is the smallest sieve which is permitted to have 100 % of the material pass but is not required.*

## **SAMPLE TYPES**

The Technician should recognize that there are different types of samples. The most common samples are the stockpile (load-out) and production samples which are normally the method by which material is tested using CAP Program (in accordance with ITM 207 or other approved manner). The gradation of the production sample may not be the same as a load-out sample at some facilities.

Occasionally, an investigative sample is obtained when verifying a specific feature, such as a certain sieve, oversized material, etc. These tests may consist of many shortcuts and are only used as a quick comfort level check.

Every source may have other types of samples which are unique to their operation.

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# METHOD OF SAMPLING

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Because of the various sampling locations and the availability of equipment, there are several methods of taking aggregate samples. Uniformity of obtaining the sample is very important, since the sampling procedure eliminates one variable in the test results. The Technician should remember that safety comes first.

## **PRODUCTION SAMPLING**

### **BIN SAMPLING**

Sampling the top of the bin is an extremely dangerous as well as a difficult method to obtain a representative sample. For this reason, this method of sampling is undesirable.



## BELT SAMPLING

Belt sampling consists of taking samples of materials directly from the conveyor belts. This may be done by a mechanical sampling device (Figure 6.2) or manually. The proper procedure for manual belt sampling is designated in AASHTO R 90 and includes the following:

1. Make sure that the belt is carrying a normal load of material that is not segregated.
2. Have the plant operator stop the belt and use proper lock out procedures.
3. Take a complete cross section of the material, being careful to include all the material on the belt and only the material in the section. A template is recommended, especially on steeply inclined belts. Remove most of the sample with a scoop or shovel and the remainder with a brush.
4. Take as many complete cross sections as necessary to obtain a sample that meets the minimum sample size.



**Figure 6.2 Mechanical Sampling Device**

## DISCHARGE SAMPLING OF BINS AND BELTS

Bin samples may be taken at the discharge chute. In these situations, a number of small samples are taken at short intervals and combined to make the total sample. Each sample is required to include the entire cross section of the flow of material from the chute or belt.

Continuity of operation normally does not allow the Technician to control the rate of flow from the discharge chute. A mechanical diversion or slide chute system is the quickest, safest, and most accurate system (Figure 6.3). Unfortunately, very few mechanical systems exist. All methods, including manual methods, are required to be included in the Quality Control Plan for the source and the proper safety practices should be designated.



**Figure 6.3 Slide Chute**

## **SAMPLING DIRECTLY FROM TRUCKS, RAILCARS, OR BARGES**

Direct sampling from trucks, rail cars, or barges is not recommended. There are a number of factors that may influence the gradation of the material, such as segregation or particle breakdown during loading, transporting, and unloading. Therefore, material being shipped by cars or barges should be sampled at the point of delivery. Materials being shipped by trucks for local delivery points also should be sampled at the point of delivery.

## **LOAD-OUT SAMPLING**

### **COARSE AND FINE AGGREGATE STOCKPILES**

Coarse and fine aggregate samples are recommended to be sampled in accordance with ITM 207, however a fire shovel or sampling tube is used to collect the fine aggregate sample.

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# REDUCING A SAMPLE SIZE TO TEST SIZE

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The total sample (production or load-out) is required to be reduced to a sample size that may be quickly tested. The procedure is conducted in accordance with AASHTO R 76. The key to sample reduction is to ensure the sample remains representative of the material in the stockpile. This practice is commonly referred to as splitting a sample. There are four different methods to reduce a sample to the proper test size.

1. **The mechanical splitter** is the most accepted method of reducing to test size all coarse aggregate material smaller than No. 2 aggregate, except highly moistened compacted aggregate. To determine if a compacted aggregate sample is too wet, a small portion of the sample is tightly squeezed in the palm of the hand. If the small sample crumbles readily, the correct moisture range has been obtained.
2. **The sand splitter** is the accepted method of reducing fine aggregate or the minus No. 4 material from compacted aggregate samples that is drier than the saturated surface-dry condition. As a quick check to determine this condition, if the material retains the shape when molded in the hand, the material is considered wetter than saturated surface-dry.
3. **The miniature stockpile** is the method used for fine aggregate that has free moisture on the particle surfaces.
4. **Quartering** is the method that is used for highly moistened compacted aggregate or when a mechanical splitter is not available.

## MECHANICAL SPLITTER

The mechanical splitter separates the sample into equal halves (within 10% of each other) as the material passes through the spaces between the bars in the splitter.

In using the mechanical splitter (Figure 6.4), the splitter bars are adjusted so that the bar opening is approximately twice the maximum particle size of the material to be split. A No. 5 aggregate has a maximum particle size of 1½ in. Therefore, the recommended bar

opening is approximately 3 in. INDOT allows the bar opening to be 3 in. or 6 bars for all coarse aggregate No. 5 or smaller. Some models may not have adjustable chutes and workable hopper doors. The splitter is required to be level to ensure that each half of the split is approximately the same size; within approximately 10 percent of each other by weight.

The splitting procedure is as follows:

1. Properly place the pans under the splitter in such a way that all of the particles diverting in both directions will be caught.
2. Pour the sample evenly into the hopper.
3. Material is distributed evenly edge to edge.
4. Open the hopper fully and allow the material to free fall through the splitter.
5. If wet particles stick inside the splitter, gently tap the splitter with a rubber hammer to loosen them.
6. To ensure the sample has not segregated during sampling, place both halves of the sample back into the hopper and repeat the splitting operation.
7. After the second splitting, the two receiving pans contain approximately the same amount of material. Only one pan is placed back into the hopper and the splitting procedure repeated until a sample of the desired weight is obtained. Skillful manipulation of the splitter allows a sample of nearly any size to be made that is still representative of the material in the stockpile.



**Figure 6.4 Mechanical Splitter**

## SAND SPLITTER

The sand splitter (Figure 6.5) is a small version of the mechanical splitter. Some models may not have adjustable chutes and workable hopper doors.

The splitting procedure is as follows:

1. Properly place the pans under the splitter in such a way that all of the particles diverting in both directions will be caught.
2. Pour the sample evenly into the hopper.
3. Material is distributed evenly edge to edge.
4. Open the hopper fully and allow the material to free fall through the splitter.
5. To ensure the sample has not segregated during sampling, place both halves of the sample back into the hopper and repeat the splitting operation.
6. After the second splitting, the two receiving pans contain approximately the same amount of material. Only one pan is placed back into the hopper and the splitting procedure repeated until a sample of the desired weight is obtained. Skillful manipulation of the splitter allows a sample of nearly any size to be made that is still representative of the material in the stockpile.



**Figure 6.5 Sand Splitter**

## MINIATURE STOCKPILE



**Figure 6.6 Miniature Stockpile**

The miniature stockpile method is used for reducing all samples of fine aggregates when the material is in a damp or moist condition. If the sample to be split is dry, then the material should be moistened before using this method.

The splitting procedure is as follows:

1. Place the original sample on a clean, dry plate or other hard, smooth, non-absorptive surface.
2. Using a trowel or other suitable tool, turn the entire sample over three times i.e. starting at the 3 o'clock position turn the sample over at 3 o'clock, 6 o'clock, 9 o'clock, 12 o'clock then back to 3 a total of three times.
3. Shape the material into a conical pile.
4. With a straight edge putty knife or small trowel, randomly take at least five small portions of material around the pile and one-third way up the cone until the required test sample is obtained.

## QUARTERING

Quartering is a non-mechanical method of reducing a sample. This is the best method of reducing highly moistened compacted aggregate or when a mechanical splitter is not available.

The quartering procedure is as follows:

1. Place the sample on a hard, clean, level, non-absorptive surface where there will be neither loss of material nor the accidental addition of material.
2. Using a large trowel, shovel, or other suitable tool turn the entire sample over at least three times. Form the sample into a conical pile by depositing individual lifts on top of the preceding lift.
3. Flatten the pile to a uniform thickness by pressing down the apex with a shovel or trowel. Each quarter sector of the resulting pile is required to contain the material originally in the pile. The diameter of the pile should be equal to 4-8 times the thickness of the pile.
4. With a large trowel or other tool, divide the sample into four equal quarters. Remove two diagonally opposite quarters, including all fine material, and brush the cleared spaces clean.
5. Combine diagonally opposite quarters of the material into two samples. All fine materials shall be included by brushing the surface clean. Store one of these two halves. If the



**Figure 6.7 Quartering**



remaining material still weighs too much, repeat the entire quartering process until the proper test sample size is obtained.

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## SIZE OF TEST SAMPLE (AFTER SPLITTING)

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The original sample is required to be reduced to a test sample size that is within the minimum and maximum weights of the following table (weights are determined after sample is dry).

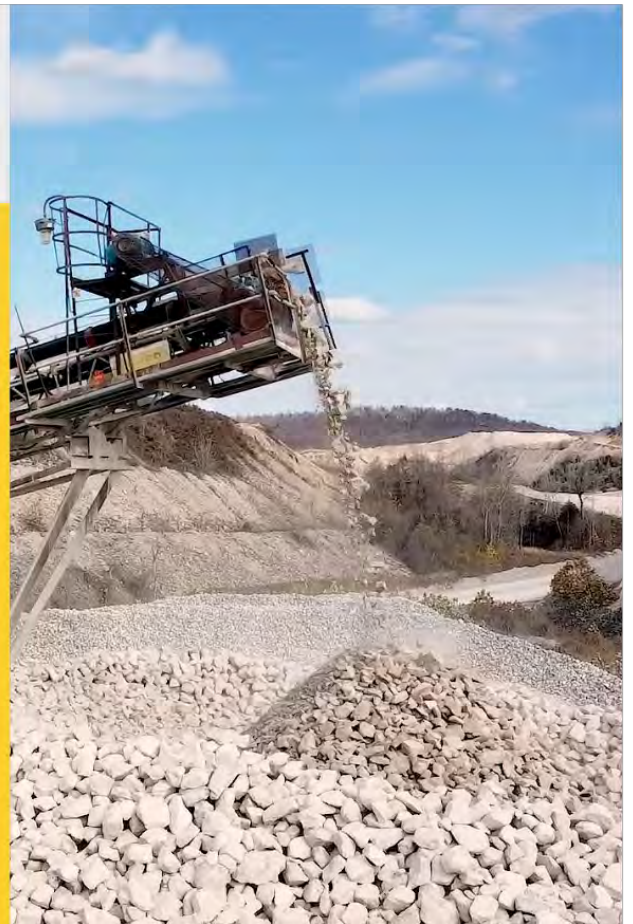
Aggregate Size	Minimum	Maximum
No. 2	24.8 lbs or 11,300 g	
No. 5	13.2 lb or 6000 g	17.6 lb or 8000 g
No. 8	13.2 lb or 6000 g	17.6 lb or 8000 g
No. 9	8.8 lb or 4000 g	13.2 lb or 6000 g
No. 11	4.4 lb or 2000 g	
No. 12	2.2 lb or 1000 g	
No. 16	2.2 lb or 1000 g	
No. 43	13.2 lb or 6000 g	17.6 lb or 8000 g
No. 53	13.2 lb or 6000 g	17.6 lb or 8000 g
No. 73	13.2 lb or 6000 g	17.6 lb or 8000 g
No. 91	13.2 lb or 6000 g	17.6 lb or 8000 g
B Borrow	8.8 lb or 4000 g	13.2 lb or 6000 g
Structure Backfill, 2 in.	24.8 lb or 11,300 g	
Structure Backfill, 1 ½ in. and 1 in.	13.2 lb or 6000 g	17.6 lb or 8000 g
Structure Backfill, ½ in.	8.8 lb or 4000 g	13.2 lb or 6000 g
Structure Backfill, No. 4 and No. 30	0.7 lb or 300 g	
Fine Aggregate	0.7 lb or 300 g	

**Figure 6.8 Test Sample Size**

# INTRODUCTION TO CAPP MATH

CHAPTER 7  
Introduction to CAPP Math

## CHAPTER 7





# INTRODUCTION TO CAPP MATH

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## **MATHEMATICAL CONCEPTS**

Rounding

The Mean

Five Point Moving Average

Standard Deviation

# MATHEMATICAL CONCEPTS

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## ROUNDING

When calculations are conducted, rounding is required to be in accordance with Indiana Specification 109.01(a) using the standard "5" up procedure. There are two rules for rounding numbers:

1. When the first digit discarded is less than 5, the last digit retained should not be changed.

Examples:      2.4 becomes 2

                    2.43 becomes 2.4

                    2.434 becomes 2.43

                    2.4341 becomes 2.434

2. When the first digit discarded is 5 or greater, the last digit retained should be increased by one unit.

Examples:      2.6 becomes 3

                    2.56 becomes 2.6

                    2.416 becomes 2.42

                    2.4157 becomes 2.416

The Certified Aggregate Producer Program requires that test and statistical values be calculated to the nearest decimal place as indicated in Figure 7.1.

Property	Nearest Whole Unit (0)	First Decimal Place (0.0)	Second Decimal Place (0.00)	Third Decimal Place (0.000)
Crushed Particles	X			
Flat & Elongated	X			
Percent Compliance	X			
Control Limits*	X	X		
Absorption		X		
Decantation		X		
Deleterious		X		
Gradation		X		
Surface Moisture		X		
Target Mean		X		
5-Point Moving Ave.		X		
Fineness Modulus			X	
Standard Deviation			X	
Z Value			X	
Bulk Specific Gravity				X
Proportionate Factor				X

\* May be rounded to (0.0) or (0)

**Figure 7.1 Decimal Places**

## THE MEAN

The simple mathematical average of any group of numbers is the mean. In other words, the mean is the sum of all the measurement values divided by the number of measurements. The symbol for the mean is  $\bar{x}$ . As an example, the mean for five numbers would be calculated as follows:

$$\bar{x} = \frac{x_1 + x_2 + x_3 + x_4 + x_5}{5}$$

## FIVE-POINT MOVING AVERAGE

The moving average is a useful tool for tracking trends of the mean. The CAPP requires that the moving average be the average of the most recent five production data points.

For a moving average of five test values, the group of the first five measurements is averaged. When an additional test value is obtained, the first value is dropped, the sixth value is added, and the new group averaged. When a seventh value is obtained, the second value is dropped, and the new group averaged, and so on. An example of this procedure is as follows:

Data Points
4.8
5.3
5.0
4.7
5.1
5.5
4.6

$$\text{First Average} = \frac{4.8 + 5.3 + 5.0 + 4.7 + 5.1}{5} = \frac{24.9}{5} = 5.0$$

The first number, or 4.8, is dropped and the sixth value, or 5.5, is added and the second average is:

$$\text{Second Average} = \frac{5.3 + 5.0 + 4.7 + 5.1 + 5.5}{5} = \frac{25.6}{5} = 5.1$$

Next, the 5.3 is dropped and 4.6 is added:

$$\text{Third Average} = \frac{5.0 + 4.7 + 5.1 + 5.5 + 4.6}{5} = \frac{24.9}{5} = 5.0$$

## STANDARD DEVIATION

Whereas the mean is the average of all the data values, the standard deviation is an average value of the dispersion of the data from the mean. Standard deviation is usually signified by a small s of the Greek letter Sigma ( $\sigma$ ). For the CAP Program,  $\sigma_{(n-1)}$  is used.  $\sigma_{(n-1)}$  represents the *Sample Standard Deviation*:

$$\sigma_{(n-1)} = \sqrt{\frac{\sum (x_i - \bar{x})^2}{n - 1}}$$

Where:

$\sigma_{(n-1)}$  = standard deviation of the sample

$x_i$  = individual sample data values

$\bar{x}$  = sample mean (or average)

$n$  = the number of values in the sample

Note that squaring the deviations for the mean removes the negative signs. Dividing by  $(n - 1)$  gives us approximately an average squared deviation. Taking the square root puts the result back into the same units as the original values.

**Example:**

$i$	$x_i$	$(x_i - \bar{x})^{**}$	$(x_i - \bar{x})^2$
1	14.3	1.7	2.89
2	11.2	1.4	1.96
3	14.1	1.5	2.25
4	12.6	0.0	0.00
5	12.9	0.3	0.09
6	12.7	0.1	0.01
7	13.2	0.6	0.36
8	11.4	1.2	1.44
9	12.3	0.3	0.09
10	11.6	1.0	1.0
$\Sigma$	126.3		10.09

*\*\*Round these values to 0.0 before squaring them!*

$$n = 10$$

$$\bar{x} = \frac{\Sigma x_i}{n} = \frac{126.3}{10} = 12.6$$

$$\bar{x} = 12.6$$

$$\sigma_{(n-1)} = \sqrt{\frac{\Sigma(x_i - \bar{x})^2}{n - 1}}$$

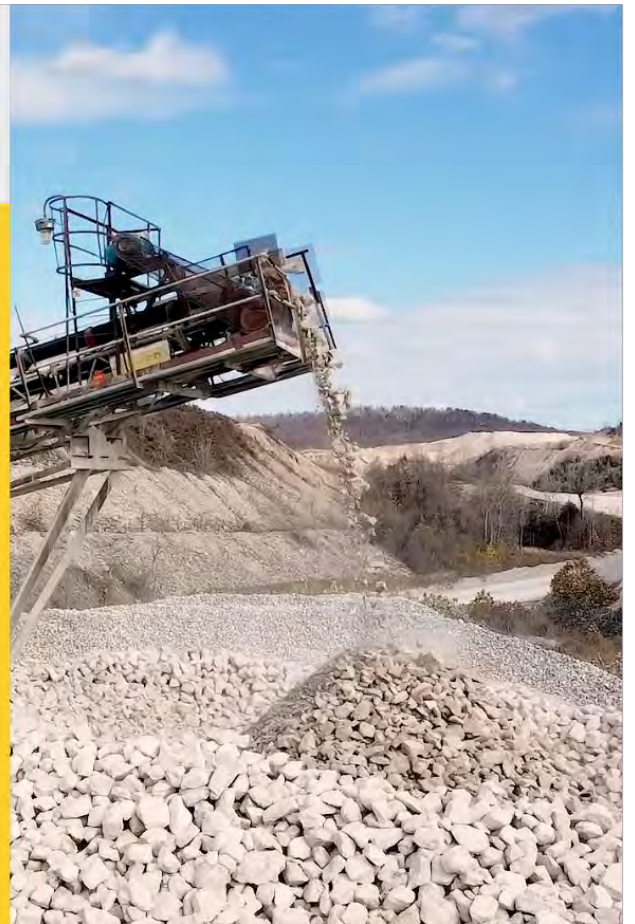
$$\sigma_{(n-1)} = \sqrt{\frac{10.09}{9}} = \sqrt{1.12111} = 1.05882 = 1.06$$

$$\sigma_{(n-1)} = 1.06$$

**Figure 7.2 Standard Deviation**

# TESTING

## CHAPTER 8



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# TESTING

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## **GRADATION**

Total Moisture Content

Sieving

Decantation

Total Moisture Content

Sieve Analysis Test

Fineness Modulus

Sieve Analysis for Dense Graded (Long Graded) Materials

## **DELETERIOUS MATERIALS**

Deleterious Materials in Coarse Materials

Deleterious Materials in Natural Sands

## **CRUSHED PARTICLES**

## **FLAT AND ELONGATED PARTICLES**

## **PLASTIC LIMIT**

## **TABLE OF FORMULAS**



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# GRADATION

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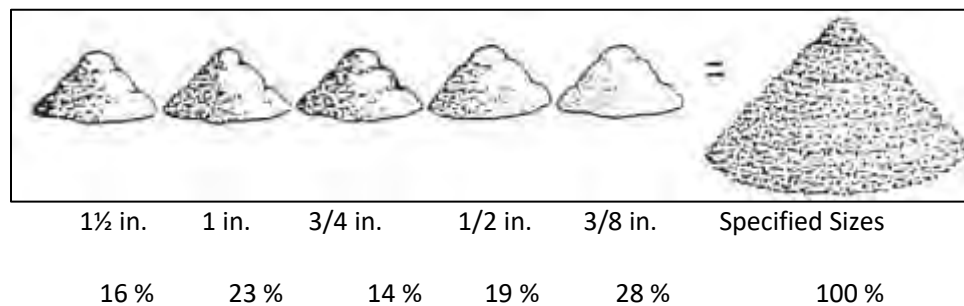
Gradation is the range and relative distribution of particle sizes in the aggregate material.

Range refers to the size limits of an aggregate set and to the number of sizes in that set. For example, the sizes in a set may extend from 1½ in. aggregates to 3/8 in. aggregates and include sizes of 1 in., 3/4 in. and 1/2 in. Another set may extend from 2½ in. aggregates to 1/2 in. aggregates with intermediate sizes of 1½ in., 1 in., and 3/4 in.

The relative distribution refers to the percentage of each particle size in the total material. For example, in a given set of aggregates, 16 percent of the total material could be 1½ in. aggregates, 23 percent could be 1 in. aggregates, 14 percent could be 3/4 in. aggregates and so on.

Sets of graded aggregates are referred to by size number with each having a specified range and relative distribution.

The sizes of fine and coarse aggregates used by INDOT and the gradation requirements for each size are found in Standard Specification **Section 904**.



**Figure 8.1 Sizes of Fine and Coarse Aggregates**

## TOTAL MOISTURE CONTENT

When aggregates are used in portland cement concrete mixtures, the moisture of the aggregates is required to be determined to adjust aggregate weights for moisture content and to determine the moisture contribution to the total mix water.

When a moisture content is desired, the sample is required to be reduced to test size and the test is conducted as quickly as possible after the sample has been obtained. Any delay in

conducting the test after the sample has been selected may allow the material to lose moisture and result in inaccurate results.

The test is conducted in accordance with **AASHTO T 255** as follows:

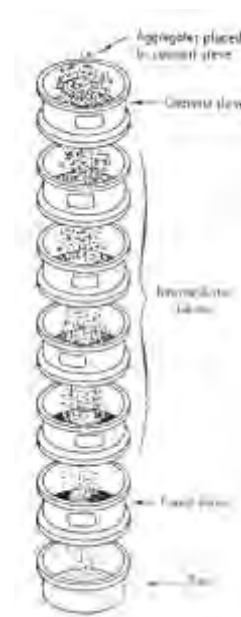
1. Weigh the sample before drying and record the weight.
2. Dry the sample and cool to room temperature.
3. Weigh the sample and record the weight.
4. Determine the moisture percent using this formula:

$$\% \text{ Moisture} = \frac{\text{Weight Wet} - \text{Weight Dry}}{\text{Weight Dry}} \times 100$$

## SIEVING

Gradation is determined by sieving. A sample of the aggregate material being tested is weighed to a constant weight in accordance with **AASHTO T 255** and then passed through a series of sieves (Figure 8.2). Sieve sizes correspond to the size of the openings in the mesh. Range is determined by the number and sizes of sieves used. Relative distribution is calculated by weighing the aggregates retained on each sieve.

The coarser sieves are classified according to the size of the opening in linear inches. Thus the 1 in. sieve has opening 1 in. square. Thus, the 1 in. sieve has openings 1 in. square. Finer sieves, below No. 4, are classified as 4 openings per linear inch.



**Figure 8.2 Sieves**

Aggregates coarser than the 1 in. sieve are called plus 1 in. material. Aggregates finer than the 1 in. sieve are called minus 1 in. material. Plus (retained) means coarser than and minus (passing) means finer than. To be retained on any sieve, the aggregates are required to be coarser in every direction than the sieve size.

## DECANTATION

The decantation test (Figure 8.3) determines the amount of material finer than the No. 200 sieve. The test is conducted on both fine and coarse aggregate and is usually done in conjunction with the sieve analysis test. The test is conducted according to **AASHTO T 11**, with exceptions noted in Standard Specification **Section 904.06**.

If the total amount passing the No. 200 sieve is required to be determined by the Specifications, the amount is determined by a combination of wet and dry sieving and is represented by the total amount passing the No. 200 sieve following both decantation and dry sieve analysis.



**Figure 8.3 Decantation**

The procedure for decantation is:

1. After the sample has been reduced to the proper size, the sample is thoroughly dried and allowed to cool to room temperature. The weight is recorded on a sieve analysis sheet.
2. The dried material is then placed in a container large enough to hold the sample with adequate wash water and room for agitating the sample.
3. The sample is covered with water.
4. The sample is agitated with a spoon or trowel to separate all particles and to suspend the minus No. 200 material.

5. The wash water is immediately poured or allowed to overflow through a No. 200 sieve. A protector sieve (No. 16) is nested above the No. 200 sieve for protection from the larger particles. Only the wash water (not the sample) is poured on the sieves.
6. The washing and sieving of the wash water is continued until the water runs clear.
7. After the wash water has cleared, the excess water is drained from the sample through the No. 200 sieve. Any residue material is removed from the protector sieve and the No. 200 sieve and placed with the test sample.
8. The washed sample is dried, allowed to cool to room temperature, and weighed. The weight is recorded in the decant section of the gradation analysis sheet.
9. The percentage of material finer than a No. 200 sieve is calculated by using the formula:

$$\% \text{ Decant} = \frac{\text{Original Dry Weight} - \text{Dry Weight after Decant}}{\text{Original Dry Weight}} \times 100$$

**Example:**

$$\% \text{ Decant} = \frac{6917.7 - 6811.3}{6917.7} \times 100 = 0.015 = 1.5\%$$

Decant	Original	Final	Grams Loss	% Loss	% Req.
	6917.7	6811.3	106.4	1.5	

**Figure 8.4 Decantation Chart**

## SIEVE ANALYSIS TEST

Sieve analysis is used primarily to determine the particle-size distribution or gradation of materials. The results are used to determine compliance with the applicable Specification requirements. The test is conducted on both the fine and coarse aggregates and is usually done in conjunction with the decantation test.

The sieve analysis for mineral filler is conducted in accordance with **AASHTO T 37**. Because of the very fine particle-size of mineral filler, this test requires washing the material over the required sieves. The sieve analysis for all other fine aggregates and all coarse aggregates is conducted in accordance with **AASHTO T 27**. Exceptions to **AASHTO T 37** and **AASHTO T 27** are listed in Standard Specification **Section 904.06**.

The procedure for the sieve analysis in accordance with **AASHTO T 27** is as follows:

1. The dried (decanted) sample is placed in the top sieve of properly nested sieves. The sieves are nested in sequence with the smallest sieve placed on the pan and stacked by increasing size.
2. The shaking time is required to be sufficient to ensure that the sample is divided into fractional sizes. The actual shaking time is required to be determined in accordance with **ITM 906**. The following times are minimum for shakers used by the industry.

Coarse Aggregate, Size 9 or larger	5 Minutes
Coarse Aggregate, Smaller than Size 9	10 Minutes
Fine Aggregates	15 Minutes

- a. At the conclusion of sieving, the material retained on each sieve is carefully transferred to a weigh pan and weighed.
- b. Using the following tools:

>#16 sieve	Small trowel/flat metal
#16 to #50	Wire brush
<#50	Soft bristle brush

**Figure 8.5 Tools**

- c. The weight retained of the material on each sieve is recorded on the Sieve Analysis sheet. The weight may not exceed the allowable amount on each sieve as indicated in Figure 8.6.

TABLE 1 APPROXIMATED SIEVE OVERLOAD				
SCREEN SIZE	STANDARD 15 in. x 23in.	STANDARD 14 in. x 14 in.	12 in. DIAMETER	8 in. DIAMETER
3 in.	40.5 kg	23.0 kg	12.6 kg	-----
2 in.	27.0 kg	15.3 kg	8.4 kg	3.6 kg
1-1/2 in.	20.2 kg	11.5 kg	6.3 kg	2.7 kg
1 in.	13.5 kg	7.7 kg	4.2 kg	1.8 kg
3/4 in.	10.2 kg	5.8 kg	3.2 kg	1.4 kg
1/2 in.	6.7 kg	3.8 kg	2.1 kg	890 g
3/8 in.	5.1 kg	2.9 kg	1.6 kg	670 g
No. 4	2.6 kg	1.5 kg	800 g	330 g
8 in. diameter sieves, No. 8 to No. 200 shall not exceed 200g / sieve				
12 in. diameter sieves, No. 8 to No. 200 shall not exceed 469g / sieve				

Figure 8.6 Allowable Sieve Amounts

Sieve Analysis Form				
Total Weight 6917.7				
SIEVE SIZE	WEIGHT RETAINED	WEIGHT PASSING	PERCENT PASSING	PERCENT REQUIRED
1½ in.	g	g	%	%
1 in.	0 g	6917.7 g	%	%
¾ in.	1029.1 g	g	%	%
½ in.	2910.5 g	g	%	%
⅜ in.	1367.9 g	g	%	%
No. 4	1168.1 g	g	%	%
No. 8	270.9 g	g	%	%
No. 16	g	g	%	%
No. 30	g	g	%	%
No. 50	g	g	%	%
No. 100	g	g	%	%
No. 200	g	g	%	%
PAN	55.6 g	g	%	%
<b>DECANT ORIGINAL</b>	<b>FINAL</b>	<b>GRAMS LOSS</b>	<b>PERCENT LOSS</b>	<b>PERCENT REQUIRED</b>
6917.7 g	6811.3 g	106.4 g	1.5 %	%

Figure 8.7 Sieve Analysis Form

- The weight passing each sieve is calculated next by subtracting the weight retained on the largest sieve from the total sample weight. The weight retained on the next largest sieve is subtracted from the weight of material remaining from the first subtraction. This process is continued for all sieves.

Example:

1 in.	6917.7-0 = 6917.7
¾ in.	6917.7-1029.1 = 5888.6
½ in.	5888.6-2910.5 = 2978.1
⅜ in.	2978.1-1367.9 = 1610.2
No. 4	1610.2-1168.1 = 442.1
No. 8	442.1-270.9 = 171.2
Pan	171.2-55.6 = 115.6

Sieve Analysis Form				
Total Weight 6917.7				
SIEVE SIZE	WEIGHT RETAINED	WEIGHT PASSING	PERCENT PASSING	PERCENT REQUIRED
1½ in.	g	g	%	%
1 in.	0 g	6917.7 g	%	%
¾ in.	1029.1 g	5888.6 g	%	%
½ in.	2910.5 g	2978.1 g	%	%
⅜ in.	1367.9 g	1610.2 g	%	%
No. 4	1168.1 g	442.1 g	%	%
No. 8	270.9 g	171.2 g	%	%
No. 16	g	g	%	%
No. 30	g	g	%	%
No. 50	g	g	%	%
No. 100	g	g	%	%
No. 200	g	g	%	%
PAN	55.6 g	115.6g	%	%
<b>DECANT ORIGINAL</b>	<b>FINAL</b>	<b>GRAMS LOSS</b>	<b>PERCENT LOSS</b>	<b>PERCENT REQUIRED</b>
6917.7 g	6811.3 g	106.4 g	1.5 %	%

Figure 8.8 Sieve Analysis Form



5. The percent passing is calculated for each sieve by using the formula

$$\% \text{ Passing} = \frac{\text{Weight Passing Each Sieve}}{\text{Original Dry Sample Weight}} \times 100$$

**Examples:**

$\frac{3}{4}$  in. Sieve

$$\frac{5888}{6917.7} \times 100 = 85.1\%$$

$\frac{1}{2}$  in. Sieve

$$\frac{2978.1}{6917.7} \times 100 = 43.1\%$$

Sieve Analysis Form				
Total Weight 6917.7				
SIEVE SIZE	WEIGHT RETAINED	WEIGHT PASSING	PERCENT PASSING	PERCENT REQUIRED
1½ in.	g	g	%	%
1 in.	0 g	6917.7 g	100 %	%
¾ in.	1029.1 g	5888.6 g	85.1 %	%
½ in.	2910.5 g	2978.1 g	43.1%	%
⅜ in.	1367.9 g	1610.2 g	23.3%	%
No. 4	1168.1 g	442.1 g	6.4 %	%
No. 8	270.9 g	171.2 g	2.5 %	%
No. 16	g	g	%	%
No. 30	g	g	%	%
No. 50	g	g	%	%
No. 100	g	g	%	%
No. 200	g	g	%	%
PAN	55.6 g	115.6g	%	%
<b>DECANT ORIGINAL</b>	<b>FINAL</b>	<b>GRAMS LOSS</b>	<b>PERCENT LOSS</b>	<b>PERCENT REQUIRED</b>
6917.7 g	6811.3 g	106.4 g	1.5 %	%

### 8.9 Sieve Analysis Form

6. If the test has been done accurately, the sum of all the fractional weights retained (including the material in the pan) and the weight of material removed by decantation are approximately equal to the original dry weight. If the two weights differ by more than 0.3 percent, based on the original dry sample weight, the results are considered invalid.

$$\% \text{ Error} = \frac{\text{Original Dry Weight} - \text{Summation of Weights Measured}}{\text{Original Dry Weight}} \times 100$$

**Example:**

$$\frac{6917.7 - 6908.5}{6917.7} \times 100 = 0.1\%$$

0.1% = *valid test*

## FINENESS MODULUS

The fineness modulus is related to gradation. This term is commonly associated with aggregates for portland cement concrete. The purpose of this value is to determine the relative coarseness or fineness of the aggregate grading.

The fineness modulus is computed in accordance with **AASHTO T 27** by adding the cumulative percentages retained on the 3 1/2 in., 2 1/2 in., 2 in., 1 1/2 in., 3/4 in., 3/8 in., No. 4, No. 8, No. 16, No. 30, No. 50, and No. 100 sieves, and then dividing by 100. A large number indicates a coarse material. A small number indicates a fine material.

Size	100	-	% Passing	=	% Retained
3/8 in.	100	-	100	=	0.0
No. 4	100	-	100	=	0.0
No. 8	100	-	89.2	=	10.8
No. 16	100	-	68.3	=	31.7
No. 30	100	-	45.1	=	54.9
No. 50	100	-	13.8	=	86.2
No. 100	100	-	2.6	=	<u>97.4</u>
					281.0

$$281.0 / 100 = 2.81 = \text{Fineness Modulus}$$

**Figure 8.10 Fineness Modulus**

## **SIEVE ANALYSIS FOR DENSE GRADED (LONG GRADED) MATERIALS**

Dense graded materials, such as compacted aggregates and some B borrows or subbase, consist of substantial quantities of material retained on and passing the No. 4 sieve.

The procedure for conducting a sieve analysis on a dense graded material is:

1. The entire sample is sieved and weighed in the same manner as well graded materials, except the smallest sieve is required to be the No. 4 sieve.
2. The portion of the sample passing the No. 4 sieve is weighed.
3. Using a sand sample splitter, the portion of the sample passing the No. 4 sieve is reduced to approximately 500 grams.
4. The reduced sample is weighed and a proportionate factor is determined by dividing the weight of the portion of the sample passing the No. 4 sieve by the weight of the reduced sample. For example, if the total weight of the portion of material passing the No. 4 sieve is 2221.4 grams and the reduced sample weight is 503.4 grams, the proportionate factor would be equal to 2221.4 grams divided by 503.4 grams, which equals 4.413.
5. The reduced sample is sieved for 15 minutes.
6. The material on each sieve (#8-#200) is weighed and multiplied by the proportionate factor. The calculated weight is recorded as the total weight of material retained on that sieve.
7. The calculations for percentage passing are completed as for well-graded aggregates.

<b>TOTAL WEIGHT: 6800.8g</b>					
<b>SIEVE SIZE</b>	<b>LONG GRADED WEIGHT RET.</b>	<b>WEIGHT RETAINED</b>	<b>WEIGHT PASSING</b>	<b>PERCENT PASSING</b>	<b>PERCENT REQUIRED</b>
1½ in.		0 g	6800.8 g	100 %	100 %
1 in.		312.9 g	6487.9 g	95.4 %	80-100 %
¾ in.		877.2 g	5610.7 g	82.5 %	70-90 %
½ in.		1228.3 g	4382.4 g	64.4 %	55-80 %
⅜ in.		580.5 g	3801.9 g	55.9 %	%
No. 4		1072.1 g	2729.8 g	40.1 %	35-60 %
No. 8	222.1 g	940.4 g	1789.4 g	26.3 %	25-50 %
No. 16	g	g	g	%	%
No. 30	192.7 g	815.9 g	973.5 g	14.3 %	12-30 %
No. 50	g	g	g	%	%
No. 100	g	g	g	%	%
No. 200	84.8 g	359.0 g	614.5 g	9.0 %	5.0-13.0 %
PAN	4.2 g	17.8 g	g	%	%
<b>DECANT</b>	<b>ORIGINAL</b>	<b>FINAL</b>	<b>GRAMS LOSS</b>	<b>PERCENT LOSS</b>	<b>PERCENT REQUIRED</b>
	6800.8 g	6220.7 g	580.1 g	8.5 %	%
<b>PROPORTIONATE FACTOR</b>		<b>TOTAL WEIGHT PASSING No. 4</b>	<b>SAMPLE SIZE</b>		
		2133.2 g	503.8 g		4.234

Figure 8.11 Sieve Analysis for Dense Grade (Long Graded) Materials

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# DELETERIOUS MATERIAL

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Most of the tests for deleterious materials apply to coarse aggregates. The only deleterious materials of concern in fine aggregates is organic impurities.

## DELETERIOUS MATERIALS IN COARSE MATERIALS

Deleterious tests for coarse aggregates are based on visual inspection and require training and judgment. Deleterious substances of concern are clay lumps and friable particles, non-durable materials, coke, iron, and chert. Coke and iron are only of concern in slag, and no guidelines are given.

### CLAY LUMPS AND FRIABLE PARTICLES:

Clay lumps and friable particles are defined as the material remaining after decantation that may be mashed with the fingers. The test is conducted according to **AASHTO T 112**.

A sample consists of material retained on the No. 4 sieve and each sieve above the No. 4 sieve, following decantation of sieve analysis. The sample is soaked 24 hours, plus or minus 4 hours, in distilled water. After soaking, any material or particles that may be broken by the fingers and (Figure 8.12) are removable by wet sieving are classified as clay lumps or friable material. The material retained after wet sieving is dried to constant weight and weighed.



**Figure 8.12 Clay Lumps**

The percent clay or friable material is calculated by:

$$\% \text{ Clay or Friable} = \frac{\text{Dry Weight of Sample} - \text{Dry Weight Retained (Wet Sieving)}}{\text{Dry Weight of Sample}} \times 100$$

## NON-DURABLE MATERIALS:

Non durable materials are divided into two types:

1. Soft material as determined by **ITM 206**
2. Structurally weak material as determined by visual inspection

Both tests are conducted on the sample material retained on the 3/8 in. sieve and each sieve above the No. 3/8 in. sieve.

The Scratch Hardness test (Figure 8.13) is conducted on gravel coarse aggregate. Each particle to be tested is subjected to a scratching motion of a brass rod, using a 2 lbf load.



**Figure 8.13 Scratch Hardness Test**

Particles are considered soft if a groove is made in the particle without deposition of metal from the brass rod or if separate particles are detached from the rock mass. A particle is classified as soft only if one-third or more of the volume is found to be soft. Structurally weak materials are visually identified and include:

1. Ocher
2. Unfossilized shells
3. Conglomerates -- cemented gravels
4. Shale -- laminated rock of clay-size minerals
5. Limonite -- iron oxide, ranging from yellow-brown to black in color and is frequently a concretion around a soft core

6. Weathered rock that is structurally weak
7. Coal, wood, and other foreign materials
8. Materials with loosely cemented grains or a weathered coating

Particles determined to be soft or structurally weak are combined and the percent by weight of non-durable material is calculated by:

$$\% \text{ Non-Durable} = \frac{\text{Weight of Non-Durable Material above } 3/8 \text{ in. Sieve}}{\text{Weight of Sample above } 3/8 \text{ in. Sieve}} \times 100$$

#### CHERT:

Chert is a rock of varied color, composed of glassy silica, and very fine-grained quartz, and is only picked from coarse aggregate. Unweathered chert appears hard, dense, and brittle with a greasy texture. Weathered chert appears chalky and dull. Chert is likely to have concave surfaces with sharp outer edges when freshly broken.

Total chert is picked from the sample following decantation and gradation. Chert is picked from the material retained on the 3/8 in. sieve and each sieve above the 3/8 in. sieve for aggregate sizes 2 through 8, 43, 53, and 73. For aggregate sizes 9, 11, 12, and 91, chert is picked from the material retained on the No. 4 sieve and each sieve above the No. 4 sieve. The procedure for determining the total chert includes:

1. All chert, including questionable chert, is picked from the sample.
2. All pieces of questionable chert are further tested by the following procedures:
  - a. Scratching glass. Chert pieces scratch glass.
  - b. Breaking Pieces. Chert breaks into rounded surfaces with sharp edges. If pieces do not break into rounded surfaces with sharp edges, they are added to the soft or non durable material.
  - c. Reaction with acid. Chert does not react with 0.1 N hydrochloric acid.
3. All material determined to be chert is weighed and the percent of total chert is calculated using the following formulas:

For aggregate sizes 2 through 8, 43, 53, and 73:

$$\% \text{ Total Chert} = \frac{\text{Weight of Chert above } \frac{3}{8} \text{ in. Sieve}}{\text{Total Weight of Sample above } \frac{3}{8} \text{ in. Sieve}} \times 100$$

For aggregate sizes 9, 11, 12, and 91:

$$\% \text{ Total Chert} = \frac{\text{Weight of Chert above No. 4 Sieve}}{\text{Total Weight of Sample above No. 4 Sieve}} \times 100$$

The percent chert requirement of **904.03(a)** applies to chert less than 2.45 bulk specific gravity. If the percent total chert exceeds this chert requirement, the sample is tested for lightweight pieces in accordance with **AASHTO T 113** to determine the percent chert less than 2.45 bulk specific gravity (Figure 8.14).



Figure 8.14 Chert Test

## DELETERIOUS MATERIALS IN NATURAL SANDS:

The purpose of the **AASHTO T 21** test is to provide a warning that further tests of the sand are necessary before the sands are approved for use. The procedure is as follows:

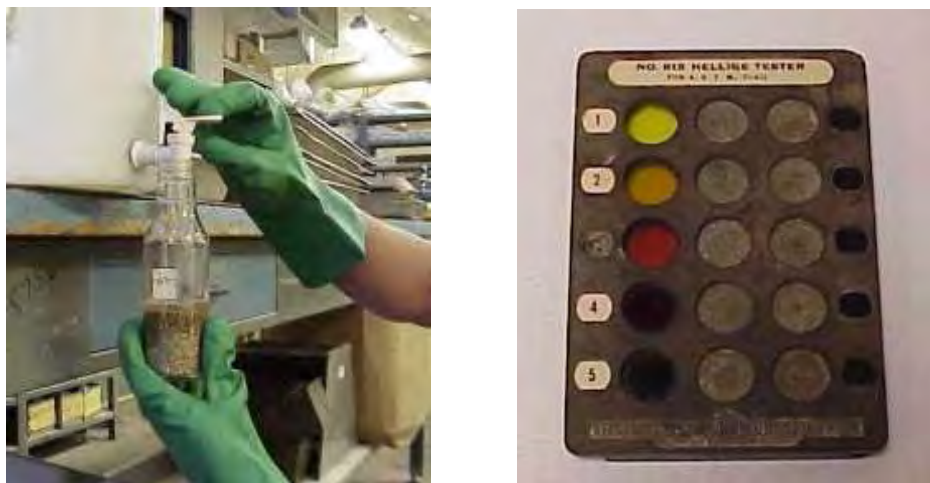
1. A glass bottle is filled with approximately 4½ fl oz of the sand to be tested.
2. A 3 percent sodium hydroxide (NaOH) solution in water is added until the volume of the sand and liquid, indicated after shaking, is approximately 7 fl oz.



3. The bottle is stoppered, shaken vigorously, and allowed to stand for 24 hours.
4. The color of the supernatant liquid above the test sample is compared to reference standard colors.
5. If the color of the supernatant liquid is darker than that of the reference color, the sand may contain injurious organic compounds, and further tests are to be made before approving the sand for use in concrete.

The **AASHTO T 71** test compares the compressive strength of mortar specimens made from the suspect sand to the compressive strength of mortar made from acceptable sand.

The colorimetric test (**AASHTO T 21**) is conducted first (Figure 8.15). If the color in solution is lighter than a standard, the fine aggregate is acceptable. If the color is darker, further testing of the fine aggregate for strength in mortar, **AASHTO T 71**, is required. If the effect of any organic matter reduces the strength no more than 5 percent, the fine aggregate is acceptable. Also, observations are required to be made to determine whether the organic material retards the mortar set or changes the necessary air entraining admixture dosage.



**Figure 8.15 Colorimetric Test**

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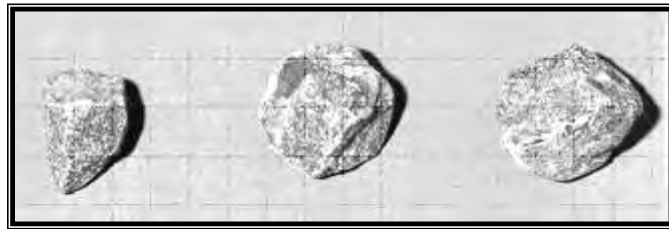
# CRUSHED PARTICLES

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**ASTM D 5821** includes the procedure for determining the quantity of crushed particles (Figure 8.16) Crushed particle requirements are used for gravel coarse aggregates and seal coats in HMA (one- and two-faced) as well as compacted aggregates (total crushed).



**Crushed Particles (Sharp Edges, Smooth Surface)**



**Crushed Particles (Round Edges, Rough Surfaces)**



**Non-Crushed Particles (Round Edges, Smooth Surfaces)**



**Non-Crushed Particles (Rounded Particles, Smooth Surfaces)**

**Figure 8.16 Crushed Particles**

The test applies to all particles retained on the No. 4 sieve and is conducted as follows:

1. The total sample is decanted in accordance with AASHTO T11 and sieved in accordance with AASHTO T27. Separate the sample weight from the #4 sieve and above weight for crush count.
2. Each particle is evaluated to verify that the crushed criteria is met. If the fractured face constitutes at least one-quarter of the maximum cross-sectional area of the rock particle and the face has sharp or slightly blunt edges, the particle is considered a crushed particle.
3. Particles are separated into two categories: (a) crushed particles, and (b) non-crushed particles.
4. When two-faced crushed particles are required for aggregates used in HMA the procedure is repeated on the same sample.
5. The percent of crushed particles is determined by the following formula:

$$P = \frac{F}{F + N} \times 100$$

where:

P = percentage of crushed particles

F = weight of crushed particles

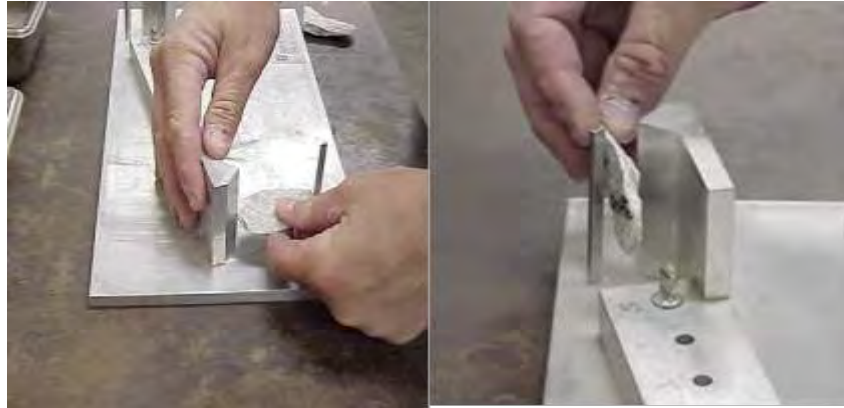
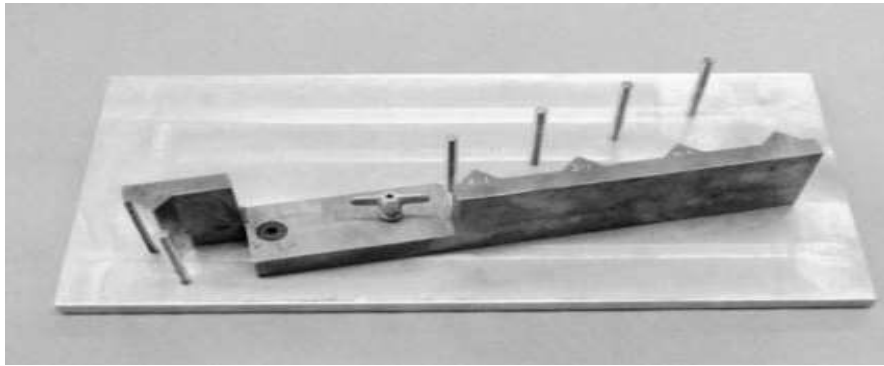
N = weight of uncrushed particles

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## FLAT AND ELONGATED PARTICLES

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**ASTM D 4791 (Method B)** includes the procedure for determining the quantity of flat and elongated particles (Figure 8.17). The Specifications define a flat and elongated particle as "one having a length to thickness ratio greater than five."



**Figure 8.17 Flat and Elongated Particles**

The test is conducted on particles retained on the 3/8 in. sieve and each sieve above the 3/8 in. sieve as follows:

1. The total sample retained on the 3/8 in. sieve is weighed.
2. Each size fraction above the 3/8 in. sieve present in the amount of 10 % or more of the original sample is reduced until approximately 100 particles are obtained for each size fraction.
3. Each particle is measured with the proportionate caliper device set at the required ratio
4. The flat and elongated particles are weighed for each sieve.
5. The percent of the flat and elongated particles is then determined on each sieve by the following formula:

$$\% \text{ Flat and Elongated} = \frac{\text{Weight of F\&E Particles for each Sieve}}{\text{Total Weight of Reduced Sample for each Sieve}} \times 100$$

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# PLASTIC LIMIT

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Compacted aggregate materials, fine aggregate for SMA, mineral filler for SMA, and coarse aggregate sizes No. 43, 53, and 73, require tests for determining the plastic limit and liquid limit (Figure 10-9) of minus No. 40 sieve material. The plastic limit test may be conducted accurately only in a laboratory; however, the possibility of a plastic condition may be determined by a field check test. The liquid limit is required to be conducted in the laboratory.



**Figure 8.17 Field Check Plastic Limit Test**

The plastic limit test may not be conducted on the same sample used for any other field tests. Therefore, in addition to the sample selected for the other field tests, the Technician is required to split and dry a sample of approximately 1000 grams. Most compacted aggregate materials do not contain plastic fines. If the specimen cannot be rolled into a thread of 1/8 in. diameter, the Technician may assume that the material is either nonplastic or has a low plastic content, and no additional testing is required. The test is conducted using a small spatula, a ground glass plate, and an evaporating dish in accordance with **AASHTO T 90** as follows:

1. Using sufficient sieves, remove the material above the No. 40 sieve. All of the minus No. 40 sieve material in the sample is required. Any minus No. 40 sieve material clinging to the

larger particles is required to be scraped free and all the dried composite particles retained above the No. 40 sieve is required to be broken up.

2. Thoroughly mix the minus No. 40 sieve material and select a sample of about 20 grams.
3. Place the sample in a suitable container, preferably an evaporating dish, and thoroughly mix with distilled or de-mineralized water until the material becomes plastic enough to be easily shaped into a ball.
4. Take about half of the sample and squeeze and form the sample into the shape of a small cigar. Place the specimen on a ground glass plate. With fingers, using just sufficient pressure, roll the specimen into a thread of uniform diameter throughout the sample length. The rate of rolling will be between 80 and 90 strokes per minute, counting a stroke as a complete motion of the hand forward and back to the starting position. The rolling continues until the thread is 1/8 in. in diameter.

If the specimen may be rolled into a thread of 1/8 in., the material is considered plastic and a sample is required to be sent to the District laboratory for an accurate determination of plasticity index.

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# TABLE OF FORMULAS

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## SIEVING

$$\% \text{ Decant} = \frac{\text{Original Dry Weight} - \text{Dry Weight after Decant}}{\text{Original Dry Weight}} \times 100 \quad (0.0) \quad \text{p. 113}$$

$$\% \text{ Passing} = \frac{\text{Weight Passing Each Sieve}}{\text{Original Dry Sample Weight}} \times 100 \quad (0.0) \quad \text{p. 118}$$

### AASHTO T 27 VERSION

$$\% \text{ Error} = \frac{\text{Original Dry Weight} - \text{Summation of Weights Measured}}{\text{Original Dry Weight}} \times 100 \quad (0.0) \quad \text{p. 119}$$

$> \pm 0.3\%$  is **Invalid Test**

### QUICK VERIFICATION METHOD

$$\% \text{ Error} = \frac{\text{Weight Passing Pan} - \text{Decant Grams Loss}}{\text{Original Dry Weight}} \times 100 \quad (0.0)$$

$$\text{Fineness Modulus} = \frac{\text{Cumulative Percentages "Retained"}}{100} \quad (0.00) \quad \text{p. 119}$$

## DELETERIOUS, CRUSHED, FLAT AND ELONGATED, MOISTURE

$$\% \text{ Clay or Friable} = \frac{\text{Dry Weight of Sample} - \text{Dry Weight Retained (Wet Sieving)}}{\text{Dry Weight of Sample}} \times 100 \quad (0.0) \quad \text{p. 123}$$

$$\% \text{ Non-Durable} = \frac{\text{Weight of Non-Durable Material above } 3/8 \text{ in. Sieve}}{\text{Weight of Sample above } 3/8 \text{ in. Sieve}} \times 100 \quad (0.0) \quad \text{p. 124}$$

**For aggregate sizes 2 through 8, 43, 53, and 73:**

$$\% \text{ Total Chert} = \frac{\text{Weight of Chert above } 3/8 \text{ in. Sieve}}{\text{Total Weight of Sample above } 3/8 \text{ in. Sieve}} \times 100 \quad (0.0) \quad \text{p. 125}$$

**For aggregate sizes 9, 11, 12, and 91:**

$$\% \text{ Total Chert} = \frac{\text{Weight of Chert above No. 4 Sieve}}{\text{Total Weight of Sample above No. 4 Sieve}} \times 100 \quad (0.0) \quad \text{p. 125}$$

**% Crushed (P)**

$$= \frac{\text{Weight of Crushed Particles (F)}}{\text{Wt. of Crushed Particles (F) + Wt. of Uncrushed Particles (N)}} \times 100 \quad (0) \quad \text{p. 128}$$

$$P = \frac{F}{F + N} \times 100$$

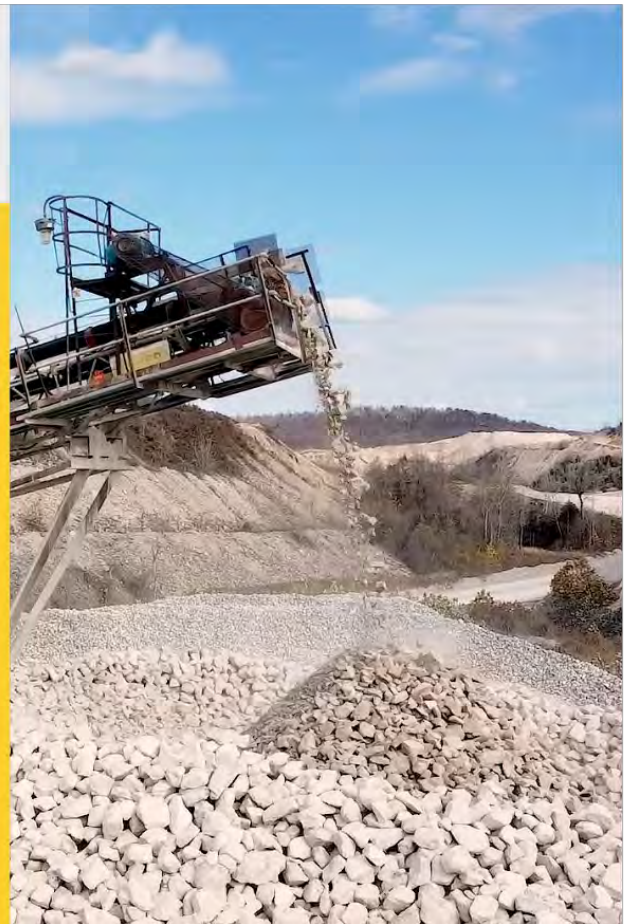
$$\% \text{ Flat and Elongated} = \frac{\text{Weight of F\&E Particles for each Sieve}}{\text{Total Weight of Reduced Sample for each Sieve}} \times 100 \quad (0) \quad \text{p. 129}$$

$$\% \text{ Moisture} = \frac{\text{Weight Wet} - \text{Weight Dry}}{\text{Weight Dry}} \times 100 \quad (0.0) \quad \text{p. 111}$$



# QUALITY CONTROL PLAN

## CHAPTER 9



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# QUALITY CONTROL PLAN

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## **THE QUALITY CONTROL PLAN**

Development

Operational Types

Details

Addenda

## **QUALITY CONTROL PLAN ANNEX**

## **QUALITY CONTROL PLAN CHECKLIST**

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# QUALITY CONTROL PLAN

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If a single part of the Certified Aggregate Producer Program is considered to be the most important, then that is the Quality Control Plan (QCP). The QCP is required to encompass the total process from preliminary site approval up to the point where the material leaves the Producer's control. The QCP is required to identify and address all products generated and the type, frequency, and limits of sampling and testing to be done. The QCP focuses on a quality product and answers the questions of who, what, when, where, and how.

## **DEVELOPMENT**

The QCP is developed while the Producer is in the Coordinated Testing Phase. When starting to develop the QCP, the Producer may refer to this chapter, the model QCPs (Appendix C), INDOT's ledge letters, summary of quality letters, AP letters, the CAP Program (ITM 211), and Section 917.

The QCP is site and plant specific. A QCP for one site would not necessarily be satisfactory for another site.

## **OPERATIONAL TYPES**

The CAPP provides for Plants and Redistribution Terminals. The QCP is required to identify the intended type of operation. In some instances, a primary source may also sell material produced at another source and therefore would be operating as both a Plant and a Redistribution Terminal.

Directive 201 gives great definitions and more detail of each phase for entering into the Certified Aggregate Producer Program. This document also defines redistribution and producer yards.

## DETAILS

The following list is provided to assist in the preparation of the QCP; however, the list is not to be all-inclusive (ITM 211 Section 14.0). A QCP is required to include (*Note: Clarifications of the content will not be found in ITM 211 but only here in the manual.*):

**14.2** The QCP shall include the following information for each Plant/Redistribution Terminal, if applicable:

**14.2.1** The location of the site, including latitude and longitude, physical address, and mailing address if different than the physical address. Reference to the nearest identifiable points such as highways and towns shall also be included.

📍 *This is where the Plant/Redistribution portion is defined as well.*

**14.2.2** The Parent Company, Management Representative, Certified Aggregate Technician(s) and Qualified Technician(s) at each location and the procedure for contacting these individuals by phone, US Mail, and email address. Also, the CAPP duties and responsibilities of each of the people listed shall be specified.

**14.2.3** A list and description of all portions of the mineral deposit(s) indicating the different quality classes as established in the current editions of ITM 203 and ITM 210, and as indicated on the Source Category Classification Approval letter. The manner in which each quality class will be processed, handled, and stockpiled shall be specified.

📍 *This information can be found on the ledge letters and quality letters. Adding specific details of the geology for limestone and sand and gravel plants needs to be added in this section as well.*

**14.2.4** A statement regarding AP Aggregate shall be made to include the ledges for stone, production zone for gravel, the general handling and crushing procedures, and stockpile signage. An AP Production Control Plan may be included in the Appendix instead of this statement.

📍 *This is the AP letter that gets sent out saying it's been approved for a certain number of years.*

**14.2.5** A statement regarding ACBF shall be made to include the procedures for sampling and testing for determination of leachate in accordance with ITM 212.

📍 *ACBF is Blast Furnace Slag.*

**14.2.6** A statement regarding SF shall be made to include the procedures for sampling and testing for determination of bulk specific gravity when SF is used in SMA mixtures. Also, a statement regarding SF coarse aggregate shall be made to include the procedures for sampling and testing for determination of deleterious materials in accordance with ITM 219.

↳ *SF is Steel Slag.*

**14.2.7** A statement regarding lightweight aggregate shall be made to include the procedures for sampling and testing for determination of the specific gravity factor and the absorption in accordance with ITM 222.

↳ *This is for lightweight aggregate specified in ITM 222 and not to be confused with lightweight chert.*

**14.2.8** A statement regarding natural sand fine aggregate composite stockpiling shall be made to include the procedures for sampling and testing when composite stockpiling of multiple sources into one stockpile.

**14.2.9** An explanation for each material having marginal quality characteristics, and the plan or controls to be used for such materials.

↳ *Examples of this can be Absorption Number or result that can be high, An LA that may not always able to pass for Class A material, or zones of high percent of chert and deleterious. These issues will make the producer test more frequently to make sure material passes the specifications.*

**14.2.10** An identification of the category in which each material produced is classified.

↳ *This information can be found on the summary of quality letters and AP letters. The CAPP D List, found on the INDOT website, would be a great tool as well.*

**14.2.11** A generic production flow diagram, which shall be a step-by-step chart using standard symbols, showing all the steps involved with mining and processing from the natural deposit to the finished material and the points of sampling. Detailed items such as equipment manufacturer's names, screen sizes, dimensions, etc., are not required to be listed. A copy of the symbol legend shall be included.

**14.2.12** A sampling plan that includes locations, devices, techniques, frequencies, and test methods, if applicable.

↳ *This includes strike-off method as described in ITM 207. If doing strike-off method, then it needs to be addressed in the QCP for all products or certain products.*

**14.2.13** A testing plan that includes the types of tests, and test methods. The procedure for isolating nonconforming material shall be specified.

↳ *The nonconforming procedures can be found in ITM 211 Section 12.7. These procedures shall be followed.*

**14.2.14** A list of the target mean values, standard deviations, and control limits on the critical sieves for each material identified as being controlled by critical sieve requirements. Changes in the target mean are allowed by addenda to the QCP.

Materials for which no control limits are appropriate shall be identified.

The gradation limits for all Quality Assurance materials shall be included.

Gradation limits for products that are intended to meet multiple gradation requirements shall use the more stringent controls for each product and these limits shall be included.

**14.2.15** A description of any other process control techniques that will be used beyond the minimums established by Department specifications and policies. These controls may include, but are not limited to, the following:

- a) Different types or greater frequencies of material testing.
- b) Other midstream sampling and testing prior to material completion.
- c) Visual checks and monitoring.

**14.2.16** A plan for downstream controls after material completion. These controls shall address such items as the identification of material stockpiles by signing, including ledges if applicable, or other acceptable methods, techniques for construction of proper stockpiles, material retrieval techniques and safeguards to ensure the loading and shipping of uncontaminated material.

**14.2.17** A list of test equipment that is calibrated or verified, the test methods and frequency of calibration or verification of the equipment, and a statement of accessibility of the laboratory and documentation to Department personnel.

If the laboratory is not located at the Plant, the location of the laboratory shall be designated, and the procedure for transporting samples to the laboratory shall be included.

↳ *Multiple Labs can be added to the QCP but will need the addresses of labs.*

- *Having multiple labs can help when lab equipment breaks or when the technicians are not available.*

**14.2.18** A documentation plan with details on control charting, test data, and the diary. Copies of the forms may be included.

↳ *Include any deviations to the charts, tests, and diaries. An example would be: if the producer has a circle instead of a square for Load-out test on the chart; diaries that have extra content.*

**14.2.19** The method by which the frequency of production and load-out testing of the Certified Materials is verified.

↳ *Start of Production, Normal Production, and Load-out frequencies can be found in ITM 211 Section 11.3.*

- *For example, on the Loadout frequencies, for determining the appropriate sampling frequency for your QC Plan, a producer could look at their past or anticipated production and shipping history for each product. However, this is how the loadout frequency is stated in ITM 211 Section 11.3.3.*
  - *Less than 100 tons shipped per year equals no samples required.*
  - *First 1,000 tons shipped of the year will need 1 test.*
  - *Sample within  $\pm 1,000$  tons or  $\pm 1$  business day*
  - *Frequency is based on what the producer decides is best. If anything, the 1 test per 8,000 tons shipped can still be in place.*

**14.2.20** The location of the reference documents, control charts, diary, test data, material shipment records, and any other pertinent information.

↳ *Reference documents would be the AASHTOs, ASTMs, and ITMs.*

**14.2.21** The method of control for each Producer Yard.


↳ *Method of control is about handling the material at the Producer Yard and keeping documentation. Directive 201 has great definitions of the Producer Yard.*

**14.2.22** A statement of the procedure for handling addenda.

**14.2.23** Annual Aggregate Source Report (stone sources only). The report shall be in accordance with ITM 203, 8.1 and shall be included in the Appendix of the QCP. Redistribution Terminals are required to include this report for materials received from a source that is not a Certified Aggregate Producer.

**14.2.24** An Appendix. As a minimum the appendix shall contain an Addenda Summary Sheet.

**14.3** Authentication. The last page of the QCP shall contain two signature blocks. The right-hand block shall be signed and dated at the time of submittal by the Producer's Management Representative, and shall include the title of the person making the signature. The left hand block shall be signed and dated at the time of approval by the State Materials Engineer, Division of Materials and Tests. The Producer shall submit the QCP to the District Geologist for review. The District Geologist will forward it to the INDOT Statewide Geologist for final review and approval. The authentication page will be returned to the Producer after approval.

 *This is for when producers are coming into the Certified Aggregate Producer Program.*

Remember the QCP is a document that describes the process of the material at your quarry/gravel pit. It is also a document on how the producer tests, how samples are collected and reported, how diaries and charts are done, and what products are still being produced, etc. Always review the QCP to make sure that no procedures have changed. If so, then an addendum is required.



## **ADDENDA**

The addenda applies to changes of the QCP when Trial Phase has ended and the QCP was signed and approved by the State Materials Engineer. Addenda are defined as any addition or deletion to the QCP. Each page of the QCP that is revised is required to include the source number, date of revision, and means of identifying the revision. The addenda are required to include a signed and dated authentication page.

Revisions for Certified Material additions, Certified Material deletions, target mean and control limit values, or Certified Aggregate Technicians are submitted in the format of the QCP Annex as they occur. Upon approval by the District Testing Engineer, the QCP Annex is placed in the Appendix of the QCP until such time that the revisions are incorporated into the QCP.

Revisions, other than items on the QCP Annex, are maintained on an Addenda Summary Sheet. The Addenda Summary Sheet is a page of the QCP Appendix that is used to record a brief description of the revision until such time that the revision is incorporated into the QCP.

Addenda may be submitted at the audit close-out meeting or between January 1st and April 1st of each calendar year. The addenda are required to include items on the QCP Annex, items on the Addenda Summary Sheet, and any other necessary revisions at the time of submittal. Upon incorporation into the QCP as addenda, the QCP Annex and items on the Addenda Summary Sheet are required to be removed from the QCP Appendix.

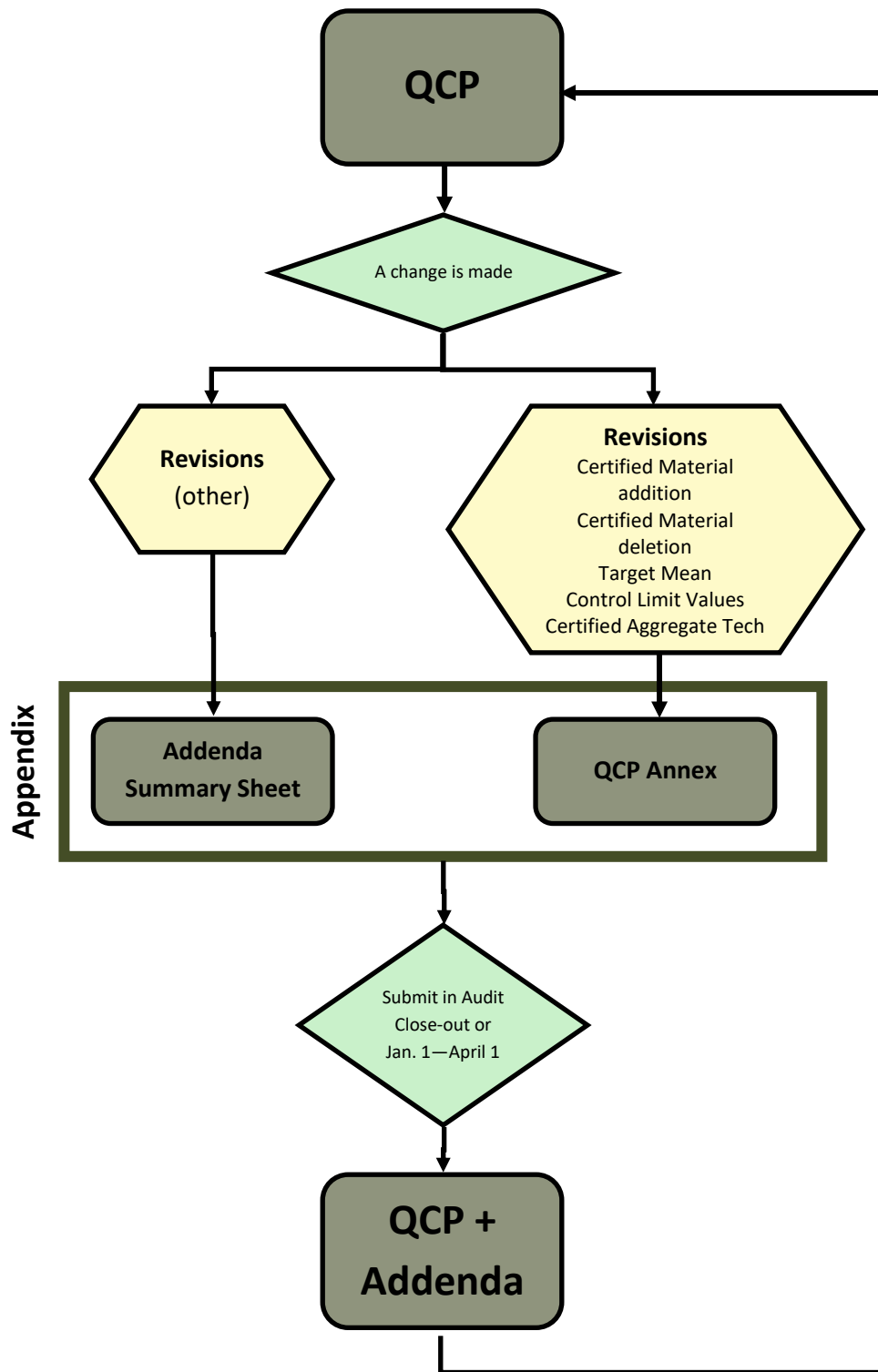


Figure 9.1 QCP Revisions Flowchart

## QCP ANNEX

Company \_\_\_\_\_

Source No. \_\_\_\_\_

Q No. \_\_\_\_\_

### **NEW CERTIFIED MATERIAL ADDITION**

Size Designation: \_\_\_\_\_ Specification: Standard or QA (see attached gradation)  
Originating SC #: \_\_\_\_\_ Category Rating: IA IB IIA IIB III GS-A GS-B

#### **Circle all that apply**

Type: [Stone] [Gvl (Crushed/Uncrushed)] [Sand (Man./Nat./Slag)] [Slag (ACBF/SF)]  
[Dolomite Approved (ITM 205)] [Recycled Concrete (Contract #: \_\_\_\_\_)]  
[Alternate Polish Resistant Aggregate (ITM 214)] [Other \_\_\_\_\_]

Product Quality Rating: AP AS A B C D E F NA

Ledges and/or Area of Production: \_\_\_\_\_

Does finished product go into Separate or Composite Stockpile? \_\_\_\_\_

Is material from New Production, Existing Stockpile, or Both? \_\_\_\_\_

Size of Existing Stockpile: \_\_\_\_\_ t Annual Production: <10000 t ≥10000t

### **EXISTING CERTIFIED MATERIAL REVISION**

Current Size Designation: \_\_\_\_\_ Originating SC #: \_\_\_\_\_

New Size Designation: \_\_\_\_\_ Type (see above): \_\_\_\_\_

Ledges: \_\_\_\_\_ Product Quality Rating: AP AS A B C D E F NA

### **EXISTING CERTIFIED MATERIAL DELETION**

Size Designation: \_\_\_\_\_ Originating SC #: \_\_\_\_\_ Type (see above): \_\_\_\_\_

Product Quality Rating: AP AS A B C D E F NA D# (DTE) \_\_\_\_\_

### **TARGET MEAN and CONTROL LIMITS REVISION**

Certified Material: \_\_\_\_\_

Current  $\bar{X}$ : \_\_\_\_\_ Existing Control Limits: \_\_\_\_\_

New  $\bar{X}$ : \_\_\_\_\_  $\sigma$ : \_\_\_\_\_ # Tests: \_\_\_\_\_ New Control Limits: \_\_\_\_\_

### **CERTIFIED AGGREGATE TECHNICIAN REVISION**

Delete CAT from QCP \_\_\_\_\_

Add CAT to QCP \_\_\_\_\_

\_\_\_\_\_  
District Testing Engineer

\_\_\_\_\_  
Date

\_\_\_\_\_  
Management Representative

\_\_\_\_\_  
Date

**CERTIFIED AGGREGATE  
QUALITY CONTROL PLAN CHECKLIST**

**Date** \_\_\_\_\_

**Source No.** \_\_\_\_\_

**Plant/Redistribution Terminal Name** \_\_\_\_\_

**Plant/Redistribution Terminal Location** \_\_\_\_\_

- ☐ Telephone Number
- ☐ Address
- ☐ County
- ☐ Section
- ☐ Township
- ☐ Range
- ☐ Reference to Identifiable Points

**Parent Company Name** \_\_\_\_\_

- ☐ Address

**Type of Aggregate Source**

- ☐ Plant, Redistribution Terminal, or Combination

**Organizational Structure**

- ☐ Management Representative
- ☐ Certified Technician(s) by Location
- ☐ CAPP Duties and Responsibilities of People Listed

**Mineral Deposits**

- ☐ List
- ☐ Description
- ☐ Quality Class
- ☐ Processing, Handling, & Stockpiling Procedures
- ☐ Summary of Ledge Quality Test Letter Date (Stone)
- ☐\* Marginal Quality Products and Plan for Control

\* Only If Occurs

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**AP Aggregate \***

- ☐ Ledges for Stone or Production Zone for Gravel
- ☐ General Handling and Crushing Procedures
- ☐ Stockpile Signage
- ☐ AP Production Control Plan in Appendix (optional)

**Air Cooled Blast Furnace Slag -- Leachate Testing\***

- ☐ Sampling Procedure
- ☐ Testing Procedure (ITM 212)
- ☐ Frequency

**Steel Furnace Slag – Deleterious Testing\***

- ☐ Sampling Procedure
- ☐ Testing Procedure (ITM 219)
- ☐ Frequency

**Steel Furnace Slag -- Bulk Specific Gravity Testing (SMA)\***

- ☐ Sampling Procedure
- ☐ Testing Procedure (AASHTO T 85)
- ☐ Frequency

**Composite Stockpiling\***

- ☐ Sources
- ☐ Monthly Summary Report
- ☐ Means of Tracking Bulk Specific Gravity and Absorption

**Material Categories - Each**

- ☐ Standard Specifications
- ☐ Quality Assurance
- ☐ Alternate

**Production Flow Diagram**

- ☐ Points of Sampling
- ☐ Symbol Legend

\* Only If Occurs

## **Sampling Plan**

- ☐ Frequency
- ☐ Locations
- ☐ Sampling Devices and Techniques
- ☐ Test Method Numbers
- ☐ Means of Tracking Production and Load-out Tests

## **Testing Plan**

- ☐ Gradation
- ☐ Decantation (Load-out only)
- ☐\* Crushed Particles (Min. 1/Week, None If < 100 t)
- ☐\* Deleterious Material (Min. 1/Week, None If < 100 t)
- ☐ Procedure for Isolating Non-Conforming Material
- ☐ Test Method Numbers

## **Gradation Control**

- ☐ Critical Sieve for Quality Assurance Materials
- ☐ Target Mean Values - Each
- ☐ Standard Deviations - Each
- ☐ Control Limits - Each
- ☐ Gradation Limits for all Applicable Sieves for Quality Assurance Materials
- ☐\* Identification of Materials with no Control Limits
- ☐\* Load-Out Target Mean and Control Limits Different from Normal Production Values

## **Process Control Techniques**

- ☐\* Types or Greater Frequencies of Testing
- ☐\* Mid-Stream Sampling & Testing
- ☐\* Visual Checks & Monitoring

## **Downstream Control**

- ☐ Identification of Stockpiles (Size of Material)
- ☐ Stockpile Construction Technique
- ☐ Product Retrieval Technique - Loading & Shipping Safeguards

\* Only If Occurs

## **Laboratory Capability**

- ☐ Location
- ☐ List and Description of Verified Equipment
- ☐ Verification Test Methods and Frequency

## **Documentation Plan**

- ☐ Reference Publications
- ☐ Diary
- ☐ Control Charts
- ☐ Test Data
- ☐ Material Shipment Record
- ☐ Location of Documents
- ☐ Copies of Forms (optional)

## **Producer Yard**

- ☐\* Method of Control

## **Addenda**

- ☐ Means of Handling Addenda
- ☐ Statement Concerning Source Number, Date of Revision, and Means of Identifying Revision

## **Annual Aggregate Source Report (Stone Only)**

- ☐ Included in Appendix

## **Authentication**

- ☐ Last Page
- ☐ Right Hand Signature Block Signed and Dated by Producer Management Representative
- ☐ Left Hand Blank & Title – Manager, Office of Materials Management

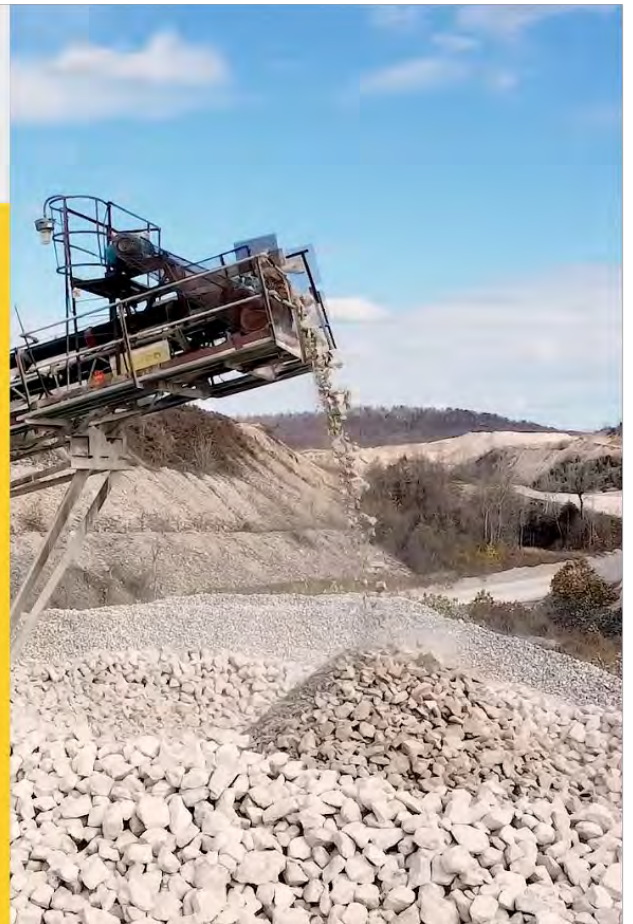
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# STATISTICAL QUALITY CONTROL FOR AGGREGATE PROCESSING

## CHAPTER 10





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# STATISTICAL QUALITY CONTROL FOR AGGREGATE PROCESSING

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## **QUALITY CONTROL**

- Accuracy
- Precision
- Capability

## **UNDERSTANDING THE PROCESS**

- Current Process
- Process Stability
- Decision Making
- Process Capability
- Process Control

## **PROCESS CHARACTERISTICS**

- Continuous Processing
- Product Alteration
- Multiple Products

## **STATISTICAL CONCEPTS**

- Data Sets
- Mean
- Standard Deviation
- Normal Distribution
- Variability
- Capability and Compliance –  
The Z Value

## **CONTROL CHARTING**

- When to Use Charts
- Control Chart Legend

## **QUALIFYING A PRODUCT**

- Setting a Target – 1.65 Rule
- Examples for Setting a Target
- Mean & Control Limits
- Plotting the Data
- Interpreting Control  
Charts

## **COMPLIANCE**

### **LOADOUT MATERIAL**

### **REQUIREMENTS & PERCENT WITHIN LIMITS**

- ITM 211-2026 Loadout
- Sampling & Testing
- Percent Within Limits
- Loadout Data Reporting /  
Department Spreadsheet

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# STATISTICAL QUALITY CONTROL FOR AGGREGATE PROCESSING

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The process of producing and shipping mineral aggregate is a relatively simple one. The procedure does not require high technology, and the methods used to control this process are equally as simple. These methods, however, account for all the many difficulties a Producer may encounter in production of aggregate. Each time a decision is made that affects the process, at least three principal characteristics of this industry are required to be kept in mind: continuous processing, product alteration, and multiple products.

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## QUALITY CONTROL

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The process control techniques that are most desirable are predictive in nature rather than detective techniques that provide information on the product after the material has been stockpiled for shipping. Quality control is a system of maintaining standards in aggregate products by testing a sample of the finished product compared to the specification. Two principles of quality control that are required to be adhered to are:

1. Make sure the correct target is understood and achievable
2. Control variability within pre-established limits

Once the techniques for prediction of performance are developed, then quality control is required to address three issues: accuracy, precision, and capability.

## ACCURACY

If the average of all measurements falls relatively close to an understood point (on target) then the process is said to be accurate.

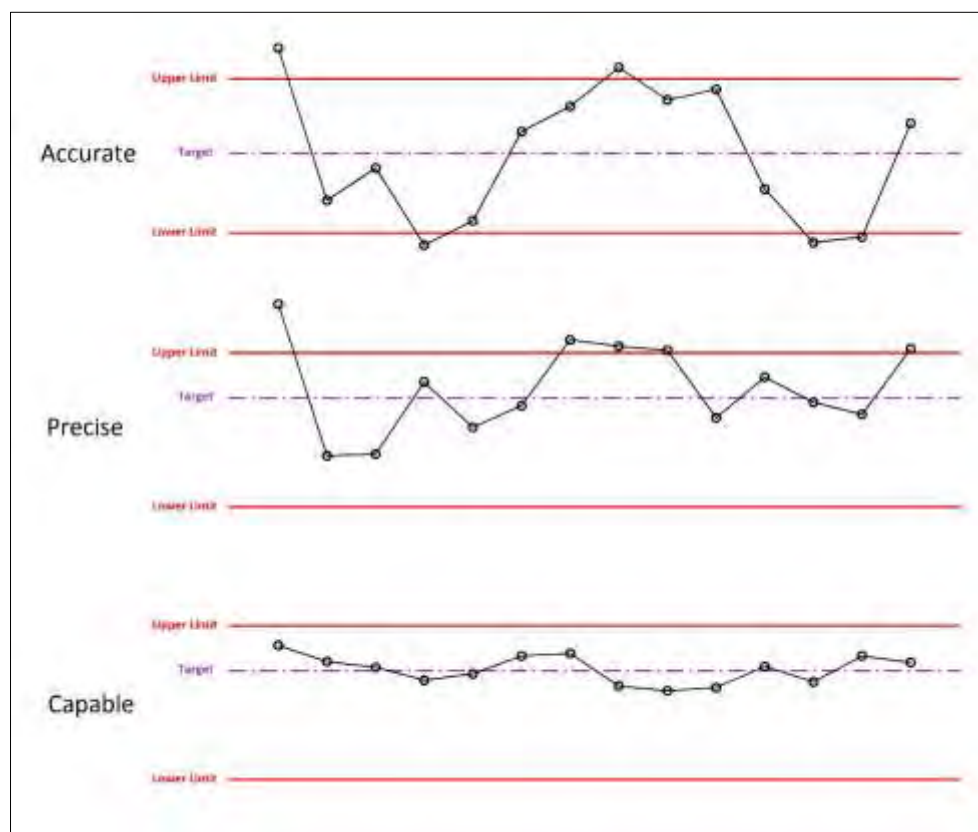
## PRECISION

When all the measurements over time are very close together, then the process is said to be precise.

## CAPABILITY

If the process is both accurate and precise such that the process remains within Specification or other predetermined limits with a high degree of confidence, then the process is said to be capable.

Figure 10.1 gives a graphical representation of accuracy, precision, and capability.



**Figure 10.1 Accuracy, Precision, and Capability**

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# UNDERSTANDING THE PROCESS

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Attempting to control an unstable process is like trying to answer an unsolvable riddle. Nothing is predictable; hence, nothing may be assumed. The Technician is required to follow a logical path to understand how, when, and what controls are necessary. The following path is a series of measurements, observations, communications, and decision-making.

## **CURRENT PROCESS**

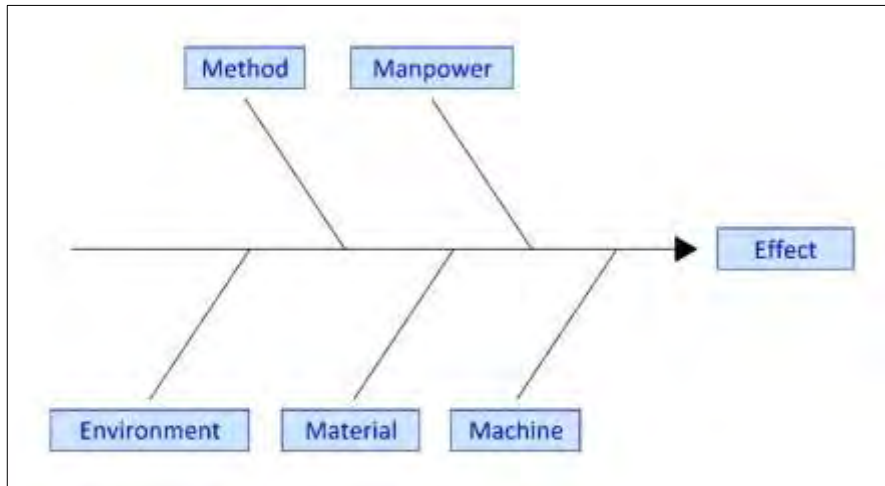
The current process is required to be thoroughly understood before making wise decisions on how to make improvements:

1. Gather honest employee input so that management knows what they know
2. Conduct and document visual observations of which elements seem to cause the greatest variability. Excavating or blasting, crushing, screening, total process stockpiling, hauling, handling, and loading are all items that may affect quality.
3. Learn how to make accurate measurements at uniform intervals over time. Apply statistical principles to determine current stability and capability of the process.

## **PROCESS STABILITY**

The process is required to first be stabilized before any other improvements may be made using the following procedures:

1. Identify the variables that most affect the process, called the Key Process Variables. These variables require the greatest attention from the operations managers (Figure 10.13).
2. Establish standards. The first reduction in variability may be recognized through "Standard Operating Procedures" (S.O.P.'s). These procedures include job descriptions, measurements (type and frequency), protocol for extreme conditions, etc.
3. Determine special causes. An absolute requirement for a stable process is the elimination of special causes of variability, namely, the ones that are external and not a part of the natural process. (e.g. conditions created by personnel)



**Figure 10.13 Key Process Variables**

## DECISION MAKING

After operating for some time with a stable process, some important decisions may be made. There are two items that are required to occur first:

1. Communicate with customers so they understand the new stable products. Also, obtain input from the customers on the need for further adjustment.
2. Make concurrent measurements to assess the need for further improvement

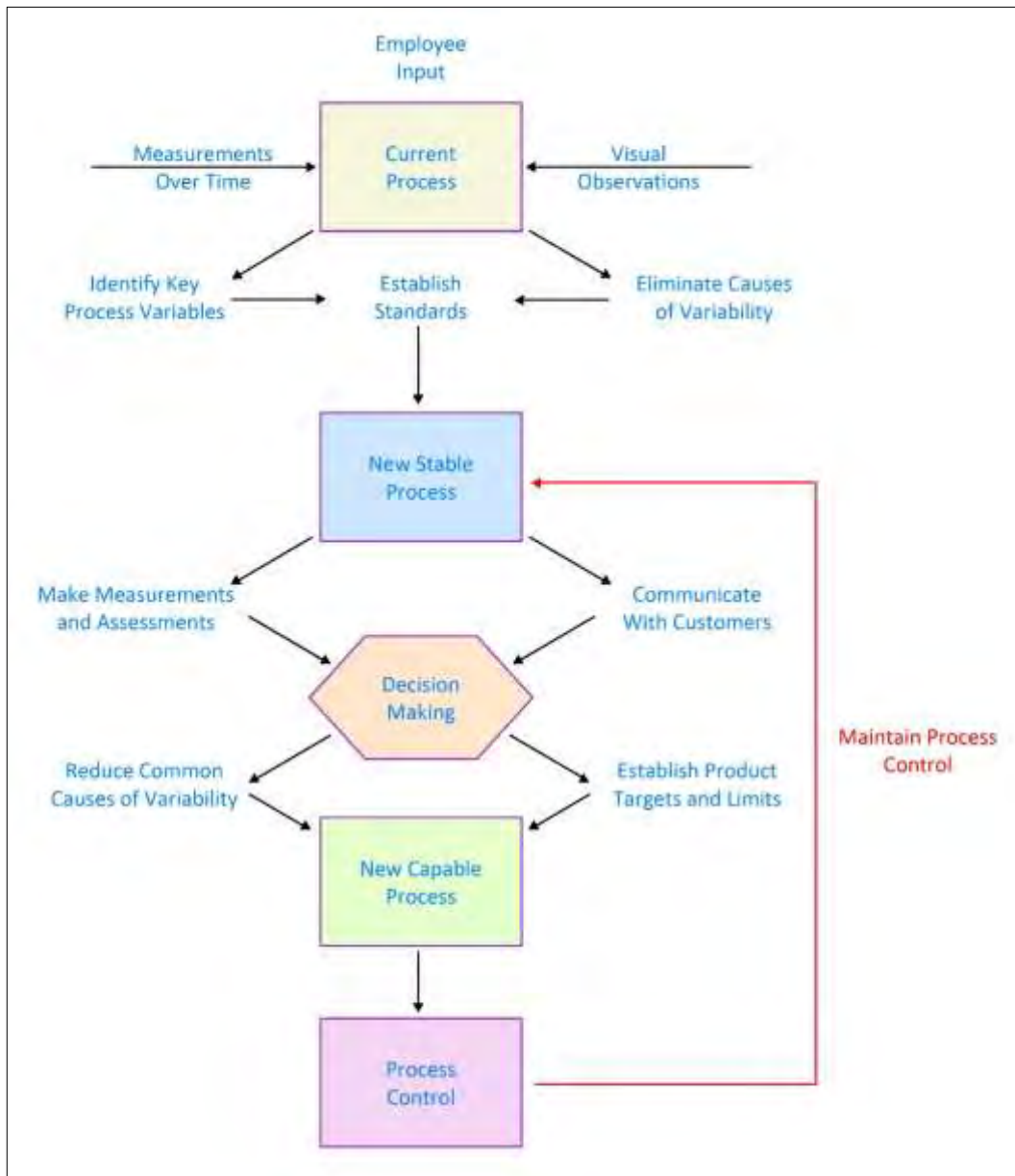
## PROCESS CAPABILITY

Decisions previously made are required to include any techniques needed to bring the process into desired compliance with a high degree of confidence such as:

1. Establish final desired product targets and limits
2. Reduce common causes of variability as required. This generally means a change in the process.

## PROCESS CONTROL

Implement ongoing statistical process control along with continuous improvement.



**Figure 10.14 Steps for Managing Quality**

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# PROCESS CHARACTERISTICS

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## **CONTINUOUS PROCESSING**

Generally, a continuous run of material is produced which tends to lose identity through stockpiling and shipping. Good controls as far upstream as possible in production are very important.

## **PRODUCT ALTERATION**

All aggregate products degrade and segregate with handling and time. This process occurs from beginning to the end of any production. This process may occur later, such as when the aggregates are used for producing other materials.

## **MULTIPLE PRODUCTS**

Most aggregate operations make more than one product concurrently. A change in one product may affect each and every one of the other products.

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# STATISTICAL CONCEPTS

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Complete control and improvements on any process is made by accurate measurements at critical points within the process. In order to gain confidence, the numbers are required to be generated often at various points so that all the variations of the process are detected. The quantity of measurements accumulates over time and simple tables or listings of these numbers are not enough to evaluate the process. The following statistical tools are used to understand what the numbers mean.

## **DATA SETS**

The numbers from measurements that represent something in common rather than a scattering

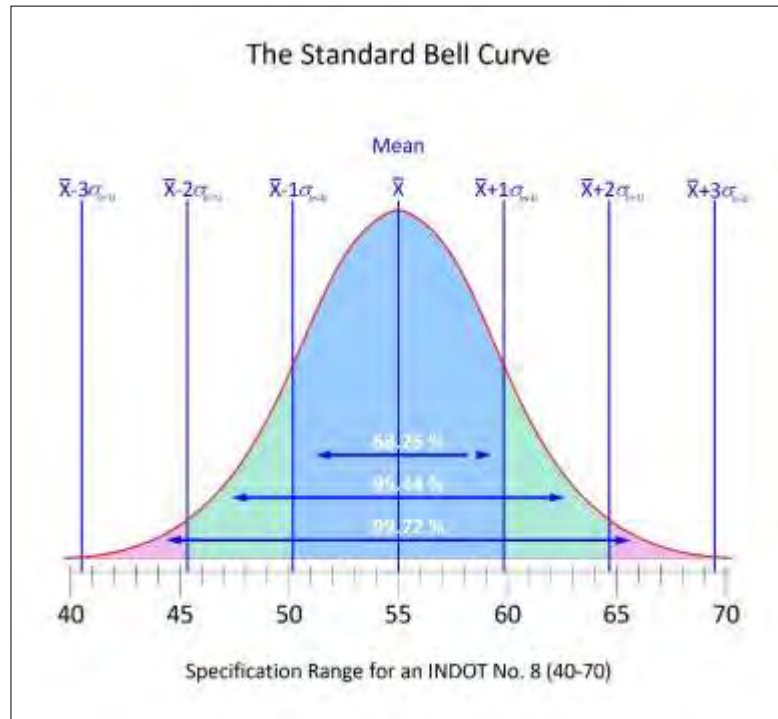
of unrelated numbers are called sets. When measuring properties of the process that are different, for example, gradation, crush count, or chert count, each property requires a set of numbers. Also, each property has different sets of numbers for different points in the process if the characteristics are known to change. (For example, production gradations versus stockpile gradations). Furthermore, even when properties and points of sampling are the same, a new set of data is required to be created if there is a significant sustained change in the process. All the efforts at understanding, controlling, and predicting the outcome of a process are only as good as the accuracy and make-up of the related data sets. The importance of this step should not be underestimated.

## THE MEAN

The average of all the data over time of an unchanged process is sometimes called the "grand mean" or the "population mean" and is represented by  $(\bar{x})$ . For a shorter snapshot in time, the average may be called the "local mean" or just the "mean". The mean  $(\bar{x})$  is the center of any distribution of numbers. Figure 10.2 is a graph of a very large group of numbers that are equally distributed on each side of the mean  $(\bar{x})$ . The graphical representation of these numbers is called a "standard bell curve".

In the initial stages of the CAP Program, the mean of the test data is represented by  $(\bar{x})$ . However, at some point the process will become stable, and a Target Mean will be declared. The target mean is represented by  $(\bar{x}_t)$ . Any mean calculated after establishing the target mean is referred to as the Current Mean and is represented by  $(\bar{x}_c)$ .





**Figure 10.2 Standard Bell Curve**

## STANDARD DEVIATION

Whereas the mean is an average of all the data values, the standard deviation ( $\sigma_{(n-1)}$ ) is an average value of the dispersion of data from the mean. The standard deviation ( $\sigma_{(n-1)}$ ) indicates how much the process varies and determines the shape of the bell curve. Small values reflect a tall, narrow curve, indicating that there is less of a spread in the data. Large values reflect a flat, broad curve, indicating that there is more of a spread in the data.

## NORMAL DISTRIBUTION

To simplify the interpretation of the data sets, the assumption is made that the data mathematically falls into a normal distribution which when plotted resembles the bell-shaped curve. Although few actual processes exactly follow this assumption, they are close enough when stable and in control to be useful statistically. By assuming a normal distribution of the data, a few simple formulas may be applied to give the desired picture of the process. Any area under the bell curve falling between certain limits from left to right when expressed as a percentage of the total indicate the portion of that process that conforms to those limits (Figure 10.2). The further data values move right or left from the center of the curve, the less often these values occur.

Some values that serve as handy reference points for the normal distribution are:

1. About two-thirds of the area under the normal curve lies between one standard deviation below the mean and one standard deviation above the mean
2. About 95 percent lies between plus and minus two standard deviations
3. About 99.75 percent lies within three standard deviations of the mean

## VARIABILITY

Conditions within control limits are required to be achieved for each critical characteristic and point in the process. Sources of variability for the same characteristic at different points in the process are cumulative. During the production, handling, and stockpiling of mineral aggregates, the sources of error are potentially many. Therefore, controls are required to be instituted upstream as well as throughout the process. Also, sampling and testing error may affect the variability. Although sampling and testing error will not affect the actual variation of the process, misleading information may cause incorrect control techniques to be employed and possibly increase variability in the product. The lower the sampling and testing error, the more indicative the data is of the process.

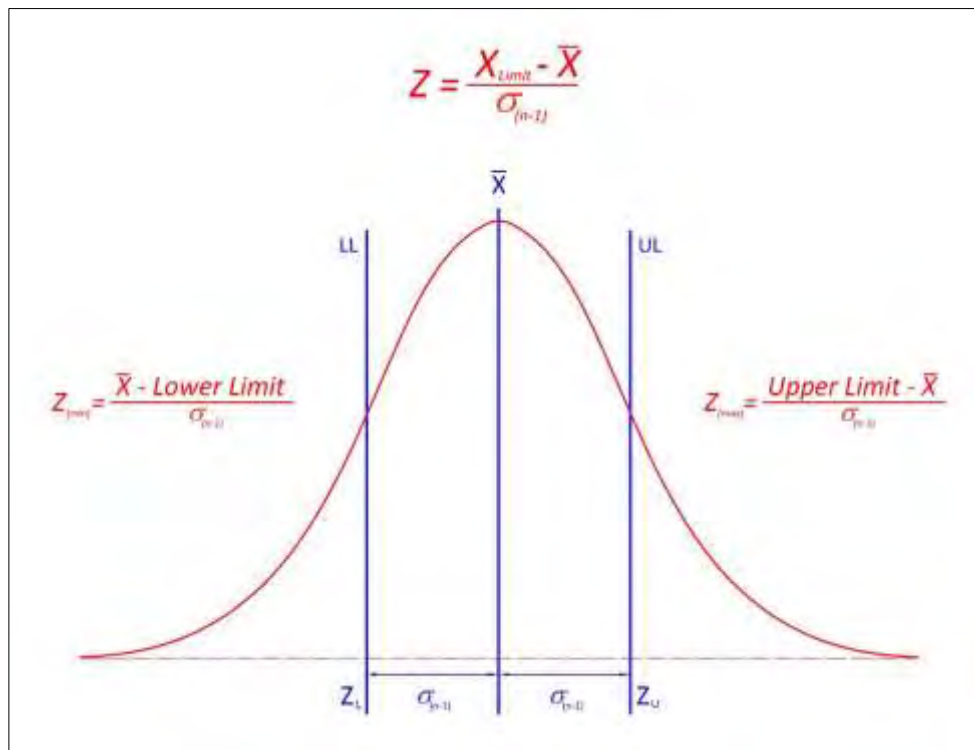
## CAPABILITY AND COMPLIANCE – THE Z-VALUE

The mean, standard deviation and variability indicate the location of the process and how consistent the process is. This is very important in exercising control. By themselves, however, they do not indicate how well the process meets certain specifications or other limits. The ability of the process to comply with externally imposed limits is called capability. The first useful tool in making this assessment is the Z-value. This value indicates the number of standard deviations that the mean is from a particular limit. The greater the Z-value, the more compliant or capable the process is (Figure 10.3).

There are two principal applications of the Z-value in the Certified Aggregate Producer Program (CAPP):

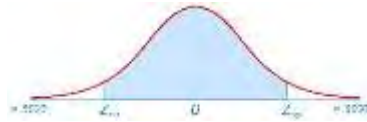
1. **Qualifying a Product** -- Before a critical sieve product may qualify for use under the CAP Program, the data generated during new product qualification testing is required to demonstrate a Z-Value of at least 1.65 or higher within the specification limits of that product.

2. **Control and Compliance** -- After qualification of a product, the Z-Value from the data generated during control and shipping is required to result in a compliance level of 95 % or better for all control sieve products within (a maximum of) 10 % above and 10 % below the *Target Mean* ( $\bar{x}_t$ ).



**Figure 10.3 Graphical Depiction of Z**

When the Z-values to each limit are known, this table indicates the area of probability between limits by summing the area to the left of ( $\bar{x}$ ) with the area right of ( $\bar{x}$ ). The sum of the two area factors should be multiplied by 100 to give the percent probability of compliance (Figure 10.4).



Z-Table										
Z	0.00	0.01	0.02	0.03	0.04	0.05	0.06	0.07	0.08	0.09
0.0	0.0000	0.0040	0.0080	0.0120	0.0159	0.0199	0.0239	0.0279	0.0319	0.0359
0.1	0.0398	0.0438	0.0478	0.0517	0.0557	0.0596	0.0636	0.0675	0.0714	0.0753
0.2	0.0793	0.0832	0.0871	0.0910	0.0948	0.0987	0.1026	0.1064	0.1103	0.1141
0.3	0.1179	0.1217	0.1255	0.1293	0.1331	0.1368	0.1406	0.1443	0.1480	0.1517
0.4	0.1554	0.1591	0.1628	0.1664	0.1700	0.1736	0.1772	0.1808	0.1844	0.1879
0.5	0.1915	0.1950	0.1985	0.2019	0.2054	0.2088	0.2123	0.2157	0.2190	0.2224
0.6	0.2257	0.2291	0.2324	0.2357	0.2389	0.2422	0.2454	0.2486	0.2518	0.2549
0.7	0.2580	0.2612	0.2642	0.2673	0.2704	0.2734	0.2764	0.2794	0.2823	0.2852
0.8	0.2881	0.2910	0.2939	0.2967	0.2995	0.3023	0.3051	0.3078	0.3106	0.3133
0.9	0.3159	0.3186	0.3212	0.3238	0.3264	0.3289	0.3315	0.3340	0.3365	0.3389
1.0	0.3413	0.3438	0.3461	0.3485	0.3508	0.3531	0.3554	0.3577	0.3599	0.3621
1.1	0.3643	0.3665	0.3686	0.3708	0.3729	0.3749	0.3770	0.3790	0.3810	0.3830
1.2	0.3849	0.3869	0.3888	0.3907	0.3925	0.3944	0.3962	0.3980	0.3997	0.4015
1.3	0.4032	0.4049	0.4066	0.4083	0.4099	0.4115	0.4131	0.4147	0.4162	0.4177
1.4	0.4192	0.4207	0.4222	0.4236	0.4251	0.4265	0.4279	0.4292	0.4306	0.4319
1.5	0.4332	0.4345	0.4357	0.4370	0.4382	0.4394	0.4406	0.4418	0.4430	0.4441
1.6	0.4452	0.4463	0.4474	0.4485	0.4495	0.4505	0.4515	0.4525	0.4535	0.4545
1.7	0.4554	0.4564	0.4573	0.4582	0.4591	0.4599	0.4608	0.4616	0.4625	0.4633
1.8	0.4641	0.4649	0.4656	0.4664	0.4671	0.4678	0.4686	0.4693	0.4699	0.4706
1.9	0.4713	0.4719	0.4726	0.4732	0.4738	0.4744	0.4750	0.4758	0.4762	0.4767
2.0	0.4773	0.4778	0.4783	0.4788	0.4793	0.4798	0.4803	0.4808	0.4812	0.4817
2.1	0.4821	0.4826	0.4830	0.4834	0.4838	0.4842	0.4846	0.4850	0.4854	0.4857
2.2	0.4861	0.4865	0.4868	0.4871	0.4875	0.4878	0.4881	0.4884	0.4887	0.4890
2.3	0.4893	0.4896	0.4898	0.4901	0.4904	0.4906	0.4909	0.4911	0.4913	0.4916
2.4	0.4918	0.4920	0.4922	0.4925	0.4927	0.4929	0.4931	0.4932	0.4934	0.4936
2.5	0.4938	0.4940	0.4941	0.4943	0.4945	0.4946	0.4948	0.4949	0.4951	0.4952
2.6	0.4953	0.4955	0.4956	0.4957	0.4959	0.4960	0.4961	0.4962	0.4963	0.4964
2.7	0.4965	0.4966	0.4967	0.4968	0.4969	0.4970	0.4971	0.4972	0.4973	0.4974
2.8	0.4974	0.4975	0.4976	0.4977	0.4977	0.4978	0.4979	0.4980	0.4980	0.4981
2.9	0.4981	0.4982	0.4983	0.4984	0.4984	0.4984	0.4985	0.4985	0.4986	0.4986
3.0	0.4986	0.4987	0.4987	0.4988	0.4988	0.4988	0.4989	0.4989	0.4989	0.4990
3.1	0.4990	0.4991	0.4991	0.4991	0.4992	0.4992	0.4992	0.4992	0.4993	0.4993
3.2	0.4993	0.4993	0.4994	0.4994	0.4994	0.4994	0.4994	0.4995	0.4995	0.4995
3.3	0.4995	0.4995	0.4995	0.4996	0.4996	0.4996	0.4996	0.4996	0.4996	0.4997
3.4	0.4997	0.4997	0.4997	0.4997	0.4997	0.4997	0.4997	0.4997	0.4997	0.4998
3.5	0.4998	0.4998	0.4998	0.4998	0.4998	0.4998	0.4998	0.4998	0.4998	0.4998
3.6	0.4998	0.4998	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999
3.7	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999
3.8	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999	0.4999
3.9	0.5000	0.5000	0.5000	0.5000	0.5000	0.5000	0.5000	0.5000	0.5000	0.5000

Figure 10.4 Area of Probability Table (Z-Table)

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# CONTROL CHARTING

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Controlling a process with just one measurement is not possible. Also, only a few measurements do not provide the level of confidence needed for proper decision-making and a clear picture of the process. The only way control and decisions may be made with confidence is through the use of large data sets. The control chart is a process that may be used on a daily basis to guide the Aggregate Producer. Graphic representation of the data, in conjunction with prescribed limits, may provide the Aggregate Producer with everything that is needed if used with the proper interpretation techniques (ITM 211 section 13.0)

## WHEN TO USE CHARTS

INDOT requires that gradation control charts be maintained for most products made by a certified plant for use on INDOT contracts. These charts may be kept electronically. Any characteristic critical to a product is a candidate for control charting. For example, crush count, chert count, or any other characteristics that may apply are considered for charting. In these cases, the items considered, and the proposed limits are required to be included in the Quality Control Plan (QCP) submitted to INDOT for approval.

## CONTROL CHART LEGEND

CAPP establishes a legend for specific information to be plotted on control charts. This legend convention is required to be followed, except that any proposed deviation from the procedures may be clearly identified in the Quality Control Plan.

The target mean is represented by a heavy long dash followed by a short dash.

Target Mean ( $\bar{x}_t$ )      — . — . — . — . — . — . — . —

Control limits are represented by heavy solid lines placed at plus and minus two standard deviations, but no greater than plus or minus 10 percent from the target mean.

Upper Control Limit (UCL)



Lower Control Limit (LCL)



Upper and lower specification limits are indicated by short, dashed lines.

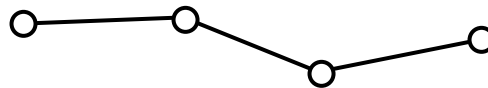
Upper Specification Limit (USL)



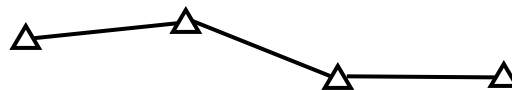
Lower Specification Limit (LSL)



Production plot points are surrounded by small circle and each consecutive point is connected by a solid straight line.



The 5-point moving average plot point is indicated by a small triangle and connected by straight lines.



The stockpile load-out plot point is indicated by a small square.



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# QUALIFYING A PRODUCT

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## SETTING A TARGET – 1.65 RULE

Control charts indicate constant accuracy and precision if the process is in control and repeatable. The scattering of individual data points gives a feel for precision or variability of the process when viewed against the control limits. In addition, a running average of the most current five data points is required to track the accuracy of the process. Averages tend to lessen the effect of erratic data points that could reflect errors not related to the actual material (sampling, testing, etc.) and that distract the viewer away from trends comparing to the target mean. Although this technique is not as accurate as data points that are each comprised of averages of subsets and which require an accompanying chart of ranges, the process works well for the mineral aggregate industry. When aggregates are tested at frequencies of 2000 tons per sample, the requirement to wait for the accumulation of five tests before generating a single data point is not acceptable.

The principal purpose of the control chart is to visually depict a repeatable and controlled process. If the new data is expected to be part of the process population, then some definition of the process is required. The entire chart is (eventually) centered around the target mean ( $\bar{x}_t$ ) value. Ideally, the target mean is the grand mean which would be based on as much data as possible (perhaps a year), providing the process has not changed. If valid data does not exist on the process, then the control chart is established around a mean ( $\bar{x}$ ) calculated from the first ten test results. The CAPP requires a QCP Annex to the Quality Control Plan (QCP) identifying the new target mean to be filed with INDOT. Next, control limits are required to be added at plus and minus two standard deviations from the target mean. In no case may these limits exceed plus and minus 10 %.

A quick check of the location of the target mean in relation to the closest specification limit is to multiply 1.65 times the standard deviation. To calculate the upper specification limit (USL) and lower specification limit (LSL), add or subtract the 1.65 times the standard deviation to the proposed target mean ( $\bar{x}_t$ ). If the resultant number falls outside the specification band, the current process does not meet the requirements of CAPP.

$$\text{Upper Specification Limit (USL)} = (\bar{x}_t) + 1.65(\sigma_{(n-1)})$$

$$\text{Lower Specification Limit (LSL)} = (\bar{x}_t) - 1.65(\sigma_{(n-1)})$$

The upper Z-value ( $Z_{\max}$ ) is checked by subtracting the mean ( $\bar{x}$ ) from the upper specification limit (USL) and dividing the answer by the standard deviation ( $\sigma_{(n-1)}$ ). The answer should be greater than 1.65 for the upper Z-value ( $Z_{\max}$ ) check to be an acceptable value.

$$Z_{\max} = \frac{\text{USL} - \bar{x}}{\sigma_{(n-1)}} > 1.65 = \text{Acceptable}$$

The lower Z-value ( $Z_{\min}$ ) is checked by subtracting the lower specification limit (LSL) from the mean ( $\bar{x}$ ) and then dividing by the standard deviation ( $\sigma_{(n-1)}$ ). The answer should be greater than 1.65 for the lower Z-value ( $Z_{\min}$ ) check to be an acceptable value.

$$Z_{\min} = \frac{\bar{x} - \text{LSL}}{\sigma_{(n-1)}} > 1.65 = \text{Acceptable}$$

The check of the specification limits and establishment of the control limits for Figure 10.5 are conducted as follows.

## EXAMPLES FOR SETTING A TARGET MEAN & CONTROL LIMITS

For the 3/8 in. critical sieve for the INDOT Standard Specifications No. 9 material:

### 1) Data Set Statistics

Number of tests ( $n$ ) = 25  
Mean ( $\bar{x}$ ) = 39.6  
Standard Deviation ( $\sigma_{(n-1)}$ ) = 4.05

### 2) Specification Limits Check

$$1.65(\sigma_{(n-1)}) = 1.65(4.05) = 6.7$$

$$\text{USL} = \bar{x} + 1.65(\sigma_{(n-1)}) = 39.6 + 6.7 = 46.3 \approx 46 \leq 60$$

$$\text{LSL} = \bar{x} - 1.65(\sigma_{(n-1)}) = 39.6 - 6.7 = 32.9 \approx 33 \geq 30$$

### 3) Z-Value Check

$$Z_{\max} = \frac{\text{USL} - \bar{x}}{\sigma_{(n-1)}} = \frac{60.0 - 39.6}{4.05} = \frac{20.4}{4.05} = 5.04 > 1.65 = \text{Acceptable}$$



$$Z_{\min} = \frac{\bar{x} - \text{LSL}}{\sigma_{(n-1)}} = \frac{39.6 - 30.0}{4.05} = \frac{9.6}{4.05} = 2.37 > 1.65 = \text{Acceptable}$$

4) Declare Target Mean ( $\bar{x}_t$ ) = 39.6

5) Establish Control Limits

$$\text{Upper Control Limit (UCL)} = \bar{x} + 2(\sigma_{(n-1)}) = 39.6 + 2(4.05) = 47.7 \text{ or } \mathbf{48}$$

$$\text{Lower Control Limit (LCL)} = \bar{x} - 2(\sigma_{(n-1)}) = 39.6 - 2(4.05) = 31.5 \text{ or } \mathbf{32}$$

So, moving forward, these are the critical sieve parameters that will need to be annexed into the QCP:

Target Mean ( $\bar{x}_t$ ) = 39.6

Upper Control Limit (UCL) = 48

Lower Control Limit (LCL) = 32

**Material Size: INDOT No. 9**

**Plant: Indiana**

**Test Results Data**

Test #	Date	100 3/4 in.	60-85 1/2 in.	30-60 3/8 in.	0-15 No. 4	0-10 No. 8
1	3/19	100.0	68.9	38.4	4.9	2.3
2	3/19	100.0	71.2	40.8	5.2	2.9
3	3/25	100.0	70.8	36.4	3.3	2.8
4	3/25	100.0	69.8	35.2	4.5	3.6
5	3/27	100.0	69.2	37.7	3.9	2.2
6	3/31	100.0	66.3	36.9	3.3	2.1
7	3/31	100.0	70.1	40.1	3.9	2.5
8	4/6	100.0	68.0	37.2	3.6	2.8
9	4/6	100.0	69.7	34.1	3.5	2.8
10	4/8	100.0	71.6	35.1	3.0	1.9
11	4/8	100.0	70.9	37.5	3.7	2.6
12	4/11	100.0	74.8	46.0	4.0	3.1
13	4/15	100.0	77.4	42.9	3.9	1.8
14	4/17	100.0	80.3	49.2	4.9	3.1
15	4/17	100.0	74.0	34.5	3.9	2.4
16	4/20	100.0	73.4	35.4	2.9	1.9
17	4/20	100.0	79.3	40.1	4.4	3.0
18	4/21	100.0	77.5	39.7	4.0	3.2
19	4/21	100.0	78.4	43.1	3.7	2.1
20	4/22	100.0	75.2	39.7	3.6	2.3
21	4/24	100.0	80.9	45.1	4.5	1.9
22	4/24	100.0	80.4	46.5	4.6	2.3
23	4/25	100.0	75.5	38.5	3.5	1.9
24	4/30	100.0	77.2	38.0	5.8	3.6
25	4/30	100.0	76.8	42.2	3.3	2.2
<b>Mean</b>		100.0	73.9	39.6	4.0	2.5
<b>Standard Deviation</b>		0.00	4.34	4.05	0.71	0.54

**Figure 10.5 Historical Data Gradation Analysis**

Control Chart Setup  
INDOT Standard Specification No. 9  
( Critical Sieve = 3/8 inch )

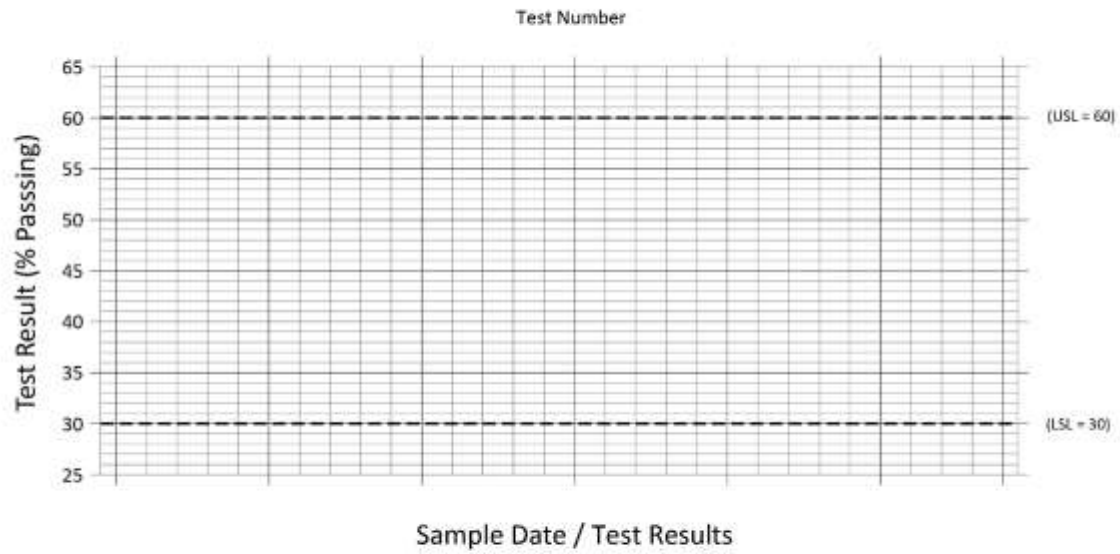
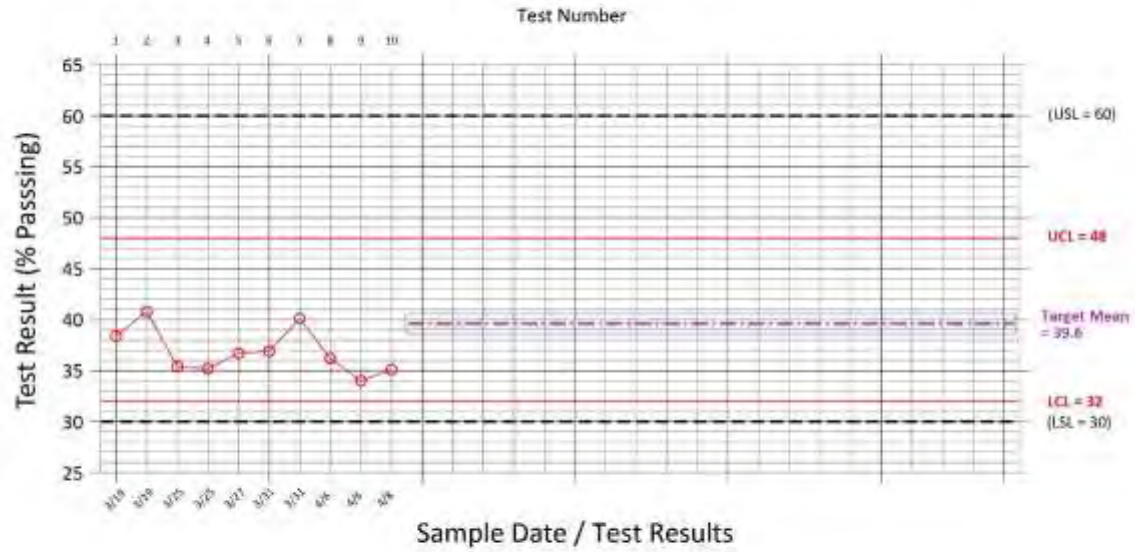


Figure 10.6 Control Chart Setup

**Control Chart Setup**  
**INDOT Standard Specification No. 9**  
 ( Critical Sieve = 3/8 inch )



**Figure 10.7 Control Chart Setup**

The check of the specification limits and establishment of the control limits for Figure 10.8 are conducted as follows.

For the No. 4 critical sieve for the INDOT Standard Specifications No. 11 material:

1) Data Set Statistics

Number of tests ( $n$ ) = 36  
 Current Mean ( $\bar{x}_c$ ) = 14.8  
 Standard Deviation ( $\sigma_{(n-1)}$ ) = 2.57

2) Specification Limits Check

$$1.65(\sigma_{(n-1)}) = 1.65(2.57) = 4.2$$

$$USL = \bar{x} + 1.65(\sigma_{(n-1)}) = 14.8 + 4.2 = 19.0 \approx \mathbf{19} \leq 30 \quad OK$$

$$LSL = \bar{x} - 1.65(\sigma_{(n-1)}) = 14.8 - 4.2 = 10.6 \approx \mathbf{11} \geq 10 \quad OK$$

3) Z-Value Check

$$Z_{\max} = \frac{USL - \bar{x}}{\sigma_{(n-1)}} = \frac{30.0 - 14.8}{2.57} = \frac{15.2}{2.57} = \mathbf{5.91} > 1.65 = \textit{Acceptable}$$

$$Z_{\min} = \frac{\bar{x} - LSL}{\sigma_{(n-1)}} = \frac{14.8 - 10.0}{2.57} = \frac{4.8}{2.57} = \mathbf{1.87} > 1.65 = \textit{Acceptable}$$

4) Establish Control Limits

$$\text{Upper Control Limit (UCL)} = \bar{x} + 2(\sigma_{(n-1)}) = 14.8 + 2(2.57) = 19.9 \text{ or } \mathbf{20}$$

$$\text{Lower Control Limit (LCL)} = \bar{x} - 2(\sigma_{(n-1)}) = 14.8 - 2(2.57) = 9.7 \text{ or } \mathbf{10}$$

So, this data set is valid, and the target mean may now be declared, and control limits may be established:

Target Mean ( $\bar{x}_t$ ) = 14.8  
 Upper Control Limit (UCL) = 20  
 Lower Control Limit (LCL) = 10

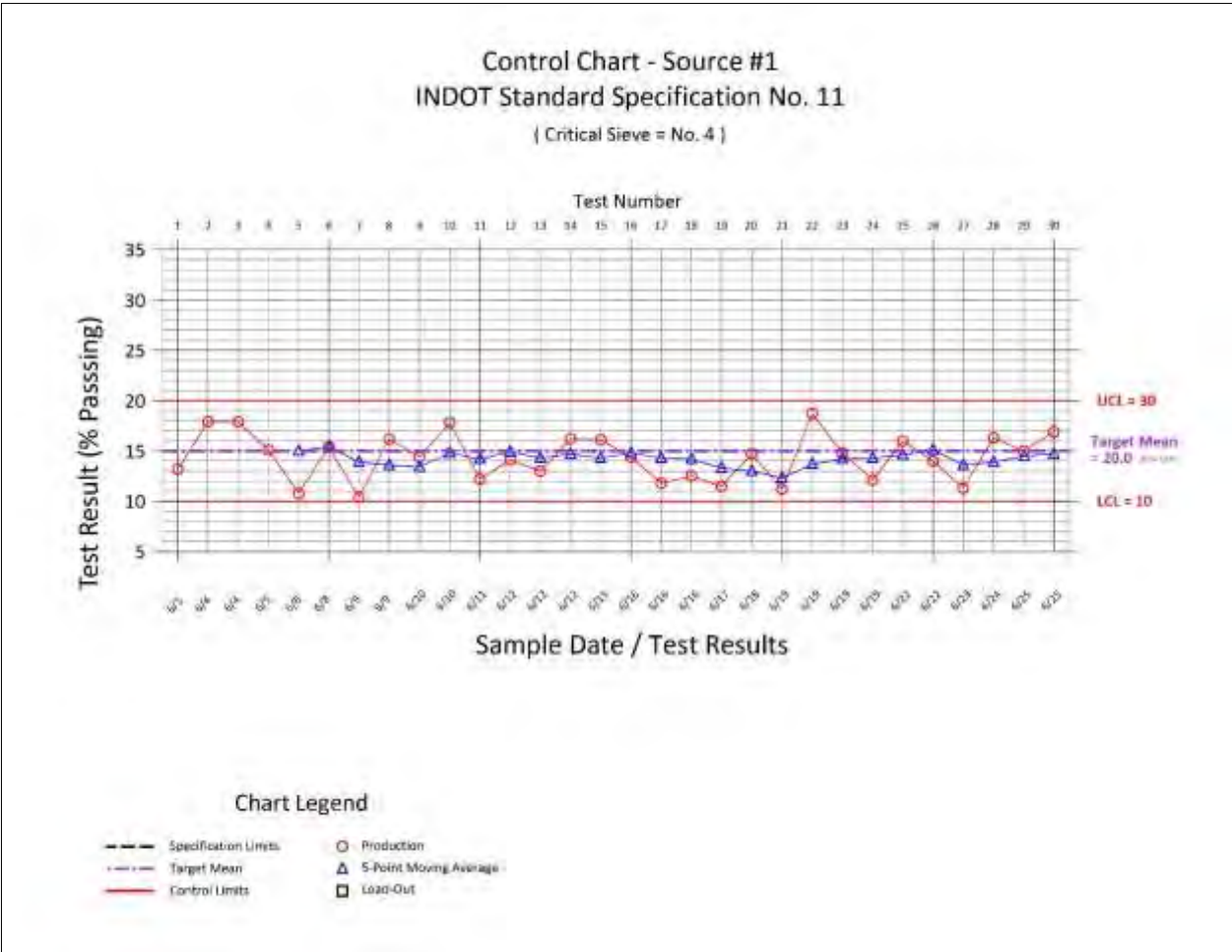
## PLOTTING THE DATA

Figure 10.8 and Figure 10.9 illustrate how data points and the running average for a product critical sieve are plotted on a control chart with an established target mean and control limits.

**Material Size: INDOT No. 11****Plant: Indiana****Test Results Data**

Test #	Date	100 1/2 in.	75-95 3/8 in.	10-30 No. 4	5-Point MA No. 4	0-10 No. 8
1	6/3	100.0	87.5	13.1		3.3
2	6/4	100.0	86.7	17.9		4.4
3	6/4	100.0	90.8	17.9		6.1
4	6/5	100.0	85.9	15.1		5.7
5	6/8	100.0	87.1	10.8	15.0	3.9
6	6/8	100.0	89.6	15.4	15.4	5.1
7	6/9	100.0	84.8	10.4	13.9	3.9
8	6/9	100.0	84.8	16.2	13.6	3.8
9	6/10	100.0	85.2	14.4	13.4	4.9
10	6/10	100.0	88.9	17.8	14.8	3.1
11	6/11	100.0	86.2	12.2	14.2	4.4
12	6/12	100.0	87.2	14.1	14.9	5.3
13	6/12	100.0	86.0	13.0	14.3	4.9
14	6/12	100.0	87.7	16.2	14.7	4.5
15	6/15	100.0	82.0	16.1	14.3	4.2
16	6/16	100.0	88.3	14.4	14.8	5.4
17	6/16	100.0	89.7	11.8	14.3	3.5
18	6/16	100.0	89.4	12.5	14.2	4.7
19	6/17	100.0	86.2	11.5	13.3	2.9
20	6/18	100.0	86.1	14.7	13.0	4.3
21	6/19	100.0	88.5	11.2	12.3	5.4
22	6/19	100.0	86.0	18.7	13.7	3.3
23	6/19	100.0	87.4	14.8	14.2	5.8
24	6/19	100.0	87.5	12.1	14.3	3.3
25	6/22	100.0	85.9	16.0	14.6	4.9
26	6/22	100.0	96.3	14.0	15.1	4.5
27	6/23	100.0	86.9	11.3	13.6	3.7
28	6/24	100.0	88.5	16.3	13.9	4.2
29	6/25	100.0	88.6	15.0	14.5	5.0
30	6/25	100.0	89.5	16.9	14.7	5.5
31	6/26	100.0	86.6	13.9	14.7	5.0
32	6/29	100.0	87.9	14.7	15.4	5.1
33	6/29	100.0	89.6	16.7	15.4	6.2
34	6/30	100.0	90.1	18.2	16.1	8.8
35	6/30	100.0	92.3	21.8	17.1	8.3
36	6/30	100.0	90.7	14.0	17.1	4.1
Mean		100.0	87.8	14.8		4.8
Standard Deviation		0.00	2.50	2.57		1.27

**Figure 10.8 Gradation Analysis**



**Figure 10.9 Good Process Control**

## INTERPRETING CONTROL CHARTS

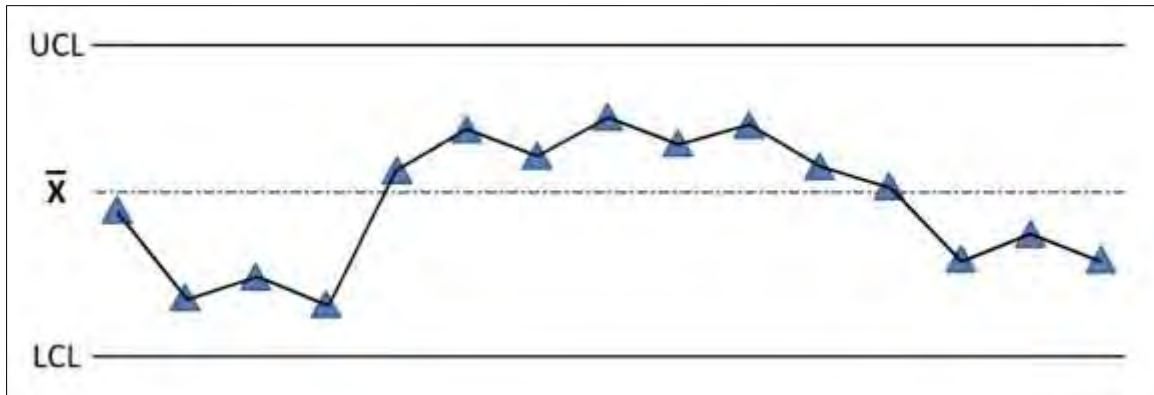
**ITM 211** requires that non-conforming normal production or load-out tests be followed immediately by a corrective action to include as a minimum: an investigation for assignable cause, correction of known assignable cause, and retesting. These retests are not plotted on the control charts.

Action is required to be taken after the first nonconforming test (outside of control limits). These requirements are required to be met in all cases and take precedence over any other control technique. When individual test results, even on an intermittent basis, frequently fall outside the control limits or specification limits, a nonconforming condition exists. A compliance calculation in conjunction with whichever limits are being violated may quickly verify the condition. The following trends involving the 5-point moving average points (Figure 10.10) may require investigation by the Producer and as a minimum an entry in the diary to denote the problem.

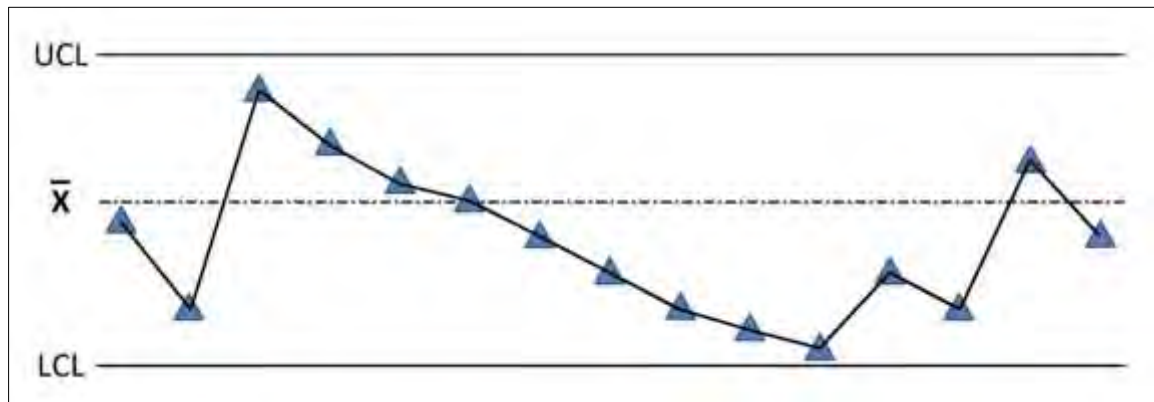
1. Seven or more points in a row are above or below the target mean ( $\bar{x}_t$ )
2. Seven or more points in a row are consistently increasing or decreasing

Finally, the Technician is required to always be alert for a sudden jump in the data, whether the data remains in control or not. This condition usually represents the addition of a completely different process and may be detected immediately without waiting for trends in the moving average (Figure 10.10). Corrective action is required to be taken immediately. If the shift to a new process is done intentionally, then a clean break is required to be made in the control chart by means of a vertical line on the chart. After ten valid test results on the new process, a new target mean ( $\bar{x}_t$ ) is required to be calculated and new control limits established (Figure 10.11 and Figure 10.12).





7 or more points above or below the Target Mean ( $\bar{x}_t$ )



7 or more points in a row are increasing or decreasing

**Figure 10.10 5-Point Moving Average Trends**

**Test Results (% Passing)**

**Test Number**

**Sample Date**

**Chart Legend**

- Specification Limits
- Control Limits
- Target Mean
- 5-Point Moving Average
- Production
- Load-Out

USL = 70  
LSL = 40  
UCL = 58  
LCL = 38  
Target Mean = 48.2

Process Change

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Source Code #2001  
 INDOT Standard Specification No. 8  
 ( Critical Sieve = 1/2 inch )

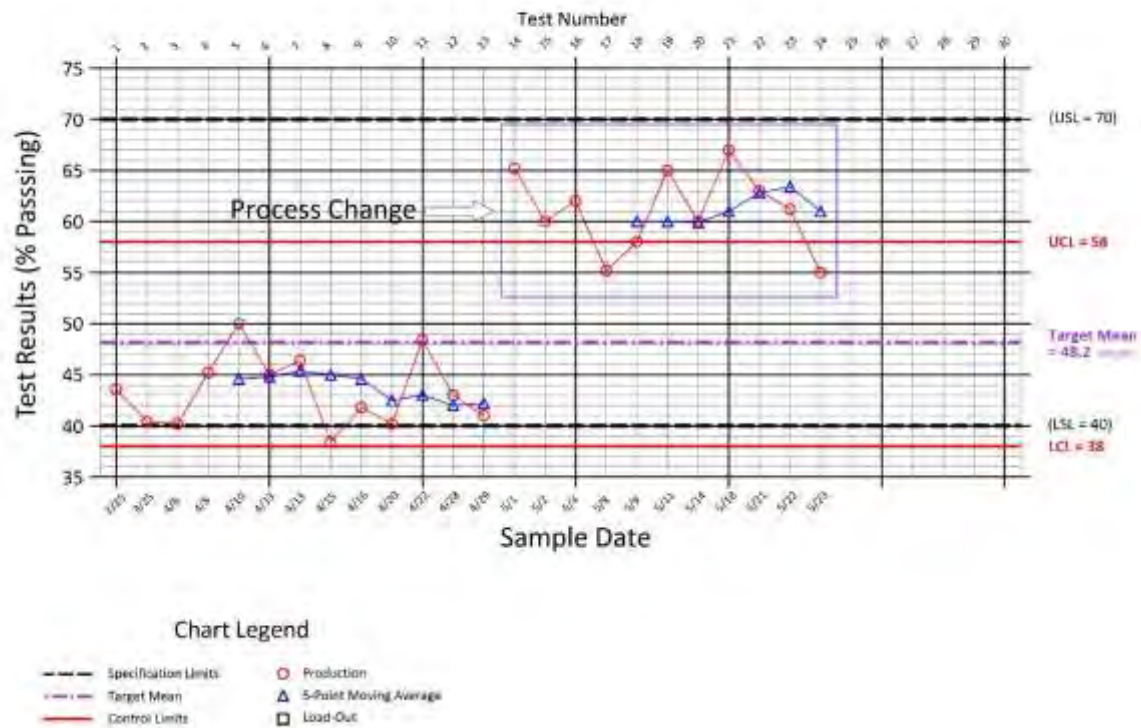


Figure 10.12 Process Adjustment

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# COMPLIANCE

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The CAP Program requires that 95 % of all gradation test results on the critical sieve statistically be between 10 % below and 10 % above the target mean at any one point of sampling. A compliance calculation of less than 95% is indicative of a nonconforming condition and will need to be investigated. An example of how to calculate percent compliance is as follows:

Product: #8 Stone

Critical Sieve: 1/2 in.

QCP Target Mean ( $\bar{x}_t$ ): 52.2%

The most recent 30 production sample test results:

<u>55.5</u>	<u>48.1</u>	<u>65.7</u>	<u>55.2</u>	<u>52.8</u>	<u>49.7</u>
<u>51.2</u>	<u>49.4</u>	<u>50.3</u>	<u>56.1</u>	<u>53.6</u>	<u>54.6</u>
<u>53.2</u>	<u>46.0</u>	<u>49.5</u>	<u>59.1</u>	<u>52.6</u>	<u>58.1</u>
<u>56.4</u>	<u>42.4</u>	<u>50.8</u>	<u>55.6</u>	<u>52.1</u>	<u>56.4</u>
<u>54.2</u>	<u>53.1</u>	<u>50.5</u>	<u>53.8</u>	<u>61.3</u>	<u>50.9</u>

## 1) Data Set Statistics

Number of tests ( $n$ )= 30

Current Mean ( $\bar{x}_c$ ) = 53.3

Standard Deviation ( $\sigma_{(n-1)}$ ) = 4.53

## 2) Calculate $Z_{\max}$ and $Z_{\min}$

$$Z_{\max} = \frac{(QCP \text{ Target Mean } (\bar{x}_t) + 10) - \text{Current Mean } (\bar{x}_c)}{\sigma_{(n-1)}} = \frac{(52.2 + 10) - 53.3}{4.53} = \mathbf{1.96}$$

$$Z_{\min} = \frac{\text{Current Mean } (\bar{x}_c) - (QCP \text{ Target Mean } (\bar{x}_t) - 10)}{\sigma_{(n-1)}} = \frac{53.3 - (52.2 - 10)}{4.53} = \mathbf{2.45}$$

## 3) Calculate $P_{\max}$ and $P_{\min}$

$$P_{\max} = \text{Probability (from Z Table)} \times 100 = .4750 \times 100 = \mathbf{47.50}$$

$$P_{\min} = \text{Probability (from Z Table)} \times 100 = .4929 \times 100 = \mathbf{49.29}$$

## 4) Calculate % Compliance

$$\% \text{ Compliance} = (P_{\max} \times 100) + (P_{\min} \times 100) = 47.50 + 49.29 = 96.79 \approx \mathbf{97} \text{ (Whole Number)}$$

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# LOADOUT MATERIAL REQUIREMENTS & PERCENT WITHIN LIMITS

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## **ITM 211-2026 LOADOUT SAMPLING AND TESTING**

### Loadout Sampling and Testing

Historically, CAPP was primarily focused on sampling, testing and statistical analysis with respect to production data. While this is generally still the case, the 2026 update to ITM 211 has added significant emphasis to loadout sampling, testing and statistical analysis.

Loadout sampling, which is also referred to as stockpile or shipping sampling, refers to sampling and testing material that is taken directly from the stockpile where material is sold to customers. This is different than production sampling, which is usually taken directly from material as it is being produced. Loadout sampling is a more realistic representation of the final product as it tests the material being loaded into customer trucks and shipped off to a job site. One of the main reasons sampling and testing loadout material is important is because the process of stockpiling aggregates can have significant effects on the gradation of the material. Segregation, degradation and contamination can all have effects on the final gradation of material that is shipped from the stockpile. These changes can be significant enough that material passing gradation when tested during production then fails when sampled at the point of loadout. Ultimately, loadout sampling and testing is critical in understanding changes to the material from the point of production through the stockpiling and shipping process and ensuring quality material is being sold from the source.

In previous versions of ITM 211, loadout sampling and testing was fairly limited in scope. Certified materials needed to be sampled and tested from the actively shipping stockpile a minimum of once every 8000 tons of material shipped. If the result of the loadout test was outside of the established control limits identified in the QCP for Quality Assurance materials, or outside the specification limits for products controlled by Standard Specification requirements (nonconforming test), a corrective action would be required. Results would be charted but no statistical analysis was required under CAPP.

## ITM 211-26 Changes

As mentioned, significant changes to the CAPP loadout program were made in the 2026 update to ITM 211. The first of which details the frequency for which loadout testing occurs. Table 10.13 (from 11.3.3 in ITM 211-26) details the revised loadout sampling requirements.

<b>Load-Out Frequency</b>	
<b>Shipping Tons/Year</b>	<b>Frequency</b>
< 100 t/year	No samples required
≥ 100 t/year	1 / ≤ 8000 t
Notes on Frequency:	
<p>1. Begin every calendar year with at least one load-out sample within the first 1000 tons. The remaining samples shall be at the QCP established frequency tonnage interval, calculated beginning at zero tons for the calendar year.</p> <p>2. No daily, monthly, or annual limit on the number of tests.</p> <p>3. Sample within ±1000 t shipped or ±1 business day at the completion of each full interval.</p> <p>4. Report and plot <u>all</u> QCP scheduled frequency test results, even when corrective action is later taken based on the test results.</p> <p>5. Track tonnage continuously throughout the year for determination of the next sample. “Reset” the sampling tonnage to zero on January 1 of each calendar year.</p>	
<p>For example, if the QCP for a product sets the frequency at 8000 tons, then: Obtain the first sample of the calendar year between 0 and 1000 tons. Obtain the second sample of the calendar year at 8000+/-1000 tons, the third sample of the calendar year at 16,000+/-1000 tons, and so on.</p>	

**Table 10.13:** ITM 211-26 Loadout Sampling Requirements

The first change to understand is related to sampling frequency. Any certified material that ships less than 100 tons per year does not require any sampling. Any certified material that ships more than 100 tons per year will require sampling and testing. For all such products, the first sample must be taken within the first 1000 tons of material shipped. This could be as low as zero tons, before shipping begins, or right at 1000 tons shipped as long as the first sample falls within that interval. From there, loadout sampling and testing is conducted on a set interval of a minimum of one sample per 8000 tons. Sources can name smaller sampling intervals in the QCP (2000-, 4000- or 6000-ton intervals would be acceptable, for example) but this cannot exceed 8000 tons. It is important to note that the initial sample taken does not affect the tonnage of the first 'on frequency' sample. For example, if the selected sampling interval is 4000 tons, the first test needs to be taken in the first 1000 tons and the target tonnage for the second test will be 4000 tons, not 5000. These new sampling rules apply to all CAPP products. Each loadout sample being tested will be decanted, as in previous versions of ITM 211.

An additional clarification that was introduced in ITM 211-26 is that shipping tonnage is tracked continuously throughout each year, resetting to 0 on January 1<sup>st</sup> of each calendar year. There is no daily, monthly or annual limit to the number of loadout tests. This means that there are no circumstances where a sample on the established interval should be skipped-the frequency should be adhered to no matter how many samples have been taken prior.

Samples must be taken within 1000 tons of the established interval or within one business day of when that shipping interval occurs. This requirement is intended so that producers have a practical window for acquiring loadout samples while maintaining a regular, uniform frequency.

Lastly, changes have been made to plotting requirements. The intent here is that all regularly scheduled on-frequency loadout tests are always reported and plotted on control charts. If there are retests or investigative tests after the loadout sample is tested, these do not replace the test that is reported and charted.

## **PERCENT WITHIN LIMITS**

Another significant change to ITM 211-26 is the addition of statistical analysis on loadout tests. Percent Within Limits (PWL) is a statistical measure of the percentage of a product's population that falls within specified upper and lower limits. Conceptually, PWL is very similar to Percent Compliance (also a measure of a percentage of a product's population that falls within specified upper and lower limits) which is used for statistical analysis on production test results. PWL calculations even follow many of the same steps as one would use to calculate Percent Compliance

using production test results. Since CAPP does not require producers to calculate PWL on their own, details on how to do the calculation are not included in this manual. A description of how to calculate PWL can be found in ITM 588.



**Figure 10.14:** Percent Within Limits Visual Schematic

(Image from Wisconsin Asphalt Pavement Association: <https://www.wispave.org/executive-update-2016-wapa-conference-and-awards-design-guide-percent-within-limits/>)

PWL is also not required to be calculated for all certified products. Only #8, #11 and QA equivalent materials require reporting gradation data for PWL. In contrast to the 95 percent compliance requirement for all critical sieve certified materials during production testing, the minimum allowable PWL for #8, #11 and QA equivalents being loadout tested is 90 percent. This requirement only goes into effect once there are at least five loadout tests reported. While there is no limit to the number of points that will be recorded, PWL is only calculated on the most recent 30 points of loadout data, similar to percent compliance for production testing.

PWL is also calculated using upper and lower specification limits, either the same limits as generated from production testing or a separate limit named in the source QCP. This will largely depend on how the materials are stockpiled and handled.

## LOADOUT DATA REPORTING/DEPARTMENT SPREADSHEET

The CAPP loadout program does not intend for producers to have to calculate their PWL. INDOT has developed a spreadsheet that serves to calculate the PWL for all CAPP Certified 8, 11 and QA equivalent products using loadout test data added by the producer. The INDOT Loadout PWL



spreadsheet is located on an internet based file share system. The best way to access this is by contacting your district geologist or the INDOT Statewide Geologist from Materials and Tests.

When using the INDOT spreadsheet, first general information about the product needs to be entered such as source, aggregate name/size, shipment start date, target sampling frequency, critical sieve, target mean, upper and lower control limits, decant spec limit and sampling method. Figure 10.15 shows the inputs section of the department spreadsheet.

	<b>Indiana Aggregates, Anywhere, IN</b>
Source Name and Location	
Source Number	2999
Aggregate Name/Size	INDOT 8AP
Shipment Beginning Date	5/5/2023
Shipment End Date	6/30/2023
Shipment Tons	162,000
Number of Samples	22
Target Sampling Frequency (Tons)	8,000
Average Sampling Frequency (Calculated)	7,364
Critical Sieve	1/2"
Target Mean	52.0
Upper Control Limit UCL	62.0
Lower Control Limit LCL	42.0
Decant Upper Specification Limit USL	2.5
Source Software Name (if any)	Stonemont
Source Calculated Critical Sieve Mean $\bar{x}$	51.9
Source Calculated Standard Deviation	5.82
Source Calculated PWL	92.0
Producer Technician Name 1	
Producer Technician Name 2	
Producer Technician Name 3	
Producer Technician Name 4	
Producer Technician Name 5	
Sampling Method (ITM207 Mini or Strikeoff)	Strikeoff
Comments	Information Only
Count Samples Out of Spec on Other Sieves	3

**Figure 10.15: Product Information Section of Department Spreadsheet**

Once the sheet is set up with the appropriate information, data logging and analysis can begin. As each loadout test is completed, the following information is entered into the sheet for each sample and test: Sample date, percent passing value on the critical sieve, decant percent loss, whether or not all of the non-critical sieves complied with specifications and any comments or sample notes. At a minimum, this section of the sheet should be updated weekly. Figure 10.16 shows the test data section of the department spreadsheet.

Sample Date	Critical Sieve	T-11 Decant	All Other Sieves in Specification? (Yes/No)	Comments	INDOT Witness Name (Blank if none)
5/25/2023	45.1	0.6	Yes		Dusty Roads
5/26/2023	49.1	0.5	Yes		
5/27/2023	56.4	0.7	No		
5/28/2023	56.7	0.6	No		
5/29/2023	51.9	0.5	No		
5/30/2023	59.9	0.7	Yes		
5/31/2023	49.3	0.8	Yes		
6/1/2023	46.8	0.6	Yes		Sandy Rivers
6/2/2023	51.5	0.4	Yes		
6/3/2023	47.4	0.6	Yes		
6/4/2023	55.4	0.7	Yes		
6/5/2023	63.9	0.7	Yes		
6/6/2023	57.2	0.6	Yes		
6/7/2023	56.0	0.9	Yes		
6/8/2023	49.1	0.6	Yes		
6/9/2023	53.6	0.4	Yes		Janet Jackson
6/10/2023	47.9	1.1	Yes		
6/11/2023	54.6	0.3	Yes		
6/12/2023	44.5	0.5	Yes		
6/13/2023	54.2	0.6	Yes		
6/14/2023	52.6	1.0	Yes		
6/15/2023	64.4	0.8	Yes		
6/16/2023	46.3	0.7	Yes		
6/17/2023	49.8	0.6	Yes		
6/18/2023	55.1	0.9	Yes		
6/19/2023	46.5	0.6	Yes		Peyton Manning
6/20/2023	38.7	0.4	Yes		
6/21/2023	50.9	1.1	Yes		
6/22/2023	56.2	0.3	Yes		
6/23/2023	45.1	0.5	Yes		
6/24/2023	49.1	0.6	Yes		
6/25/2023	53.6	0.4	Yes		
6/26/2023	47.9	1.1	Yes		
6/27/2023	54.6	0.3	Yes		
6/28/2023	44.5	0.5	Yes		

**Figure 10.16: Sample/Test Data Entry Section of Department Spreadsheet**

As test data is added and accumulated in the spreadsheet, statistical data analysis is automatically updated for each test. The sheet will calculate PWL (and other intermediate data needed to calculate PWL) for all points until more than 30 are entered. From that point forward, the sheet will calculate PWL only on the most recent 30 data points entered. Figure 10.17 shows the statistical analysis section of the department spreadsheet. The dark blue-highlighted cell is the PWL of the dataset that needs to be continuously monitored and kept above 90 percent. The light blue-highlighted cells are the dataset statistics. The white cells are intermediate data needed to calculate PWL.

Critical Sieve	
PWL of Last 30 Tests	92.00
QL PWL	95.00
QL	1.65
QL LCL	42.00
Qu PWL	97.00
Qu	1.81
Qu UCL	62.00
Count	30.00
Calculated Average	51.55
Calculated Standard Deviation	5.78

**Figure 10.17: Statistical Analysis Section of Department Spreadsheet**

# GLOSSARY

## CHAPTER 11



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# GLOSSARY

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**AASHTO** - American Association of State Highway and Transportation Officials.

**Abrasion Resistance** - The resistance of coarse aggregates to fracturing under impact and breaking down into smaller pieces from abrasive action. Measured in accordance with AASHTO T96, ASTM C535, and ASTM C131, commonly referred to as LA Abrasion.

**Absorption** - The increase in the mass of aggregate due to water in the pores of the material, but not including water adhering to the outside surface of the particles, expressed as a percentage of the dry mass. Measured in accordance with AASHTO T85 or T84.

**Addendum Summary Sheet (Addenda)** – As part of the QCP, the Addendum is a list of yearly changes (adding or removing information) to the QCP identified by page or section and date. The list may be brief. The purpose is to be able to easily track changes that will be made or have been made in the document.

**Adherent Fines** - Fine particles smaller than the No. 200 (75 µm) sieve created from handling, or silt or clay that adheres to the coarse aggregate particles.

**Aggregate** – (in context) Granular materials that have been mined for use as a building material in the construction industry. Aggregates can come from natural and artificial origins, such as stone, sand and gravel, and slag.

**Aggregate Base** - A layer of aggregate placed on a subgrade or subbase to support a surface course. Typically, dense graded compacted aggregate material No. 53 is utilized. Class A, B, C, or D material required.

**Air-Cooled Blast Furnace Slag (ACBF)** - Material is a byproduct of iron manufacturing resulting from solidification of molten blast-furnace slag cooling slowly under atmospheric conditions.

**Annex (QCP Annex)** – Annex forms are filled out by an aggregate Source or a Department representative to add, modify, or delete Department approved aggregate products, to identify or change the Target Mean and Control Limits of a certified product, or to update CAPP Technician employment status. Annex forms are submitted to the Department for review and signature prior to approval and current forms may be included yearly with the QCP.

**Apparent Specific Gravity** - The ratio of the weight in air of a unit volume of the impermeable portion of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at a stated temperature. Measured in accordance with AASHTO T85 or T84.

**Artificial Aggregates** - Aggregates that are manufactured or co-products of an industrial process. Blast furnace slag, steel slag and wet bottom boiler slag are examples of co-product artificial aggregates.

**ASTM** - American Society for Testing and Materials

**B Borrow** - Material used for special filling such as for displaced peat deposits, bridge approach embankments, and fillings over structures. B borrow is required to be acceptable quality, free from large or frozen lumps, wood, or other extraneous matter. Materials used for B Borrow are suitable sands, gravel, crushed stone, ACBF, GBF, or other approved materials. Defined in Standard Specifications Section 904.06.

**Bedrock** - The solid rock that underlies gravel, soil, or other superficial material. In Indiana, the bedrock is mainly sedimentary rocks from the Pennsylvanian through Ordovician Periods.

**Bulk Specific Gravity** - The ratio of the weight in air of a unit volume of aggregate (including the permeable and impermeable voids in the particles but not including the voids between particles) at a stated temperature to the weight of an equal volume of gas-free distilled water at a stated temperature. Measured in accordance with AASHTO T85 or T84.

**Bulk Specific Gravity (SSD)** - The ratio of the mass in air of a unit volume of aggregate, including the mass of water within the voids filled to the extent achieved by submerging in water for approximately 15 hours (but not including the voids between particles) at a stated temperature to the weight in air of an equal volume of gas-free distilled water at a stated temperature. Measured in accordance with AASHTO T85 or T84.

**Category** - Source classification used to determine the production quality sampling frequency defined in ITM 203.

**Certified Material** - An aggregate product produced in accordance with the Certified Aggregate Producer Program (CAPP) for Department use.

**Certified Aggregate Producer (CAP)** - A Plant/Redistribution Terminal that meets the requirements of ITM 211 and Standard Specifications Section 917, continues to be under the same ownership, and is approved by the Department.

**Certified Aggregate Technician** – Any person that has successfully completed the Certified Aggregate Technician Training Program and has been certified by the Department.

**Chert** – An aggregate of varied color, composed of microcrystalline quartz meaning individual crystals cannot be seen without magnification. Chert is hard, dense, and brittle with a greasy texture. It will scratch glass because it is harder than glass but will not bubble when subject to a dilute Hydrochloric Acid solution because the composition is mainly quartz (SiO<sub>2</sub>). Chert is likely to have concave surfaces with sharp outer edges when freshly broken known as conchoidal fracture.

**Class A** - Quality rating assigned to aggregates which meet requirements for all Department uses except for specified slab on grade concrete applications. Defined in Standard Specifications Section 904.03.

**Class AP** - Quality rating assigned to coarse aggregates permitted for use in all Department concrete pavement and specified slab-on-grade concrete uses in accordance with ITM 210. Rating is attained by casting material into concrete beams and subjecting to freeze and thaw cycling procedures in accordance with ITM 210. Defined in Standard Specifications Section 904.03.

**Class AS** – Quality rating assigned to coarse aggregates permitted for use in SMA mixtures. Defined in Standard Specifications Section 904.03.

**Classes B, C, D, E, and F** - Quality ratings assigned to aggregates with restricted uses. Defined in Standard Specifications Section 904.03.

**Class G** - Quality rating assigned to materials which do not meet requirements for any Department use.

**Clay Lumps** – Materials that are easily crumbled or mashed with the fingers after soaking in water as determined by AASHTO T 112.

**Coarse Aggregate** - Aggregate that has a minimum of 20 percent retained on the No. 4 (4.75 mm) sieve.

**Coatings** – A layer of substance covering a part or all the surface of an aggregate particle. The coating may be of natural origin, such as mineral deposits formed in sand and gravel by ground water, or may be artificial such as dust formed by crushing and handling. (see Adherent Fines)

**Compacted Aggregate** – Aggregate that is spread and compacted, with optimum moisture, to Department specifications meeting either No. 2, 5, 8, 53, or 73 gradations as outlined in Standard Specifications Section 300.

**Composite Stockpiling** – Stockpiling of natural fine aggregate from multiple sources into one stockpile.

**Core Drilling Log** - A written field description of a rock core sample for the purpose of securing geological information and/or testing the quality properties of the rock as well as describing the location and the operations used to acquire the core.

**Core Sample** - A cylindrical rock sample obtained with a bit affixed to a barrel with drill rods that are advanced by a rotary drilling machine. Core sampling is preferred over face sampling.

**Crushed Stone** – In Indiana, usually gray to white rock composed of limestone. In general, crushed stone can be composed of any solid bedrock formation. In the blasting and crushing process at an aggregate source, the stone can be processed to any size from massive, to coarse to very-fine grained.

**Crushed Particles** – Physical testing on aggregate materials determining amount of non-round pieces in accordance with ASTM D 5821. Crushed particle requirements are used for gravel coarse aggregates in HMA, compacted aggregates, and asphalt seal coats.

**Decant (Decantation)** - A test performed in accordance with AASHTO T 11 utilizing water to determine the percent of material that is passing the No. 200 sieve. The decant test is conducted on both fine and coarse aggregate and is usually performed in conjunction with the sieve analysis test.

**Degradation** – As related to stockpile maintenance, the decomposition of a product as a result of handling, allowing equipment to drive on an aggregate stockpile, and/or weathering/exposure. Degradation may result in excessive fines or modified gradations from the initial processing resulting in failing loadout test results.

**Deleterious** - Undesirable aggregate material. Deleterious is an 'umbrella' term which applies to at least 5 categories identified in the Standard Specification Section 904.03. Each category has a separate maximum limit of which an aggregate product must meet in order to maintain Department approval.

**Density** - The weight per unit volume of a substance.

**Department** - Indiana Department of Transportation, INDOT

**D-List** – The Certified Aggregate Producers Product List, also referred to as the D-List identifies all products that each CAPP aggregate Source is approved to make and provide as a certified aggregate at their location.

**Dolomite** – A sedimentary carbonate rock containing at least 10.3% elemental magnesium when tested in accordance with ITM 205. Dolomite ( $\text{CaMg}(\text{CO}_3)_2$ ) may also be referred to as dolostone.

**DTE** – District Testing Engineer

**ESAL** – Equivalent single-axle loads (ITM 214, ITM 221, and Polish Resistant Aggregates)

**Face Sample** – A rock sample obtained from collecting loose rock off of the highwall at a stone quarry. Core sampling is preferred over face sampling.

**Fine Aggregate** - Aggregate that is 100 percent passing the 3/8 in. (9.5 mm) sieve and a minimum of 80 percent passing the No. 4 (4.75 mm) sieve.

**Fineness Modulus** - A factor commonly associated with aggregates used for portland cement concrete that is used to determine the relative coarseness or fineness of the aggregate grading.

**Flat and Elongated Particles** – Physical testing on aggregate materials determining amount of flat and elongated pieces in accordance with ASTM D 4791. The specifications define a flat and elongated particle as “one having a length to thickness ratio greater than five” and are often referred to as “carrot shaped”.

**Friable Particles** - Materials that are easily crumbled or mashed with the fingers after soaking in water as determined by AASHTO T 112.

**Geologist** - A geologist is an extreme "rock hound" who got a special hammer and a degree to make their hobby socially acceptable. They get paid to wander around outside and look at rocks and are professional "down-to-earth" individuals who study the Earth's faults, not their own.

**Gradation** - The range and relative distribution of particle sizes in the aggregate material. The Department requires gradations to be evaluated by percent passing a specific sieve size and ranges for all products are indicated in the Standard Specifications Section 904.



**Granulated Blast Furnace Slag (GBF)** - Glassy, granular material formed when molten blast-furnace slag from an iron blast furnace is rapidly chilled, as by immersion in water

**Gravel** - Unconsolidated deposits of all rock types. Typically rounded fragments because it was transported and deposited by modern or ancient glaciers, rivers, streams or beach deposits. Gravel deposits in Indiana are typically from ancient glaciers or river and stream deposits.

**HMA** – Hot Mix Asphalt

**Hardness** – As related to a physical property of minerals, Mohs Hardness Scale is frequently used for determination of mineral hardness. Mohs Scale identifies minerals and common items that can be used to scratch each other and assist in identifying a mineral. The properties can also be useful in rock identification.

**Hardness** – As related to Abrasion Resistance, hardness is a measure of the cementing and interlocking quality of an aggregate that controls the resistance of the aggregate to abrasion and degradation.

**Igneous Rock** - Rocks formed from hot volcanic magma-molten mineral material. Intrusive igneous rocks cooled slowly within the interior of the Earth's crust resulting in large crystals forming rocks like granite. Extrusive igneous rocks cooled quickly at a volcanic eruption on the Earth's surface resulting in very small crystals forming rocks like basalt.

**Independent Assurance (IA)** – Independent Assurance testing, often referred to as 'proficiencies', is conducted by District Testing personnel to verify the reliability of the results obtained in acceptance sampling and testing. Certified Aggregate Technicians are checked biannually by Independent Assurance Technicians through sampling and testing materials verifying proper procedures are being followed at the aggregate source.

**Lightweight Aggregate** – A material created as a by product of the manufacturing process of construction brick. The aggregate may range in dry loose weight from 6 to 70 pounds per cubic foot and is used in making lightweight concrete.

**Lightweight Chert** - Weathered (Lightweight) chert appears chalky and dull and will float in a heavy liquid of specific gravity 2.45 if tested in accordance with AASHTO T113.

**Ledge** - Any stratigraphic unit which may be separated from adjacent units by lithologic differences as defined by the Department.

**Ledge Sample** - Core or face sample taken to represent a ledge.

**Limestone** – A sedimentary rock primarily consisting of calcium carbonate. Limestone will not scratch glass because it is softer than glass but will bubble when subject to a dilute Hydrochloric Acid solution because the composition is mainly calcium carbonate (CaCO<sub>3</sub>).

**Loadout Sample** – A sample of aggregate acquired from a stockpile where material is actively being loaded into haul trucks, trains, etc. and sold. If material is not actively being sold, a loadout sample would be acquired from the stockpile where it would be anticipated to be sold from. In situations of limited available materials, aggregate may be sold out of a production stockpile (cone), and therefore a production and loadout sample may come from the same place.

**Loadout Stockpile** – Any stockpile where the material was moved from a production stockpile

(cone) to a different location. A loadout stockpile may be the same as a production stockpile if aggregate being sold is the material coming directly out of a production plant.

**Maximum Particle Size (Top Size)** - The sieve on which 100 percent of the material will pass.

**Metamorphic Rock** - Rocks that were originally igneous or sedimentary rocks, but were changed by pressure and/or heat. Limestone subject to heat and pressure will metamorphose into marble, while sandstone will change to quartzite and granite will change to gneiss.

**Mineral Filler** - Dust produced by crushing stone, portland cement, or other inert mineral matter having similar characteristics. Mineral filler for SMA shall be in accordance with the gradation requirements for size No.16.

**Moisture Content** – As related to an aggregate sample, the percent of water within/ on the surface of an aggregate sample. Moisture content is calculated by weighing and drying a sample to constant mass in accordance with AASHTO T255.

**Natural Aggregates** - Rock fragments which are used in their natural state such as crushed stone, sand, and gravel.

**Natural Sand** - Natural sand is a mineral particle or rock fragment that is larger than coarse silt but smaller than gravel. Size of sand ranges from 0.625mm (.002 in) to less than 2mm (.08 in). The rounded appearance of the aggregate particle shape is common in the natural sand. A widespread but highly variable resource in Indiana that was formed mostly by glacial actions of large ice sheets and then sorted by running water. Sand can be used as fill, or more often the coarser parts find use as components of concrete or asphalt pavement. Sand, a finer granular material, also is important in concrete and in making mortar and in snow and ice control.

**Nominal Maximum Particle Size** - The smallest sieve opening through which the entire amount of the aggregate is permitted to pass.

**Non-durable particles** - Soft particles as determined by ITM 206 and other particles which are structurally weak, such as soft sandstone, shale, limonite concretions, coal, weathered schist, cemented gravel, ocher, shells, wood, or other objectionable material

**Point-Of-Use Sample** - Production quality sample obtained at the last opportunity prior to incorporation into the end use. (ITM 203)

**Polish Resistant Aggregates** - Dolomite containing less than 10.3% elemental magnesium, crushed limestone, or gravel meeting the requirements of ITM 214. ITM 214 approves coarse aggregates for ESAL's equal to or greater than 3,000,000 and less than 10,000,000. Polish Resistant Aggregates that pass ITM 214 can be tested in accordance with ITM 221. ITM 221 tests the Polish Resistant Aggregate and can be used for HMA Surface ESAL equal to or less than 10,000,000 for INDOT highways. Aggregates meeting these requirements are maintained on the INDOT Approved List of Polish Resistant Aggregates.

**Producer** – An aggregate producer extracts and processes materials like sand, gravel, crushed limestone, and slag. Producer and Source are often used interchangeably as an equivalent term.

**Production Quality Sample** - An aggregate sample representing finished materials obtained at the aggregate source or the point-of-use. (ITM 203)

**Production Sample (Normal Production)** – An aggregate sample collected from a point where the currently produced material resides. In situations of limited available materials, aggregate may be sold out of a production stockpile (cone), and therefore a production and loadout sample may come from the same place.

**Production Stockpile (Cone)** – Aggregate coming directly out of a plant and forming a pile. If the plant utilizes a fixed conveyor, the pile will form a cone shape. If the plant utilizes a radial stacker, the pile may be slightly curved and elongated. If the plant utilizes elevated bins, production material may load directly into trucks and be transported directly to loadout stockpiles. Each plant may have multiple types of production stockpiles, and there may be additional types not listed here.

**Quality Assurance Materials** - Certified Materials controlled by aggregate gradations by the Producer.

**Quality Control Plan (QCP)** - The QCP encompasses the total process from preliminary material quality approval through the point where the aggregate leaves the Producers' control. The document is an agreement between a Producer and the Department about the processes being utilized at an aggregate source. It is site- specific and includes the production, policies, and procedures used by the Producer.

**Qualified Technician** - An individual who has successfully completed the written and proficiency testing requirements of the Department Qualified Laboratory and Technician Program run by an Independent Assurance Technician.

**Recycled Foundry Sand** - A mixture of residual materials used for the production of ferrous or non-ferrous metal castings and natural sands. Recycled foundry sand is required to comply with the Indiana Department of Environmental Management (IDEM) Class III or Class IV residual sands classification.

**Riprap** - Typically large aggregate materials used as a protective coating. Riprap may consist of steel furnace slag for dumped riprap only, sound stone, stone masonry, or other approved materials, free from structural defects and of approved quality. Standard Specification Section 904.04.

**Sandstone** - Sedimentary rock composed of siliceous sand grains containing quartz, chert, and quartzose rock fragments in a carbonate matrix or cemented with silica, calcite, or dolomite. Coarse sandstone is required to meet Class B quality requirements and can only be used in HMA or SMA surface mixtures. Southern Illinois sandstone is the only sandstone permitted to be used in asphalt for INDOT contracts.

**Sedimentary Rock** - Rocks formed from the disintegration of other rocks and organic materials. Limestone, dolomite, sandstone, shale, and siltstone are examples of sedimentary rock types. Crushed stone produced within Indiana originates from sedimentary bedrock deposits. Lightweight aggregates primary constituent is shale as byproduct from brick construction.

**Segregation** - The non-uniform distribution of coarse and fine aggregate components within a mixture of aggregates. Product conformity and uniformity may be predicted if all of the inputs into the plant are measured, evaluated, and controlled. Whenever one rock is placed up another rock, segregation may reduce the uniformity that the Producer so carefully has built into the product.

**Sieving** - A test procedure (AASHTO T 27) that is used to determine the gradation of a material. A sample of the aggregate material being tested is weighed and then passed through a series of sieves to determine the gradation.

**Slag** - Slag is a man-made or synthetic aggregate recycled from the steel-making process. It is a dark-colored and usually rough-textured aggregate.

**SMA** – Stone Matrix Asphalt (AS-904.03(a)) an asphalt mix comprised of a significant percentage of durable, coarse ( $\geq$  No. 8) aggregate resulting in stone-to-stone contact.

**Soundness** - The durability of fine and coarse aggregate and their resistance to the forces of weathering, in particular to alternate freezing and thawing conditions. The Department may measure soundness in up to 3 different procedures identified in the Standard Specifications Section 904.03 including AASHTO T103, Sodium Sulfate, or Brine Freeze and Thaw Soundness.

**Source** - Facility that processes or handles aggregates. A Redistribution Terminal is classified as a source. Producer and Source are often used interchangeably as an equivalent term.

**Source Map** - A map of the quarry showing critical features and operating areas required to be provided for crushed stone quarries as a part of their Source Report requirements outlined in ITM 203.

**Source Report**- Source Reports are required for crushed stone quarries and the necessary contents are outlined in ITM 203. The Report is a brief overview of the location, geology, ledge and mining information of the quarry to ensure a consistent product is being provided to the Department. The Source Report requires a stratigraphic column, map, and legend be included with the text and the document is updated yearly and submitted for Department review.

**Subbase** - A layer of aggregate placed on a subgrade to support an aggregate base.

**Subgrade** - The layer below the subbase that may be comprised of various aggregate types.

**Specific Gravity** - The ratio of the mass of a unit volume of a material to the mass of the same volume of gas-free distilled water at a stated temperature.

**Standard Specification Materials** - Certified Materials controlled by aggregate gradations as defined in the Department Standard Specifications Section 904 and the construction contract documents.

**Steel Furnace Slag (SF)** - A material derived from the further refinement of iron to steel.

**Subcategory** - Source classification based on results of tests conducted on source samples and used to determine the production quality sampling frequency defined in ITM 203.

**Structure Backfill** - Suitable sand, gravel, crushed stone, air-cooled blast furnace slag, or granulated blast furnace slag used to fill designated areas excavated for structures that are not occupied by permanent work.

**Wet Bottom Boiler Slag** - A material which is a by-product from coal combustion at electrical generating plants.

## **HOW TO FIND THE INDOT WEBSITE**

- > [www.in.gov/indot/](http://www.in.gov/indot/)
- > Doing Business with INDOT
- > Contractors/Construction
- > Division of Materials & Tests

<https://www.in.gov/indot/doing-business-with-indot/contractorsconstruction/division-of-materials-and-tests/>

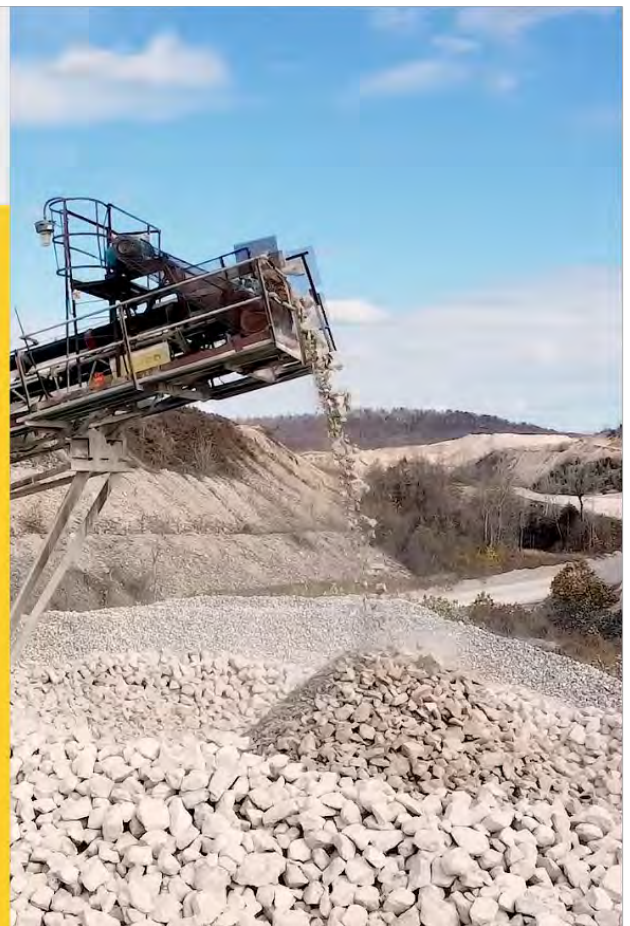
## **HOW TO FIND THE STANDARD SPECIFICATIONS ONLINE**

- > [www.in.gov/indot/](http://www.in.gov/indot/)
- > Doing Business with INDOT
- > Standards & Specifications
- > Standard Specifications

<https://www.in.gov/dot/div/contracts/standards/book/index.html>

# AUDIT CHECKLIST & QCP

## CHAPTER 12



**CERTIFIED AGGREGATE  
PRODUCER PROGRAM  
AUDIT CHECKLIST**

**Date** \_\_\_\_\_

**Source No.** \_\_\_\_\_

**\*Q No.** \_\_\_\_\_

**Plant/Redistribution Terminal Name** \_\_\_\_\_

**Plant/Redistribution Terminal Location** \_\_\_\_\_

**District Testing Engineer or** \_\_\_\_\_

**INDOT Audit Team Members**

	<u>Name</u>	<u>Position</u>
1.	_____	_____
2.	_____	_____
3.	_____	_____
4.	_____	_____
5.	_____	_____
6.	_____	_____
7.	_____	_____

**Plant/Redistribution Terminal Members**

	<u>Name</u>	<u>Position</u>
1.	_____	Certified Aggregate Technician
2.	_____	_____
3.	_____	_____
4.	_____	_____
5.	_____	_____

\* If applicable



Source # \_\_\_\_\_

## 1. GENERAL INSTRUCTIONS

*Certified Aggregate Producer Program (CAPP)  
Quality Control Plan (QCP)  
Indiana Test Method (ITM)  
Certified Aggregate Technician (CAT)  
Quality Assurance (QA)  
Percent Within Limits (PWL)*

*Any square bracket marked by an X on the Audit Checklist requires a Corrective Action Sheet to be prepared. The Corrective Action Sheet will be prepared when a deficiency is found, and a copy provided to the Producer at the end of the audit. All other square brackets will have a check, if the item is satisfactory, or NA if not applicable.*

*Begin the audit by comparing the producer's copy of the QCP with the copy stored on the Aggregate Section shared drive.*

*A listing of applicable INDOT documents and Indiana Test Methods are maintained in the CAPP Document List. The current revision date for each publication is provided in the list.*

*The Addenda Summary Sheet and the QCP Annex are required to be maintained in the QCP Appendix. Items on these two sheets should be reviewed and discussed during the audit. The Producer should be instructed that any necessary addenda for these items must be submitted at the close-out meeting.*

1.1 [ ] Addenda Summary Sheet and QCP Annex reviewed (if applicable)

Source # \_\_\_\_\_

## 2. PRODUCTION FLOW DIAGRAM

ITM 211 Reference

14.2.10

INDOT Auditor \_\_\_\_\_

- 2.1 [ ] The Annual Aggregate Source Report for Stone Producers represents conditions found at the source and as required by ITM 203

*Review the diary and note locations where material has been extracted.*

- 2.2 [ ] Locations noted in the diary match areas that have been mined as shown on the grid in the Annual Aggregate Source Report

*Begin at the origin of the material, which is the quarry or the pit. Inspect the site and review the process. Ask the producer about any changes to the production process since the last audit, and whether the QCP flow diagram is up-to-date.*

*Ask the producer about any changes in the ledge processing or changes in the plant, including crushers, washers, bins, belt routes, screen combinations, delivery and off-loading processes, etc. Specific details, such as manufacturers names, screen sizes, dimensions, etc., are not required on the flow diagram.*

- 2.3 [ ] Producer indicated that the plant is in accordance with QCP

- 2.4 [ ] All plant and process changes noted in diary

*Identify all material stockpiles and bins within the Producer's yard.*

- 2.5 [ ] All stockpiles and bins (if applicable) have signs in accordance with ITM 211 14.2.16 or the QCP

- 2.6 [ ] Stockpile map (if applicable) is current and located as indicated in the QCP

- 2.7 [ ] All material stockpiles and bins are listed as materials or otherwise accounted for in the QCP

- 2.8 [ ] (if applicable) Air-cooled blast furnace slag stockpiles designated for leachate testing are approximately 2000 tons

- 2.9 [ ] (if applicable) Steel furnace slag stockpiles designated for deleterious testing are approximately 2000 tons

*The QCP covers any other process control techniques used beyond the minimums established by INDOT specifications and policies.*

- 2.10 [ ]\* Other process control techniques, if any, are as defined in QCP

Source # \_\_\_\_\_

### 3. QUALITY CLASSIFICATIONS

#### ITM 211 References

14.2.3

14.2.4

14.2.8

INDOT Auditor \_\_\_\_\_

*A list and description of all portions of the mineral deposits indicating the different quality classes as described in ITM 203, ITM 205, and ITM 210 will be provided in the QCP. The manner in which each quality class is processed, handled, and stockpiled will be covered.*

- 3.1 [ ] Each quality class is processed, handled, and stockpiled in accordance with the QCP

*An explanation for each product having marginal quality characteristics and the plans or controls to be used for such products shall be provided in the QCP.*

- 3.2 [ ]\* Each marginal quality class material is processed, handled, and stockpiled in accordance with QCP  
\* Only applicable if Producer has materials with marginal quality characteristics

### 4. MATERIALS

#### ITM 211 References

3.10

5.2

INDOT Auditor \_\_\_\_\_

*If the Producer is a Redistribution Terminal, prior source documentation of a material obtained from another aggregate source shall be provided by the Producer.*

- 4.1 [ ] Quality is satisfactory, as verified by being from a Certified Producer and a Certified Material, or traced to original INDOT approved source

*The list of Certified Materials for the Producer as reported in the Certified Aggregate Producers Product List, CAPP D-List agrees with the list of materials indicated in the QCP.*

- 4.2 [ ] The list of Certified Materials is in accordance with the QCP.

*If the source has yet to be CAPP approved, a list of products, ledges, if applicable, and source code numbers will be tabulated and included with the Audit Checklist*

Source # \_\_\_\_\_

**5. PRODUCER GENERAL INFORMATION**

ITM 211 References

5.1  
5.2  
14.2.1

INDOT Auditor \_\_\_\_\_

- 5.1 [ ] Plant location and address in the QCP is correct
- 5.2 [ ] Plant telephone numbers in the QCP are correct
- 5.3 [ ] Producer's name and address in the QCP are correct and ownership has not changed
- 5.4 [ ] Producers telephone numbers in the QCP are correct
- 5.5 [ ] Key personnel contact information in the QCP is correct. (Management Rep and CAT mobile numbers and email addresses.)
- 5.6 [ ] Type of Producer (plant, redistribution terminal, or plant and redistribution terminal) identified in the QCP is correct

**6. PRODUCER PERSONNEL**

ITM 211 References

6.1  
6.2  
14.2.2

*The Producer employees occupy the following positions.*

- 6.1 [ ] Management Representative listed in the QCP
- 6.2 [ ] CAT(s) listed in the QCP
- 6.3 [ ] Appointed CAT(s) Certification has not expired

CAT Name	On Certification Technician List

Source # \_\_\_\_\_

**Personnel (continued)**

6.4 [ ] All personnel conducting sampling and testing for the CAPP are Qualified Technicians

Qualified Tech Name	District Qualified/Expiration Date

Source # \_\_\_\_\_

## 7. DOCUMENTS

ITM 211 References

2.5, 17.3

INDOT Auditor \_\_\_\_\_

*Determine whether the following documents are current and on file at the Producer's site or location indicated in the QCP. (The documents may be maintained either in electronic or hard copy format.) Check the CAPP Document List for the most current dates of these items.*

- 7.1 [ ] INDOT Certified Aggregate Producer Program (ITM 211)
- 7.2 [ ] INDOT Standard Specifications sections 211, 301, 302, 303, 404, 904 and 917
- 7.3 [ ] CAPP Training Manual for Producer Technicians
- 7.4 [ ] Summary of Production Quality Test Results Letter, Summary of Ledge Quality Letter, and the AP Aggregate Approval Letter for all applicable materials produced at the Plant
- 7.5 [ ] All applicable INDOT, AASHTO, and ASTM Test Methods **referenced in the QCP**. The documents are in accordance with the CAPP Document List.

ITM 206 _____	AASHTO R 76 _____
ITM 207 _____	AASHTO R 90 _____
ITM 212 _____	AASHTO T 11 _____
ITM 219 _____	AASHTO T 27 _____
ITM 902 _____	AASHTO T 84 _____
ITM 906 _____	AASHTO T 85 _____
ITM 910 _____	AASHTO T 112 _____
_____	ASTM D 4791 _____
_____	ASTM D 5821 _____
_____	_____

*Obtain weigh tickets for each certified product for material shipped for Department use. For multiple ledges and composite stockpiling from two or more ledge combinations, one ticket from each ledge combination or composite stockpiling will suffice. Check for accuracy and minimum requirements as follows:*

- 7.6 [ ] Q number listed (if applicable)
- 7.7 [ ] For Redistribution Terminals, originating source name, and source number listed
- 7.8 [ ] Source name and number listed and is correct
- 7.9 [ ] Aggregate size and class listed as per ITM 211 Section 17.3
- 7.10 [ ] Ledge combination of actual sold material is listed for stone products. In the case of approved composite stockpiling from two or more ledge combinations the multiples may be listed.

Source # \_\_\_\_\_

## 8. CONTROL CHARTS

ITM 211 Reference  
13.0

INDOT Auditor \_\_\_\_\_

### ***ALL CONTROL CHARTS***

- 8.1 [ ] All control chart construction and symbols are in accordance with ITM 211 Section 13.4 or as allowed by the software used by the producer.
- 8.2 [ ] All materials identified as products in the QCP have a control chart which is available to view
- 8.3 [ ] All materials identified as products in the QCP have a control chart which have upper and lower control limits as indicated in the QCP or in accordance with INDOT Specifications 904.03(e)
- 8.4 [ ] Aggregate sizes are clearly shown on the control charts
- 8.5 [ ] Control charts are maintained as indicated in the QCP
- 8.6 [ ] Control charts are generated electronically
- 8.7 [ ] Control charts are hand plotted

*Check the **critical sieve** material control charts for compliance with the QCP and ITM 211. Production and load-out charts (if load-out tests are plotted on a separate chart) are required to be checked.*

#### Target Mean

- 8.8 [ ] Values are the same as indicated in QCP
- 8.9 [ ] Numerically identified in the left margin of charts or in accordance with the QCP and indicated to the tenths decimal place (0.0)

#### Control Limits

- 8.10 [ ] Upper and lower control limits are the same as indicated in the QCP
- 8.11 [ ] Numerically identified in the left margin of charts or in accordance with the QCP and indicated to the tenths decimal place (0.0) or whole number (0)

***Critical sieve** materials that have not achieved a minimum of 10 normal production tests are required to have the specification limits plotted for all sieves.*

***Non-critical sieve** material control charts are required to be checked for compliance with the QCP and ITM 211. Production and load-out charts (if load-out tests are plotted on a separate chart) are required to be checked.*

*For non-critical sieve materials, check the following:*

#### Specification Limits

- 8.12 [ ] Upper and lower limits indicated on all sieves
- 8.13 [ ] Values are the same as Section 904 for Standard Specification materials or as indicated in the QCP for QA materials and multiple gradation products
- 8.14 [ ] Numerically identified in left margin of charts or in accordance with QCP and indicated to the tenths decimal place (0.0) or whole number (0)

Source # \_\_\_\_\_

## CONTROL CHARTS (continued)

*Select one control chart for a material from each of the following categories:*

Material selected for production control charts with critical sieves: \_\_\_\_\_

Material selected for load-out control charts with critical sieves: \_\_\_\_\_

Material selected for production control charts with no critical sieves: \_\_\_\_\_

Material selected for load-out charts with no critical sieves: \_\_\_\_\_

*For each of the categories listed above, check for conformance with the criteria below. Mark the square bracket with a Q for any deviation from the CAPP that is in accordance with the QCP.*

- 8.15 [ ] Charting displays 30 production or 30 load-out points if plotted electronically on separate charts, or up to 30 hand plotted production and load-out points per chart as they occur. Less than 30 points may be applicable if the product is new and 30 points do not exist or a significant change in process has occurred and justifies a new start to a chart.
- 8.16 [ ] If in the Trial Phase, charts are maintained since entering into the Trial Phase
- 8.17 [ ] All charts have been retained at least 3 years for Certified Producers that have been in CAPP more than 3 Years
- 8.18 [ ] Points plotted left to right in chronological order
- 8.19 [ ] Test dates shown along the horizontal axis

*Obtain production test reports and load-out test reports (if plotted on same chart) to check for accuracy in reporting and plotting. For hand-plotted charts, check all tests during an active period of one week. For computer generated charts, check two randomly selected tests.*

- 8.20 [ ] All production test dates for points plotted on charts are the same as dates reported on test reports and in the daily diary
- 8.21 [ ] All load-out test are all reported and plotted in accordance with ITM 211 Section 11.3.3 Table Note 4.
- 8.22 [ ] All points are plotted correctly
- 8.23 [ ] Five point moving average calculated and plotted correctly for two randomly selected points
- 8.24 [ ] Nonconforming trend in 5-point moving average of control chart (7 or more points in a row are above or below target mean, or 7 or more points in a row are increasing or decreasing) noted in the daily diary.



Source # \_\_\_\_\_

## CONTROL CHARTS (continued)

*Any nonconforming (failing decant, gradation, deleterious) normal production or load-out test shall be followed immediately by the appropriate action taken. Search control charts for nonconforming tests. If nonconforming tests are found, review the diary on the date of each test for notations regarding action taken.*

- 8.25 [ ] Nonconforming tests are noted in the diary
- 8.26 [ ] Corrective action was taken
- 8.27 [ ] (If applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated
- 8.28 [ ] (If applicable) After the second consecutive nonconforming load-out test, notations indicate that shipping from the stockpile was stopped

## COMPLIANCE RATE AND LOAD-OUT PWL RATE

### Compliance Rate

*Review the 30 most recent normal production tests in the current and previous year that are charted for each Standard Specification or QA product controlled by a critical sieve. If 30 tests are not available, the number of tests taken shall be used with at least 10 tests required. For hand-plotted charts, calculate the test compliance rate using the Compliance Rate Worksheet for all materials. For computer generated charts, check the compliance rate for all materials and calculate the compliance rate for one material using the Compliance Rate Worksheet.*

- 8.29 [ ] Compliance rate  $\geq 95\%$  and  $\sigma \leq 5.0$  for each material
  - \*Compliance rate is  $< 95\%$  and  $\sigma \leq 5.0$  for a material (The target mean,  $\sigma$ , is required to be adjusted by a QCP Annex)
  - \*Compliance is  $< 95\%$  and  $\sigma > 5.0$  for a material. (The stockpile is required to be designated as a non-Certified material)

*\*\*If the number of tests is less than 30, additional testing is required before the target mean is adjusted or the material is designated as a non-Certified material. An additional compliance rate check on the material is required after five additional tests have been taken. To close out the audit in a timely manner, additional testing and compliance rate check can happen after the audit close-out, once all the data has been collected.*

### PWL Load-out Rate

*Review the 30 most recent load-out production tests in the current PWL Load-out Worksheet that the Department provided for each Standard Specification or QA product controlled by a critical sieve for the accuracy of the worksheet calculated PWL Rate. If applicable, repeat this step for the Producer Yard.*

- 8.30 [ ] Are the PWL worksheets updated? If not, the producer has 10 business days from the date of this audit to update the PWL charts.
- 8.31 [ ] Are the control limits accurate in the PWL worksheet.
- 8.32 [ ] PWL rate  $\geq 90\%$  for each material with 5 or more data points
  - \*PWL rate is  $< 90\%$ , the producer shall develop a plan to bring PWL of the product above 90%

Source # \_\_\_\_\_

## 9. DIARY

ITM 211 References  
10.0, 12.5, 12.7

INDOT Auditor \_\_\_\_\_

*Review at least 1 month of active diary entries for the following:*

- 9.1 [ ] Diary is in electronic and/or hard copy format
- 9.2 [ ] One page for each day that there is a material-related operation
- 9.3 [ ] General weather conditions
- 9.4 [ ] Areas of mining operation – ledges or pit area
- 9.5 [ ] Materials produced and estimated quantities
- 9.6 [ ] Materials sampled and tested
- 9.7 [ ] Time samples were obtained and tests completed (may state that all samples obtained were tested the same day)
- 9.8 [ ] Compliance rate for all critical-sieve products and PWL rates for 8's and 11's are noted in the diary on a weekly basis
- 9.9 [ ] If compliance rate, standard deviation, or PWL is reported below the minimum limit on any diary page reviewed, was the District Geologist notified?
- 9.10 [ ] Any changes in key personnel have been noted in the diary.
- 9.11 [ ] Significant changes in equipment, plant, screens, etc. have been noted in the diary.
- 9.12 [ ] Significant events or problems have been noted in the diary.
- 9.13 [ ] Signature by CAT or another person's signature has been counter-signed
- 9.14 [ ] All diaries have been retained at least 3 years for Certified Producers that have been in CAPP more than 3 years.

## 10. SAMPLING AND TESTING

### ITM 211 References

11.0, 14.2.6, 14.2.7, 14.2.8

*The method of recording the quantities of materials **produced** and **shipped** at the Plant per day or time period will be identified in the QCP. Select an active one-month period at random from this record. Obtain all production and load-out test reports for materials produced and shipped during the one-month period. Perform calculations as needed and determine that the proper testing frequency has been demonstrated. The previous or subsequent monthly record may need to be obtained to verify the frequency of tests.*

- 10.1 [ ] Start of production frequency is in accordance with the QCP, but is not less than once every 1000 t for the first 5000 t (except not required to exceed 2 per day)  
\*if applicable, go to the month that start of production frequency occurred to review.
- 10.2 [ ] Normal frequency is in accordance with QCP, but is not less than once every 2000 t (except not required to exceed 2 per day)
- 10.3 [ ] Load-out frequency is in accordance with the QCP and ITM 211 Section 11.3.3 and the additional notes in the table.
- 10.4 [ ] Load-out samples obtained within the first 1000 t shipped in the calendar year in accordance with ITM 211 Section 11.3.3 Table Note 1.
- 10.5 [ ] Load-out samples obtained based on tonnage without daily, weekly, or monthly limits in accordance with ITM 211 Section 11.3.3 Table Note 2.
- 10.6 [ ] Load-out samples obtained within  $\pm 1$  business day or  $\pm 1000$  t shipped at the completion of each full interval in accordance with ITM 211 Section 11.3.3 Table Note 3.
- 10.7 [ ] All load-out samples for Standard Specifications and QA aggregates were decanted and tests are within specification requirements

*If material is obtained from another Certified Producer and is a Certified Material, then load-out tests are required. If the material is obtained from a non-Certified Producer or is not a Certified Material, then the start of production, normal production, and load-out tests are required. Search the records for these materials, if applicable, and verify that the required tests have been conducted.*

- 10.8 [ ] Load-out test conducted for Certified Material from another Producer per ITM 211 Section 11.3.3. If any redistribution product has shipped more than 1000 t to date in current year, then review at least 1 load-out test performed in current year.
- 10.9 [ ] Start of production, normal production, and load-out tests conducted for material that is not Certified and is received from another Producer

*The Producer shall check coarse aggregates for deleterious materials. Select an active week randomly from the record for quantities of materials made and note all coarse aggregates produced. Find production test reports for that week and search for deleterious test results.*

- 10.10 [ ] Start of production and normal production frequency is in accordance with the QCP but is not less than once per week for each size of Certified Material. (no test is required if the week's production is less than 100 t)
- 10.11 [ ] Tests are within specification requirements

Source # \_\_\_\_\_

## **SAMPLING AND TESTING (continued)**

*The Producer shall check seal coat aggregates for ITM 224 Flakiness testing. Select an active week randomly from the record for quantities of materials made and note all coarse aggregates produced. Find production test reports for that week and search for flakiness test results.*

- 10.12 [ ] Normal production frequency is in accordance with the QCP but is not less than once per week for each size of Certified Material. (no test is required if the week's production is less than 100 t)
- 10.13 [ ] Tests are within specification requirements
- 10.14 [ ] Nonconforming tests are noted in the diary
- 10.15 [ ] Corrective action was taken
- 10.16 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

*Randomly select three production test reports and two load-out test reports for any one product and check all calculations performed on the sheets. If test reports are electronic, check calculations on one production test report and one load-out test report.*

Indicate type of Report:    Electronic Reports: \_\_\_\_\_ Hand Calculated Reports: \_\_\_\_\_

- 10.17 [ ] Calculations on all sheets are correct and rounded to the nearest tenths decimal place (0.0) (crushed particle content values shall be rounded to the nearest whole number (0))
- 10.18 [ ] For production and load-out test reports reviewed, verify the original dry weight test mass is in accordance with INDOT Specifications 904.07(c)
- 10.19 [ ] All tests have been retained at least 3 years for Certified Producers that have been in CAPP more than 3 years

*Gravel for seal coat and other gravel products required to meet criteria for crushed testing per ASTM D5821 as outlined in INDOT Specifications 404.04 and 904.03(a) unless the QCP states otherwise. Select a random week from the record for quantities of products made and note all coarse aggregates produced. Find the production test reports for that week and search for crushed particle test results.*

- 10.20 [ ] Start of production and normal production frequency is in accordance with the QCP, but is not less than once per week for each size of Certified Material. (no test is required if the week's production is less than 100 t)
- 10.21 [ ] Tests are within specification requirements for one- and two-face fractured particles (or total fractured particles, as applicable)
- 10.22 [ ] Nonconforming tests are noted in the diary
- 10.23 [ ] Corrective action was taken
- 10.24 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

Source # \_\_\_\_\_

## **SAMPLING AND TESTING (continued)**

*Air-Cooled Blast Furnace Slag, except for use in HMA or PCC, shall be sampled and tested for leachate in accordance with ITM 212. Randomly select an active month from the record for quantities made and verify the frequency of testing.*

- 10.25 [ ] The frequency of testing is in accordance with QCP, but is not less than once for each stockpile of approximately 2000 t
- 10.26 [ ] Tests are within specification requirements
- 10.27 [ ] Nonconforming tests are noted in the diary
- 10.28 [ ] Corrective action was taken
- 10.29 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

*Steel Furnace Slag shall be sampled and tested for determination of bulk specific gravity when this material is used in SMA mixtures. Randomly select an active month of production of the steel slag and verify the frequency of testing and compliance with the specification requirements.*

- 10.30 [ ] The frequency of testing is in accordance with QCP, but is not less than once every 2000 t.
- 10.31 [ ] Individual test results are within 0.050 of the target bulk specific gravity
- 10.32 [ ] The moving average of four consecutive test results is within 0.040 of the target bulk specific gravity
- 10.33 [ ] Nonconforming tests are noted in the diary
- 10.34 [ ] Corrective action was taken
- 10.35 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

*Steel Furnace Slag shall be sampled and tested for determination of deleterious when this material is used in HMA Base and Intermediate mixtures. Randomly select an active month of production of the steel slag and verify the frequency of testing and compliance with the specification requirements.*

- 10.36 [ ] The frequency of testing is in accordance with QCP, but is not less than once every 2000 t.
- 10.37 [ ] Individual test results are less than 4.0 % (Stockpiles not meeting this acceptance criteria may be tested again after 30 days from the test date)
- 10.38 [ ] Nonconforming tests are noted in the diary
- 10.39 [ ] Corrective action was taken
- 10.40 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

Source # \_\_\_\_\_

## **SAMPLING AND TESTING (continued)**

*Composite stockpiling of natural sand fine aggregate from multiple sources into one stockpile may be done provided the fine aggregate is within a range of 0.030 for the bulk specific gravity (dry) and 0.5 % for the absorption for all of the contributing sources. Randomly select an active month of composite stockpiling from the monthly summary reports and verify the test results are within the bulk specific gravity (dry) and absorption specification requirements.*

- 10.41 [ ] Bulk specific gravity (dry) test results of all contributing sources are within a range of 0.030.
- 10.42 [ ] Absorption test results of all contributing sources are within a range of 0.5%
- 10.43 [ ] Nonconforming tests are noted in the diary
- 10.44 [ ] Corrective action was taken
- 10.45 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

*Additional required testing as specified in source's QCP. Randomly select an active month of production and verify the frequency is in accordance with the QCP. Type of test*

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- 10.46 [ ] Testing frequency meets the requirements of the QCP
- 10.47 [ ] Test results are within specification requirements
- 10.48 [ ] Test results outside the specification requirements are handled in accordance with the QCP.
- 10.49 [ ] Nonconforming tests are noted in the diary
- 10.50 [ ] Corrective action was taken
- 10.51 [ ] (if applicable) After the second consecutive nonconforming normal production test, notations indicate that the material was isolated

## **11. PRODUCER YARDS**

ITM 211 Reference  
5.1

Area Supervisor or \_\_\_\_\_

*If a source has Producer Yards, separate load-out charts are required to be maintained for the materials at these locations. Obtain the load-out charts and check the following:*

- 11.1 [ ] All certified materials have a load-out chart
- 11.2 [ ] Aggregate sizes are clearly shown on the charts
- 11.3 [ ] Target means, control limits, and specification limits for all charts are in accordance with the QCP

*Obtain load-out test reports for one material during an active period of one week. Find the corresponding control chart and check the following:*

- 11.4 [ ] All test dates have points plotted
- 11.5 [ ] Points are surrounded by a small square or in accordance with the QCP and plotted to the tenths decimal place (0.0)
- 11.6 [ ] All points are plotted correctly
- 11.7 [ ] Consecutive points are connected by solid straight line

Source # \_\_\_\_\_

**PRODUCER YARDS (continued)**

*Obtain all load-out test reports for materials shipped from the Producer Yard during a one-month period. Perform calculations as needed and compare the quantities of materials shipped against the load-out test reports, thereby determining the demonstrated frequency of testing. The previous or subsequent monthly record may need to be obtained to verify the frequency of tests.*

- 11.8 [ ] Load-out frequency is in accordance with QCP and ITM 211 Section 11.3.3 and the additional notes in the table.
- 11.9 [ ] Load-out samples obtained within the first 1000 t shipped in the calendar year in accordance with ITM 211 Section 11.3.3 Table Note 1.
- 11.10 [ ] Load-out samples obtained based on tonnage without daily, weekly, or monthly limits in accordance with ITM 211 Section 11.3.3 Table Note 2.
- 11.11 [ ] Load-out samples obtained within  $\pm 1$  business day at the completion of each full interval in accordance with ITM 211 Section 11.3.3 Table Note 3.
- 11.12 [ ] Load-out frequency is in accordance with QCP and ITM 211 Section 11.3.3 and the additional notes in the table.
- 11.13 [ ] All load-out samples for Standard Specification and QA aggregates were decanted and tests are within specification requirements

Source # \_\_\_\_\_

## 12. MATERIAL SAMPLES

### ITM 211 References

INDOT Auditor \_\_\_\_\_

11.0  
14.2.10  
14.2.11  
15.7

*Standard Specification and QA materials under production at the site on the day of the audit will be reviewed by the audit team. At least one production sample of Standard Specification or QA material shall be obtained.*

*The audit team will review the shipment records of the Standard Specification and QA materials for the previous 6 months of production. A minimum of 3 load-out samples shall be obtained of the materials with the highest tonnages of shipment. (Some producers may have less than 3 load-out samples.) If limited products are available, a combination of 4 or fewer samples of either production or load-out samples may be requested.*

*The samples shall be obtained by the CAT and reduced to the proper testing size. The INDOT audit team members will take custody of the samples and deliver them to the appropriate state testing lab. Side-by-side style testing is not required but can be performed by the producer at their own discretion. The comparison spec (ITM 211 Section 15.7) will not be used to compare the side-by-side test nor the 10% within wet weights of the samples, if the producer decides to do the side-by-side testing. For the purpose of the audit, all passing and failing results will be determined by the samples processed in the state testing laboratories.*

*Sampling shall be in accordance with the QCP, and the following requirements shall be verified.*

- 12.1 [ ] Sample locations are as described or as shown in the QCP
- 12.2 [ ] Devices are as described in the QCP
- 12.3 [ ] Techniques are as described in the QCP including strike-off procedures as described in ITM 207.
- 12.4 [ ] CAT obtained the sample and will reduce it to testing size, in accordance with CAPP

*The following production and load-out test results will be determined:*

- 12.5 [ ] Gradation is within control limits for critical sieve materials and within specification requirements for all other sieves
- 12.6 [ ] Gradation is within specification requirements or QCP identified limits on all sieves for materials without a critical sieve
- 12.7 [ ] Decant is within specification requirements
- 12.8 [ ] Deleterious content is within specification requirements
- 12.9 [ ] Crushed particles for gravel producers and redistribution terminals are within specification requirements
- 12.10 [ ] Flakiness testing for Seal Coat products are within specification requirements per ITM 224.



Source # \_\_\_\_\_

### 13. LABORATORY

#### ITM 211 References

8.0

9.0

INDOT Auditor \_\_\_\_\_

*The laboratory will be inspected for compliance with the QCP.*

- 13.1 [ ] Location as described and/or shown in the QCP
- 13.2 [ ] Facility is acceptable for testing of materials
- 13.3 [ ] All equipment listed in the QCP is at the laboratory
- 13.4 [ ] All equipment appears to be in good working order

*Check the testing equipment verification records to verify that the documentation includes the following:*

1. Description of equipment including Model or Serial Number, if applicable.
2. Name of the person performing verification
3. Identification of verification equipment, if applicable
4. Date of verification and next due date
5. Reference of procedure used
6. Verification results

#### **DATE VERIFIED**

- 13.5 [ ] Balance(s) -- 12 mo. \_\_\_\_\_
- 13.6 [ ] Weights used (Class 1, 2, or 3) -- 12 mo. \_\_\_\_\_
- 13.7 [ ] Mechanical Shaker(s) -- 12 mo. \_\_\_\_\_
- 13.8 [ ] Sieves -- 12 mo. \_\_\_\_\_
- 13.9 [ ] Go No-Go Gauges procedures in accordance with the QCP and ITM 902 6.2 (if applicable) \_\_\_\_\_

- 13.10 [ ] Additional information can be placed here for calipers, stop watches, etc. that the producer has done but is not required by ITM 211

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*Any other additional laboratories will be inspected for compliance with the QCP. If the laboratories are in another district, contact that District Geologist to see if everything is current. If the laboratory is used for another audit, mark everything current in the other audit packet.*

- 13.11 [ ] Is the lab current with paperwork? If so, list the District or Audit where the paperwork is provided from. \_\_\_\_\_

Source # \_\_\_\_\_

## 14. AUDIT CLOSE-OUT

### INDOT District Personnel

*The Audit Close-Out meeting with the Producer will typically be held within 10 business days from the date of the audit, however it may be longer if directed by INDOT district personnel. At the close-out meeting, the results of the audit will be discussed and all outstanding matters will be completely resolved, or solutions with deadlines will be established. Any addenda required by items listed on the Addenda Summary Sheet, QCP Annex, or Corrective Action Sheets shall be submitted at this time.*

*If the Audit is not completed within the original time frame provided by the District Geologist, then a reminder email will be sent to the Producer informing the Producer they are non-compliant. The producer can request a one-time extension of 14 business days.*

*Upon completion of the Audit Close-Out meeting, district personnel should review all audit documents to verify that they are prepared properly and are complete. All documents should be scanned and uploaded to the Aggregate Section folder on the shared drive and stored in the source's folder. A copy of the completed audit packet including any relevant documents, corrective actions, and resolutions shall be provided to the Producer.*

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INDOT District Geologist

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Date

# **COMPLIANCE RATE WORKSHEET** (Critical Sieve Only)

SC # \_\_\_\_\_

Product \_\_\_\_\_ Critical Sieve \_\_\_\_\_ QCP Target Mean \_\_\_\_\_

Record the most recent 30 normal production sample test results.

_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____
_____	_____	_____	_____	_____	_____

Calculate the following Statistics:

$$\bar{x} = \underline{\hspace{2cm}} \quad \sigma_{n-1} = \underline{\hspace{2cm}}$$

$$Z_{\max} = \frac{(\text{QCP Target Mean} + 10) - \bar{x}}{\sigma}$$

$$= \underline{\hspace{2cm}} = \underline{\hspace{2cm}}$$

$$Z_{\max} \text{ Area of Probability} = \underline{\hspace{2cm}} * x 100 = \underline{\hspace{2cm}}$$

$$Z_{\min} = \frac{\bar{x} - (\text{QCP Target Mean} - 10)}{\sigma}$$

$$= \underline{\hspace{2cm}} = \underline{\hspace{2cm}}$$

$$Z_{\min} \text{ Area of Probability} = \underline{\hspace{2cm}} * x 100 = \underline{\hspace{2cm}}$$

$$\% \text{ Compliance } \Sigma = \underline{\hspace{2cm}}$$

(Whole No.)

\* From Area of Probability Table

**EXAMPLE****COMPLIANCE RATE WORKSHEET  
(Critical Sieve Only)**SC # 2799Product #8 Stone Critical Sieve 12.5 mm QCP Target Mean 52.2

Record the most recent 30 normal production sample test results.

<u>55.5</u>	<u>49.4</u>	<u>50.3</u>	<u>56.1</u>	<u>53.6</u>	<u>54.6</u>
<u>51.2</u>	<u>46.0</u>	<u>49.5</u>	<u>59.1</u>	<u>52.6</u>	<u>58.1</u>
<u>53.2</u>	<u>42.4</u>	<u>50.8</u>	<u>55.6</u>	<u>52.1</u>	<u>56.4</u>
<u>56.4</u>	<u>53.1</u>	<u>50.5</u>	<u>53.8</u>	<u>61.3</u>	<u>50.9</u>
<u>54.2</u>	<u>65.7</u>	<u>55.2</u>	<u>52.8</u>	<u>49.7</u>	<u>48.1</u>

Calculate the following Statistics:

$$\bar{x} = \underline{53.3} \quad \sigma_{n-1} = \underline{4.53}$$

$$Z_{\max} = \frac{(\text{QCP Target Mean} + 10) - \bar{x}}{\sigma}$$

$$= \frac{(52.2 + 10) - 53.3}{4.53} = \underline{1.96}$$

$$Z_{\max} \text{ Area of Probability} = \underline{.4750} * \times 100 = \underline{47.50}$$

$$Z_{\min} = \frac{\bar{x} - (\text{QCP Target Mean} - 10)}{\sigma}$$

$$= \frac{53.3 - (52.2 - 10)}{4.53} = \underline{2.45}$$

$$Z_{\min} \text{ Area of Probability} = \underline{.4929} * \times 100 = \underline{49.29}$$

$$\% \text{ Compliance } \Sigma = \underline{97}$$

(Whole No.)

\* From Area of Probability Table

## CORRECTIVE ACTION SHEET

**SOURCE #** \_\_\_\_

**DATE** \_\_\_\_\_

**ITEM** \_\_\_\_\_

**Problem Explanation:** \_\_\_\_\_

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**Corrective Action to Be Taken Is:** \_\_\_\_\_

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**Deadline Date Is:** \_\_\_\_\_

**Follow-up Date** \_\_\_\_\_

**Finding:** \_\_\_\_\_

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**If NOT corrected, prepare another Corrective Action Sheet .**

[illegible]

# SAMPLE QUALITY CONTROL PLAN

FOR DEMONSTRATION

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1





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## **AUTHENTICATION**

## SECTION 1

### **PLANT LOCATION**

Limerock Quarries, Inc., Markle Plant (2799) is classified as a Plant for CAPP purposes. The location of the plant is indicated on the map in Appendix A. The address and other pertinent information is as follows:

Limerock Quarries Inc.  
(INDOT #2799)  
6593 County Rd.  
Bippus, IN 46713

Latitude - 40E 47' 58"  
Longitude - 85E 21' 26"  
County - Huntington  
Township - T 27 N  
Range - R 10 E  
Section - 11

Office Phone No. - 219-555-7214  
Fax - 219-555-6032  
Certified Technician Mobile - 219-555-8381  
Lab - 219-555-4621

The Limerock Quarry is approximately 2.3 miles south of the town of Markle and west of SR 3. To locate the plant follow SR 3 south from Markle to CR 200S. At this intersection turn west, cross over Rock Creek and the entrance to the Quarry is the first right.

The Limerock Quarry is owned by Stone City, Inc. which is located at the following address:

Stone City, Inc.  
5538 Subbase Ln.  
Mineral, IN 46220  
317-257-1996  
FAX: 317-257-1995

## SECTION 2

### ORGANIZATIONAL STRUCTURE

#### MANAGEMENT REPRESENTATIVES

The Quarry Superintendent for Limerock Quarries and Management Representative for this source is Clay Mudstone. He is responsible for all production for this site.

#### CERTIFIED TECHNICIANS

The CAPP Certified Aggregate Technician responsible for this location is Richard Quality. His duties include testing and reporting results from this site as well as assisting with the CAPP duties at several other sites within the Limerock organization. Mr. Quality communicates all CAPP concerns to Mr. Mudstone who then takes appropriate actions.

Other Quality Control Personnel include Crystal Stone and Chip Samples. Ms. Stone's duties include sampling and reporting test results for three sites within Limerock Quarries, Inc. She may also assist at this site as the need arises. Mr. Samples is a CAPP Certified Aggregate Technician who will substitute for Mr. Quality when necessary.

### SECTION 3

#### **MINERAL DEPOSITS**

All stone in the production areas are Silurian in age. The stone is derived from bedrock belonging to the Wabash and Pleasant Mills Formations and the Liston Creek, Mississinewa, Louisville and Limberlost Members.

Ledges 1 and 2 are mined as one bench for commercial purposes only. Ledge 3 is not useable for any type of aggregate use and is stripped and wasted.

Useable INDOT aggregates include only ledges 4 and 5, which are mined as one 73 foot thick bench. (A geologic cross-section of these ledges is provided in Appendix A). Test data and Class rating for these ledges appear in the Summary of Ledge Quality Tests letter dated March 31, 1993. This source is currently classified as Category 1A. Ledges 4 and 5 are allowed for use as AP aggregates and dolomite aggregates.

#### **SECTION 4**

##### **AP AGGREGATE**

Ledges 4 and 5 are approved for Class AP aggregate production as indicated on the approval letter dated November 4, 1994. The AP Production Control Plan is included in Appendix B.

## SECTION 5

### **MATERIAL CATEGORIES**

#### **STANDARD SPECIFICATION**

Aggregates produced from ledges 4 and 5 are categorized as Standard Specification materials and include the following INDOT sizes:

2, 5, 8, AP 8, 9, 11, 12, 53, 73

#### **ALTERNATE**

Aggregates produced from ledges 1 and 2 are categorized as Alternate materials and include the following INDOT sizes:

2, 53, 73, Rip Rap

#### **OTHER**

No. 23 natural sand is stockpiled at the on-site Tarcoat asphalt plant. This material is only used for the asphalt plant and is not sampled or tested by Limerock Quarries, Inc.

## **SECTION 6**

### **PRODUCTION FLOW DIAGRAM**

Aggregates produced at this source are produced from 1 production area. Only those aggregates produced from ledges 4 and 5 are used for INDOT purposes. Therefore, the process description will be limited to those matters associated with production from these ledges. The flow diagram for the entire plant and detailed flow diagrams for stations #2 and #3 are shown on the following pages.

Production of materials from ledges 4 and 5 is referred to as Class A production. This part of the plant is referred to as the A Circuit. Materials from the A Circuit that are in the muck pile are all removed with a loader and put into haul trucks. The trucks unload the aggregates into a Traylor 54" gyratory crusher where they are crushed and conveyed to the Primary Surge Pile.

#### **STATION #2**

Material from the Primary Surge Pile is fed into Station #2 where the first screening is done with a Drister 5' x 14' 3-deck vibrating screen. The following materials are processed:

1. Oversized materials are closed-circuited to a 4' shorthead Symons cone crusher where they are reduced and returned to Station #2.
2. Materials passing through the first deck and retained on the second deck are #2 size and may either be diverted back to the 4' Symons crusher or routed to a bin where they are loaded into trucks for stockpiling.
3. Materials passing through the second deck and retained on the third deck are always sent to Station #3.
4. Materials passing through all decks at this station are sent to the stacker and stockpiled as #53 or #73.

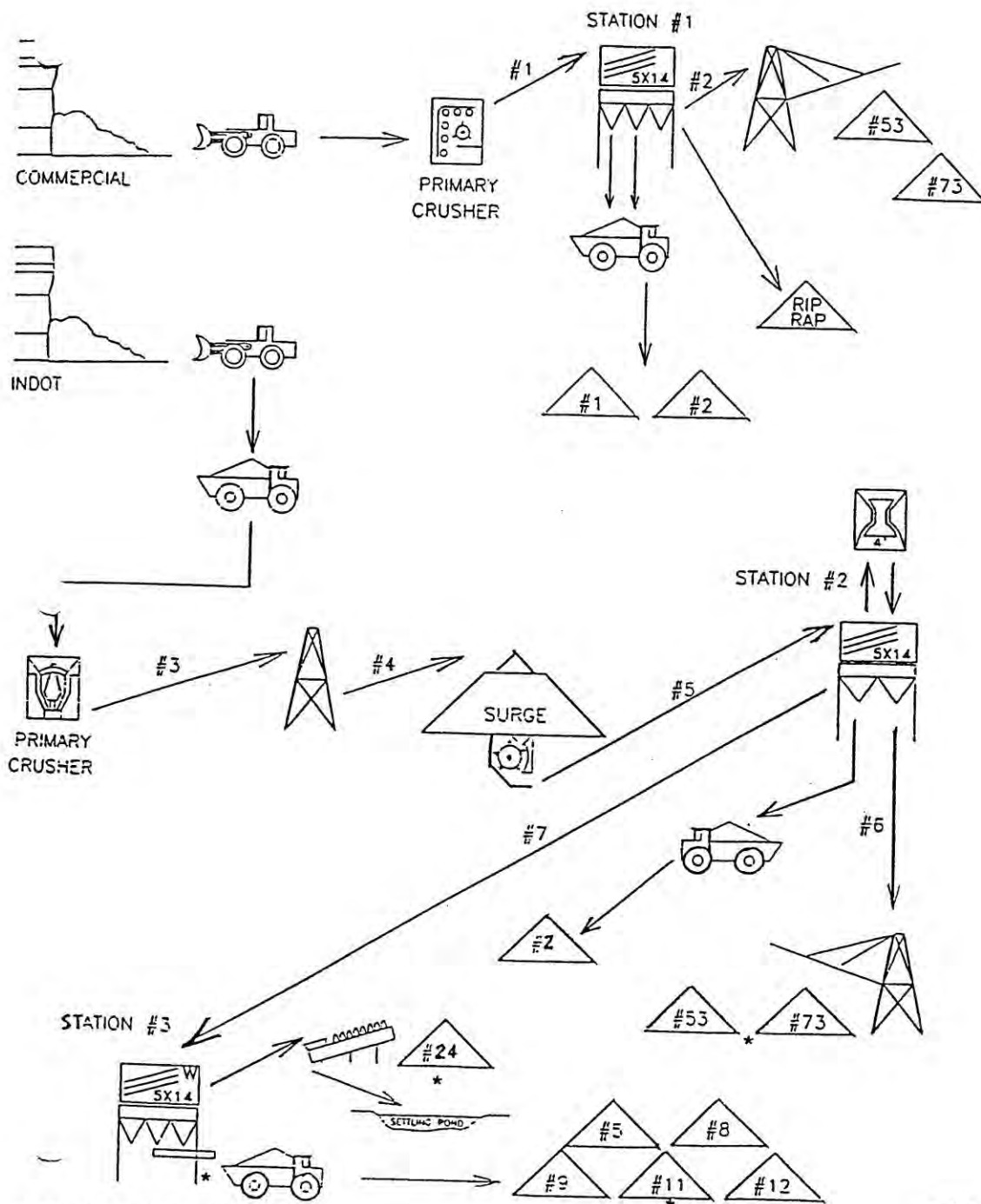
#### **STATION #3**

Material from Station #2 is fed into this station which is the wash station. The screening unit is a 3-deck 5' x 14' Deister vibrating screen with water. The top deck has a combination of 1/2" and 3/8" screen panels, the middle deck has a combination of 1/4" and 5/16" screen panels and the bottom deck has .080" x 5/16" slotted panels or 1/8" square panels. These combinations can be changed to control our products. All materials arriving at this Station are pre-screened. The following materials are washed and stored.



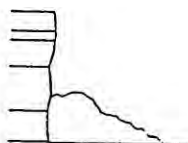
1. Oversized materials are either #5, #8 or #9 and go into the coarse bin to await stockpiling.
2. Materials retained on the second screen go into the #11 bin. The #11 aggregate may either be stockpiled or combined with the coarse bin materials or both.
3. Materials retained on the third screen go into the #12 bin. The #12 aggregate may either be stockpiled or combined with the #11 aggregate or both.
4. Materials passing through all decks at this station are sent to the stone sand screw to produce #24 manufactured fine aggregate. Fines from here go to the settling ponds.

# FLOW DIAGRAM FOR LIMEROCK QUARRIES, INC. - #2799



12/95

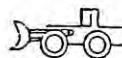
ALTERNATE TEMPLATE  
FOR LIMEROCK QUARRIES, INC. - #2799



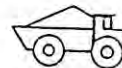
SHOT PILE



SETTLING POND



LOADER



HAUL TRUCK



IMPACT CRUSHER



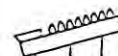
GYRATORY CRUSHER



SYMONS CRUSHER



SURGE FEEDER



SAND SCREW



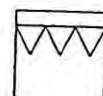
5x14  
3-DECK  
SCREEN



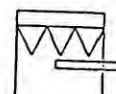
5x14  
3-DECK  
WASH  
SCREEN



2-BIN  
STATION



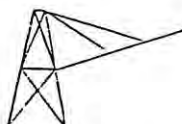
3-BIN  
STATION



3-BIN  
STATION  
WITH SIDE  
DISCHARGE



CONVEYOR  
TOWER



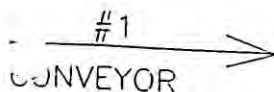
RADIAL  
STACKER



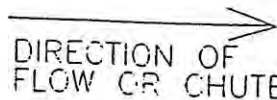
STOCKPILE



SURGE PILE



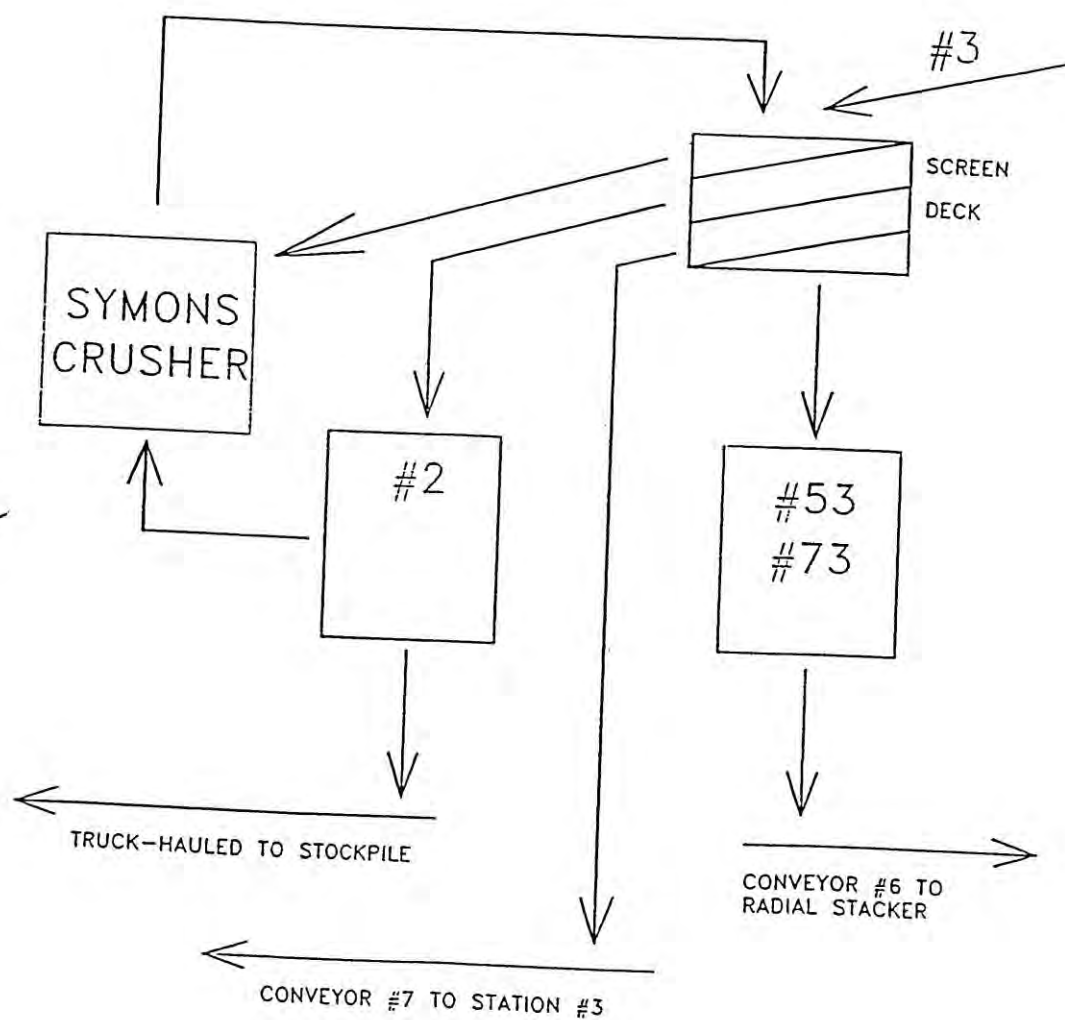
CONVEYOR



DIRECTION OF  
FLOW OR CHUTE

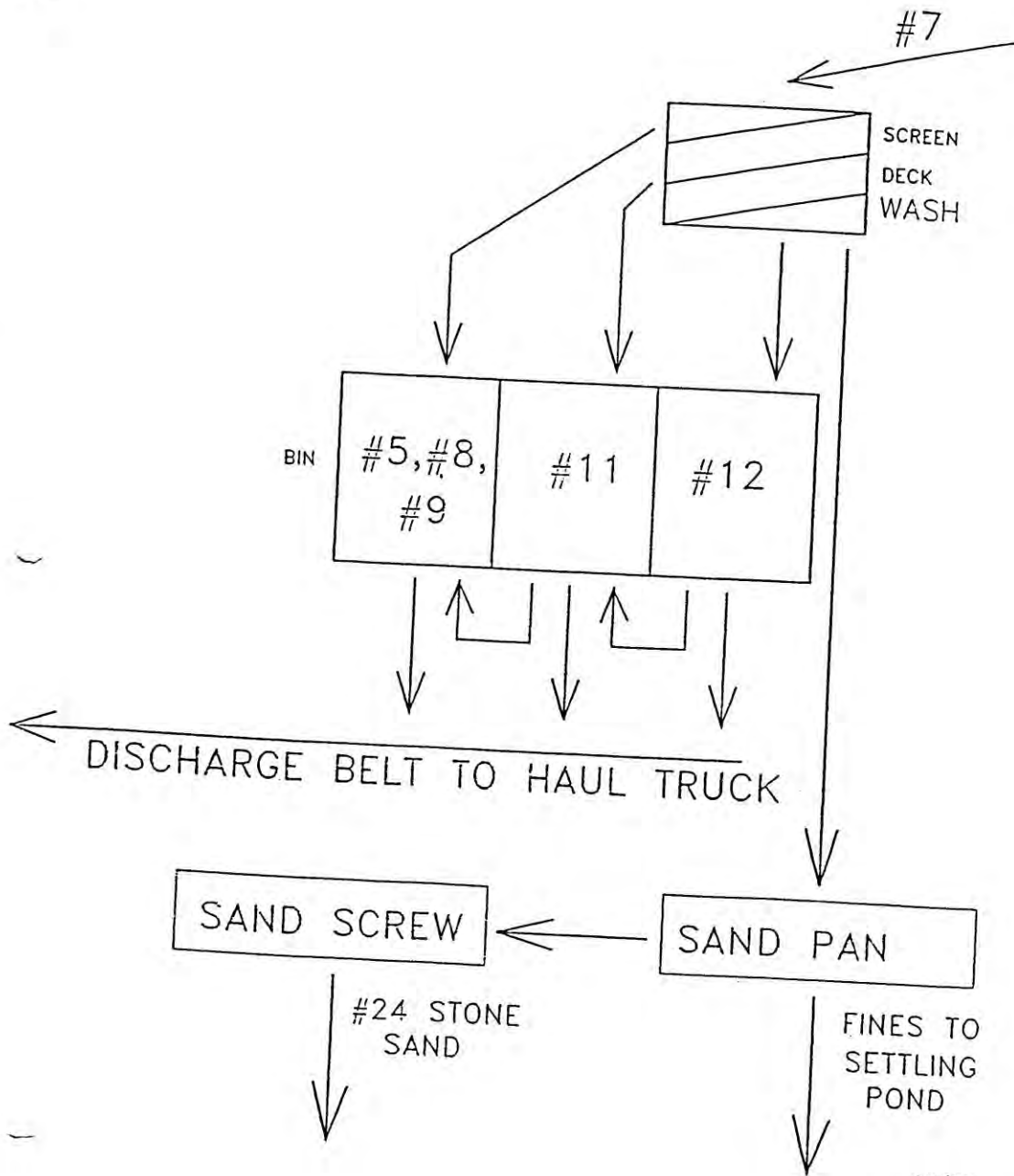
12/95

# FLOW DIAGRAM FOR #2 STATION



12/95

# FLOW DIAGRAM FOR #3 STATION



12/95

## **SECTION 7**

### **SAMPLING PLAN**

Limerock Quarries, Inc. has developed a coding scheme to distinguish the 5 different types of samples that are obtained. The type of samples, frequency of sampling, location of samples and sampling procedures of any Certified Materials are included as follows.

#### **TYPE OF SAMPLES AND FREQUENCY**

(S) Start of Production. After a seasonal shutdown or when producing a new material, start of production samples shall be obtained once every 1000 t for the first 5000 t, but shall not exceed 2 per calendar day.

(N) Normal Production. After the start of production samples have been completed for each material, normal production samples shall be obtained. The frequency of these samples shall be once every 2000 t, but shall not exceed 2 per calendar day.

(L) Load-Out. Load-out samples shall be taken from material that is shipped. The frequency of these samples shall be once every 8000 t; however, there shall be at least one sample taken each month for any Certified Material shipped that exceeds 1000 t.

(M) Miscellaneous. Miscellaneous samples are taken at our own discretion for information purposes outside the start of production or normal production samples.

(R) Resample. When there is a failing normal production or load-out test a resample shall be taken.

#### **MEANS OF TRACKING SAMPLES**

Start of production and normal production samples shall be taken on a random basis using the Random Number Form (Appendix C). The Quarry Superintendent shall be responsible for using this form and communicating with production staff as to when to obtain samples. If the random ton to be sampled is determined to be from the first or second load of the day then the third load shall be selected to sample.

Shipping tonnages shall be kept by the office bookkeepers to determine when the load-out samples are to be obtained. The bookkeepers shall inform the Quarry Superintendent of when a sample is required.

## **SAMPLE LOCATIONS**

All start of production and normal production samples, except for #24 stone sand, shall be taken from the bins before incorporation into the stockpiles. No. 24 stone sand shall be sampled from a small pile of material that is stockpiled as it comes off of the sand screw.

Load-out samples shall be taken from the Certified Material stockpiles.

The points of sampling for all samples are indicated on the flow diagram on page 6-3.

## **SAMPLING PROCEDURES**

Start of production and normal production samples, except for #24 stone sand, shall be dumped from a truck into a small stockpile. Sampling from these stockpiles shall be in accordance with ITM 207. No. 24 stone sand shall be sampled using the same procedure for the coarse aggregates after the small sample stockpile is rolled until it appears homogeneous.

Load-out samples shall be sampled in accordance with ITM 207.

## **SECTION 8**

### **TESTING PLAN**

#### **GRADATION**

Gradation analysis shall be performed in accordance with AASHTO T 27 on all start of production, normal production and load-out samples. A gradation test shall be performed on resample and miscellaneous samples when necessary.

#### **DECANT**

Decant tests shall be performed in accordance with AASHTO T 11 on all load-out samples.

#### **DELETERIOUS**

The percent of deleterious materials shall be determined in accordance with AASHTO T 112 at least once per week for each size of material for the start of production and normal production samples. No test shall be performed if the week's production is less than 100 t.

#### **NON-CONFORMING MATERIAL**

Any time there is a failing normal production or load-out test the Quarry Superintendent shall be notified immediately and a resample test taken. Typically, retests shall be accompanied by a visual check for any problems at the plant. All actions shall be documented in the Daily Diary.

In the event that a second consecutive normal production sample fails, the materials will be diverted until the problem is corrected. Failing INDOT sizes 2, 5, 8, 9, 11, 12 and 24 shall be taken to the scrap pile and wasted. Failing INDOT sizes 53 and 73 shall be incorporated into the commercial stockpiles.

In the event that a second consecutive load-out sample fails, shipping from that stockpile shall cease. The stockpile problem area shall be checked to determine if the stockpile can be remixed and restored within the quality control limits as verified by the resample tests. If the problem area cannot be remixed, the material shall be removed and taken to the scrap pile.



## SECTION 9

### GRADATION CONTROL

#### NO. 5 STONE

1/2 in. Critical Sieve

$$\bar{x} = 45.1\%$$

$$\sigma_{n-1} = 5.0$$

Upper Control Limit = 55.1%

Lower Control Limit = 35.1%

#### NO. 8 STONE

1.2 in. Critical Sieve

$$\bar{x} = 49.0\%$$

$$\sigma_{n-1} = 5.0$$

Upper Control Limit = 59.0%

Lower Control Limit = 39.0%

#### NO. 9 STONE

3/8 in. Critical Sieve

$$\bar{x} = 42.5\%$$

$$\sigma_{n-1} = 4.0$$

Upper Control Limit = 50.5%

Lower Control Limit = 34.5%

#### NO. 11 STONE

No. 4 Critical Sieve

$$\bar{x} = 22.2\%$$

$$\sigma_{n-1} = 4.7$$

Upper Control Limit = 31.6%

Lower Control Limit = 12.8%

#### NO. 12 STONE

No. 4 Critical Sieve

$$\bar{x} = 67.8\%$$

$$\sigma_{n-1} = 5.0$$

Upper Control Limit = 77.8%

Lower Control Limit = 57.8%

## SECTION 10

### **PROCESS CONTROL TECHNIQUES**

Los Angeles abrasion and absorption tests may be performed when deemed necessary and shall be posted on the Gradation Analysis Form (Appendix C). A visual check of all stockpiles is an ongoing daily procedure.

## **SECTION 11**

### **DOWNSTREAM CONTROL**

#### **IDENTIFICATION OF STOCKPILES**

All stockpiles shall be marked using signs in front of each stockpile that indicate the size of each material and the ledges the material is from. For Standard Specification stockpiles, the signs shall be blue with white lettering and for Alternate stockpiles, the signs shall be red with white lettering.

#### **STOCKPILE CONSTRUCTION**

Stockpiling of the aggregates is done by unloading truck loads side by side and then stacking the material only as high as the front-end loader can place the material.

#### **MATERIAL RETRIEVAL**

The entire front face of each stockpile shall be worked by a front-end loader from side to side when loading the truck. The sides of the face shall be occasionally mixed with the center to prevent segregation of the stockpile.

## SECTION 12

### **LABORATORY**

#### **LOCATION**

The laboratory is located near the west boundary of the property at the Bippus facility. The following verified equipment is maintained in the laboratory:

#### **EQUIPMENT**

##### Sieve Analysis

Gilson TS-1 shaker

15 in. x 23 in. screens (2 in. (50 mm), 1½ in. (37.5 mm), 1 in. (25 mm), ¾ in. (19.0 mm), ½ in. (12.5 mm), ⅜ in. (9.5 mm), No. 4 (4.75 mm) and pan)

Gilson Ro-Tap shaker

8 in. round sieves (¾ in. (9.5 mm), No. 4 (4.75 mm), No. 8 (2.36 mm), No. 16 (1.18 mm), No. 30 (600 µm), No. 50 (300 µm), No. 100 (150 µm), No. 200 (75 µm) and pan)

##### General

Humboldt oven

Ohaus IP12KS Digital Electronic Balance  
(12000 g capacity)

Mettler H10 Electronic Balance  
(160 g capacity)

#### **CALIBRATION**

The balances, mechanical shakers, oven, and sieves shall be verified in accordance with the following:

<u>Equipment</u>	<u>Minimum Frequency</u>	<u>Procedure</u>
Balances	12 mo.	ITM 910
Mechanical Shakers	12 mo.	ITM 906
Sieves	12 mo.	ITM 902

## **SECTION 13**

### **DOCUMENTATION PLAN**

Several forms have been developed for the CAP program and all information regarding the CAPP shall be entered on these forms. Examples of these forms may be found in Appendix C.

#### **REFERENCE DOCUMENTS**

The following documents are on file at the lab:

1. INDOT Certified Aggregate Producer Program (ITM 211)
2. INDOT Standard Specifications and Current Supplemental Specifications
3. INDOT Inspection and Sampling Procedures for Fine and Coarse Aggregate
4. Indiana Quality Assurance Certified Aggregate Technician Training Manual for Producer Technicians
5. Summary of Ledge Quality Results
6. Summary of Production Quality Results
7. AP Aggregate Letter
8. Quality Control Plan

#### **DIARY**

The diary is located in the Superintendent's office. One page is devoted to each day of the year that there is a material related operation and all the pages are maintained in a 3-ring binder.

#### **AGGREGATE INSPECTOR RECORD BOOK**

Each aggregate inspector working for this company is issued a number which is unique to that individual. Test data is recorded in the Aggregate Inspector Record Book and is traceable to any inspector through the identification number. This document is located in the laboratory.

#### **GRADATION ANALYSIS FORM**

This form is used for a quick visual comparison of up to 16 separate gradations of like materials. There is a different version of this form for each size of CAPP material including a generic version that may be used for any other material. This document is located in the laboratory.

## **CONTROL CHARTS**

Control charts for each size material are posted on the wall in the laboratory. We request that the following deviations be allowed to the control chart legend:

1. The target mean shall be placed at the appropriate location on the critical sieve portion of the chart rather than the middle of the chart. It shall be represented by a heavy long dash followed by a short dash and the value shall be shown in the left-hand column. This change allows us to use one form for all of our sources.
2. The specification limits shall be solid lines instead of short dashed lines. This makes for an overall more attractive control chart.
3. Because of the space limitations on an 11 in. x 17 in. control chart, and that we plot all sieves for all sizes of stone, our charts do not always have a minimum of 2 in. between control limits. The charts, however, are very legible.

## **RANDOM NUMBER FORM**

This form was developed to assist the Quarry Superintendent in determining which loads shall be sampled for production control.

## **INDOT MATERIAL SHIPPING RECORD FORM**

This form was developed to assist our bookkeepers in keeping track of how many tons of each material are shipped from the Certified Material stockpiles. This form is readily available to INDOT personnel at the office.

## **SHOT CHART**

Information concerning the exact mining point of the materials being produced may be found in the shot chart. All production shots at this source are numbered. The assigned number has 2 parts: the first digit(s) represent the sequential order in which that shot was taken during a particular year and the last 2 digits represent the year of that shot (Example: 05-95 would be the fifth shot of 1995). The shot chart is kept with the Daily Diary and the shot number for a given production day is indicated in the diary. The shot chart may be cross-referenced to the original Quarry Map to determine the exact location of the point of mining of that material.

## **SECTION 14**

### **ADDENDA**

Each page in the Quality Control Plan that is revised shall have the source number, date of revision, and a vertical line in the left margin indicating the paragraph that was revised.

Revisions to the QCP shall be maintained on an Addenda Summary Sheet or QCP Annex in the Appendix until such time that the revisions are incorporated into the QCP. Addenda shall be submitted at the close-out meeting for an annual audit. Any outstanding revisions will also be submitted in January of each year along with the Annual Aggregate Source Report.

## APPENDIX A

### ANNUAL SOURCE REPORT



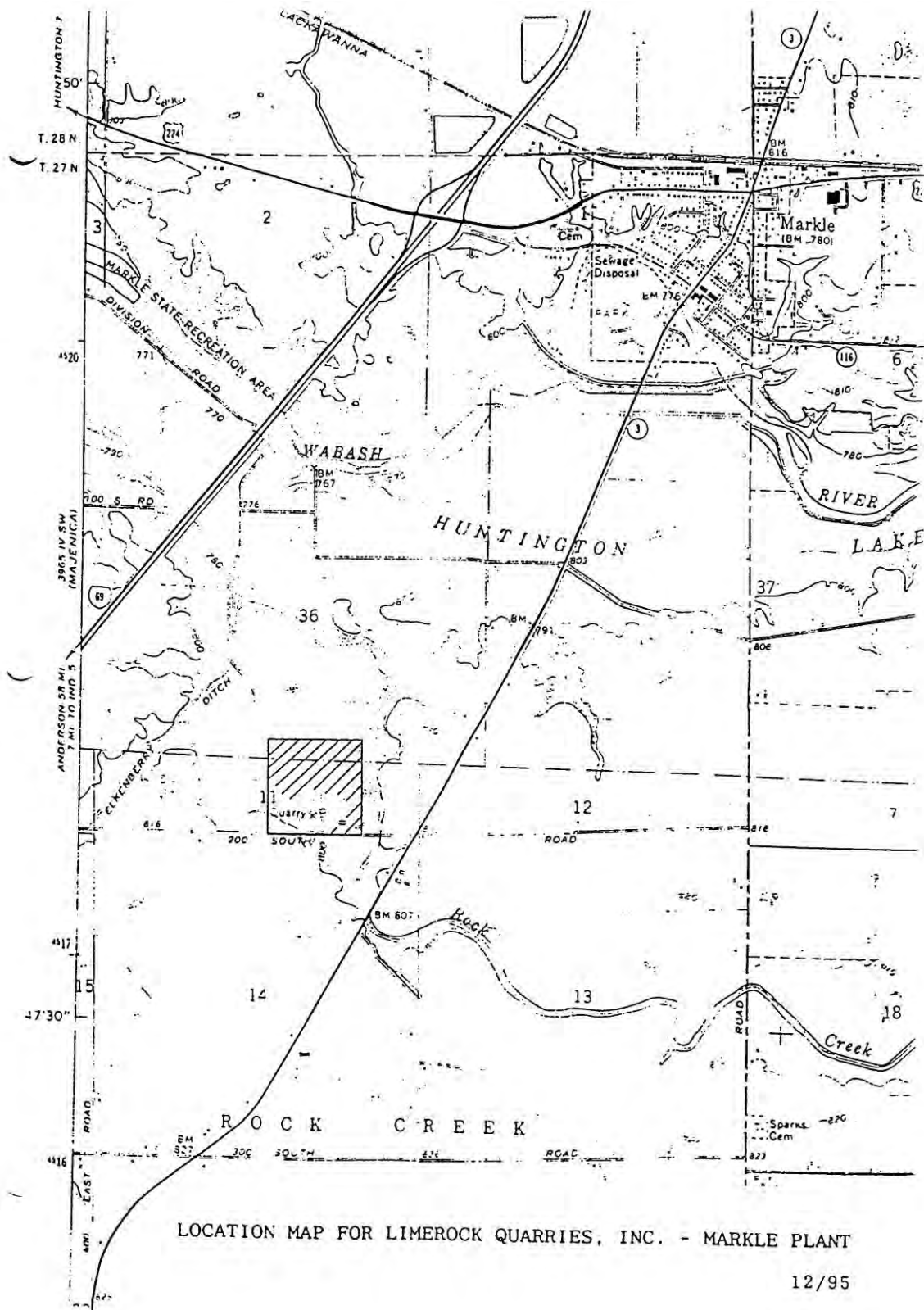
**2000 ANNUAL SOURCE REPORT  
LIMEROCK QUARRIES, INC.  
INDOT SOURCE #2799**

Limerock Quarries, Inc. is located in Huntington County, Indiana approximately 2.3 miles south of Markle, Indiana and one mile west of SR 3. The quarry property is within section 11, T 27 N and R 10 E.

The Regional Manager is Ferris Ore, the Quarry Superintendent is Clay Mudstone and quality control is conducted by Richard Quality and Crystal Stone. INDOT may contact any of these persons as the need arises.

This source currently operates two open-pit benches. The upper bench is mined for commercial purposes only and consists of ledges 1 and 2. Ledge 3 is not useable for any type of aggregate use and is stripped and wasted. The lower bench, containing ledges 4 and 5, is approximately 73' thick and is classified as Category IA, and Class AP materials.

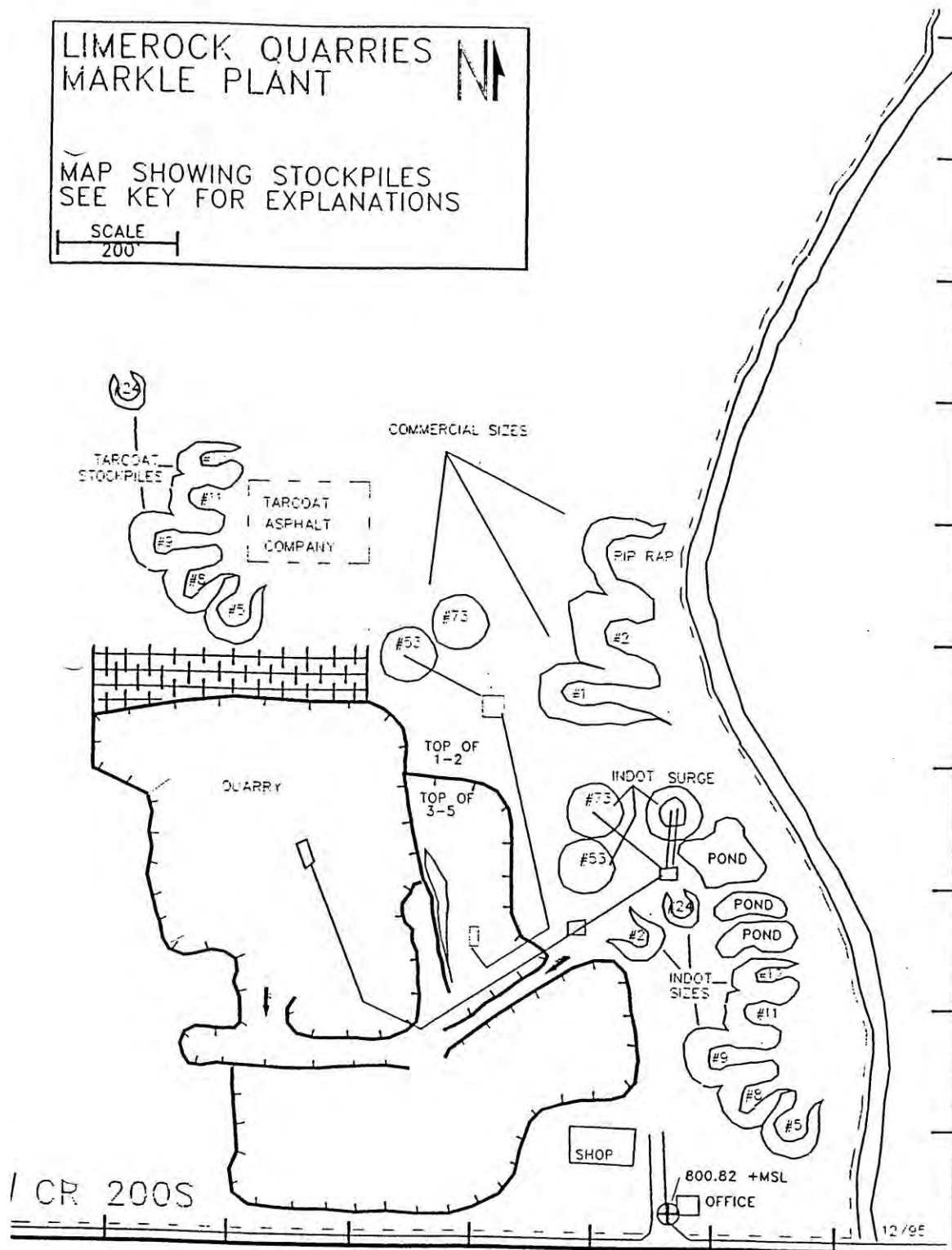
Elevations for all benches may be found on the following geologic cross-sections. A benchmark with an elevation of 800.82 ft. is located next to the office, which is southeast of the quarry. The proposed 1995 operating areas may be found on the following Source Map. Also included is the location map and quarry map indicating the stockpile areas.

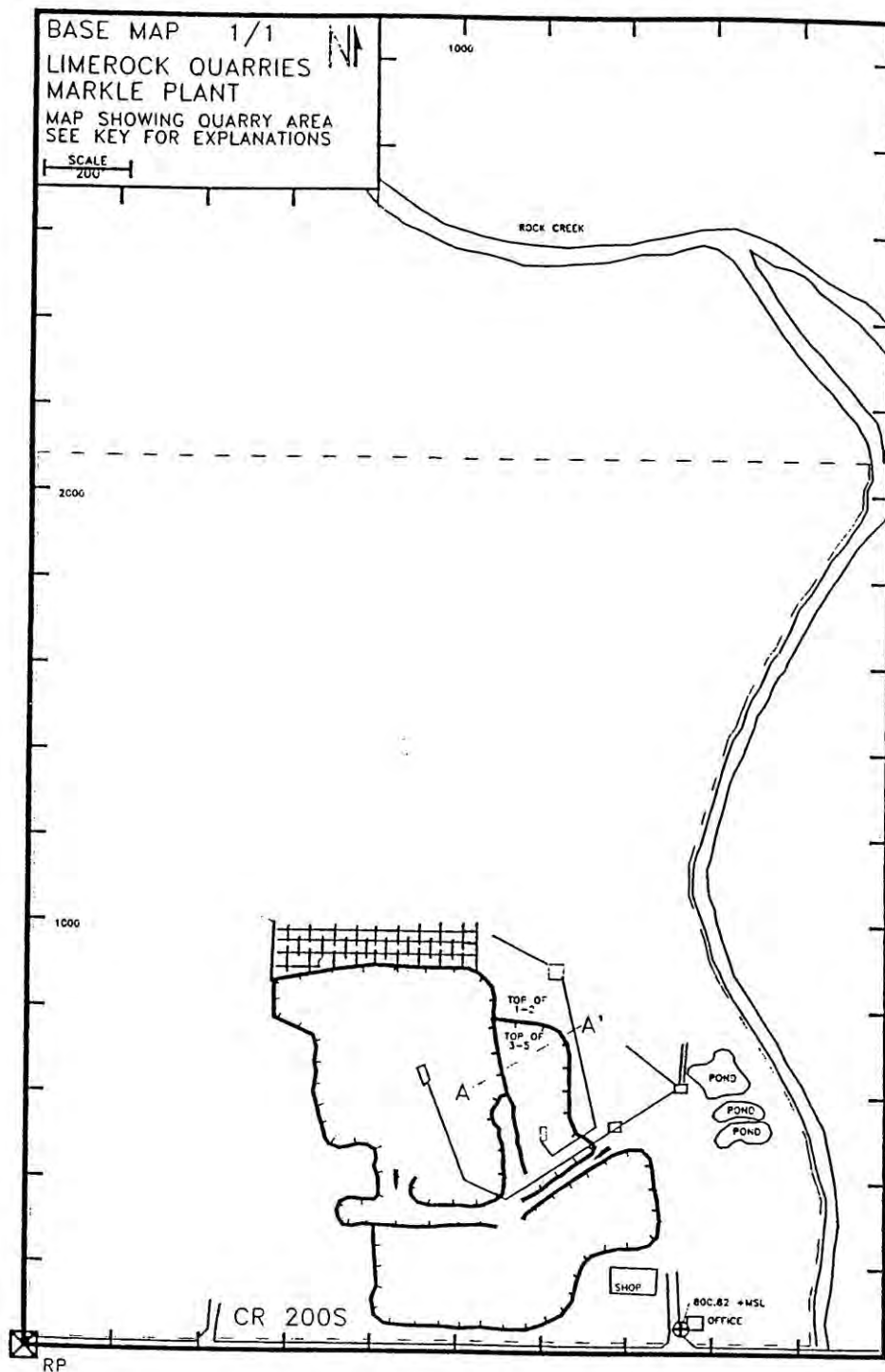


LIMEROCK QUARRIES  
MARKLE PLANT

MAP SHOWING STOCKPILES  
SEE KEY FOR EXPLANATIONS

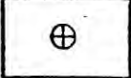




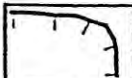

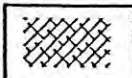
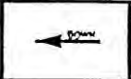
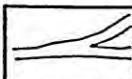
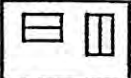
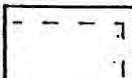

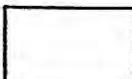

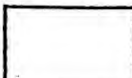



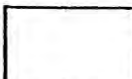


SCALE  
200'





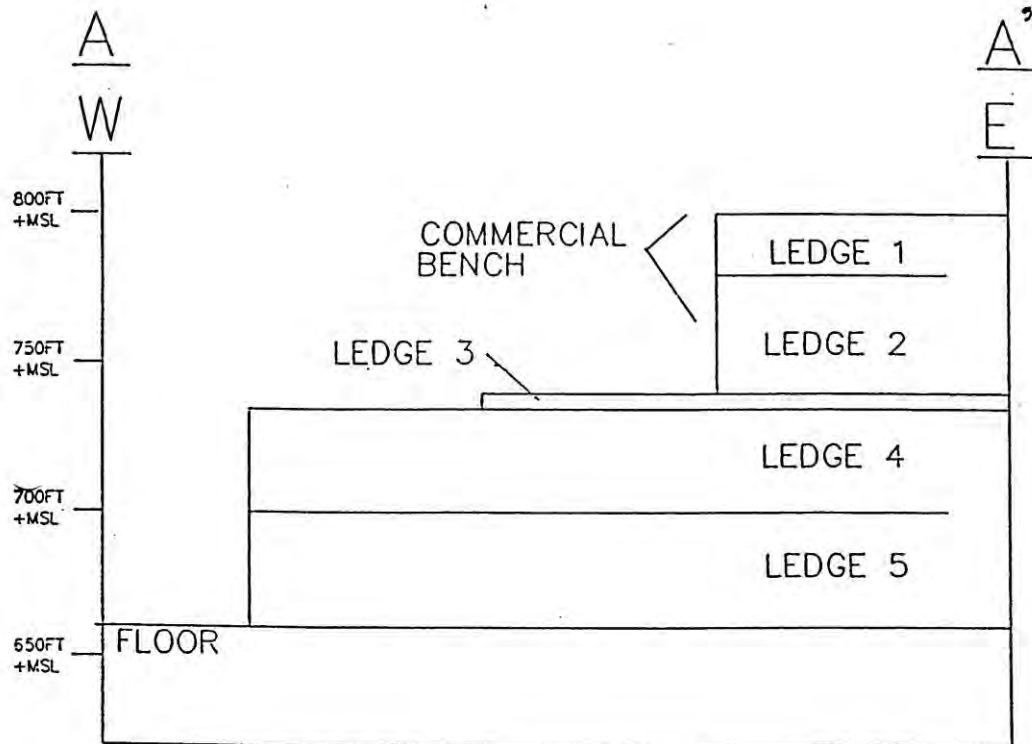
10-95

# KEY TO QUARRY MAP FOR INDOT LIMEROCK QUARRIES, INC. MARKLE PLANT

SYMBOL	EXPLANATION	SYMBOL	EXPLANATION
	PERMANANT BENCHMARK		TAILINGS / FILL
	NON-PERMANANT BENCHMARK		WATER BODIES
	REFERENCE POINT		QUARRY WALLS HANCHURES ON LOWER SIDE
	RAMP ARROW - UP		PROPOSED QUARRY AREA FOR CURRENT YEAR
	RAMP ARROW - DOWN		ROADS
	BUILDINGS		PROPERTY LINES
	PLANT STRUCTURES		
	INDOT STOCKPILES		
	COMMERCIAL STOCKPILES		
	MISC STOCKPILES		
	SURGEPILES		

12/95

# GEOLOGIC CROSS – SECTION FOR LIMEROCK QUARRIES, INC.



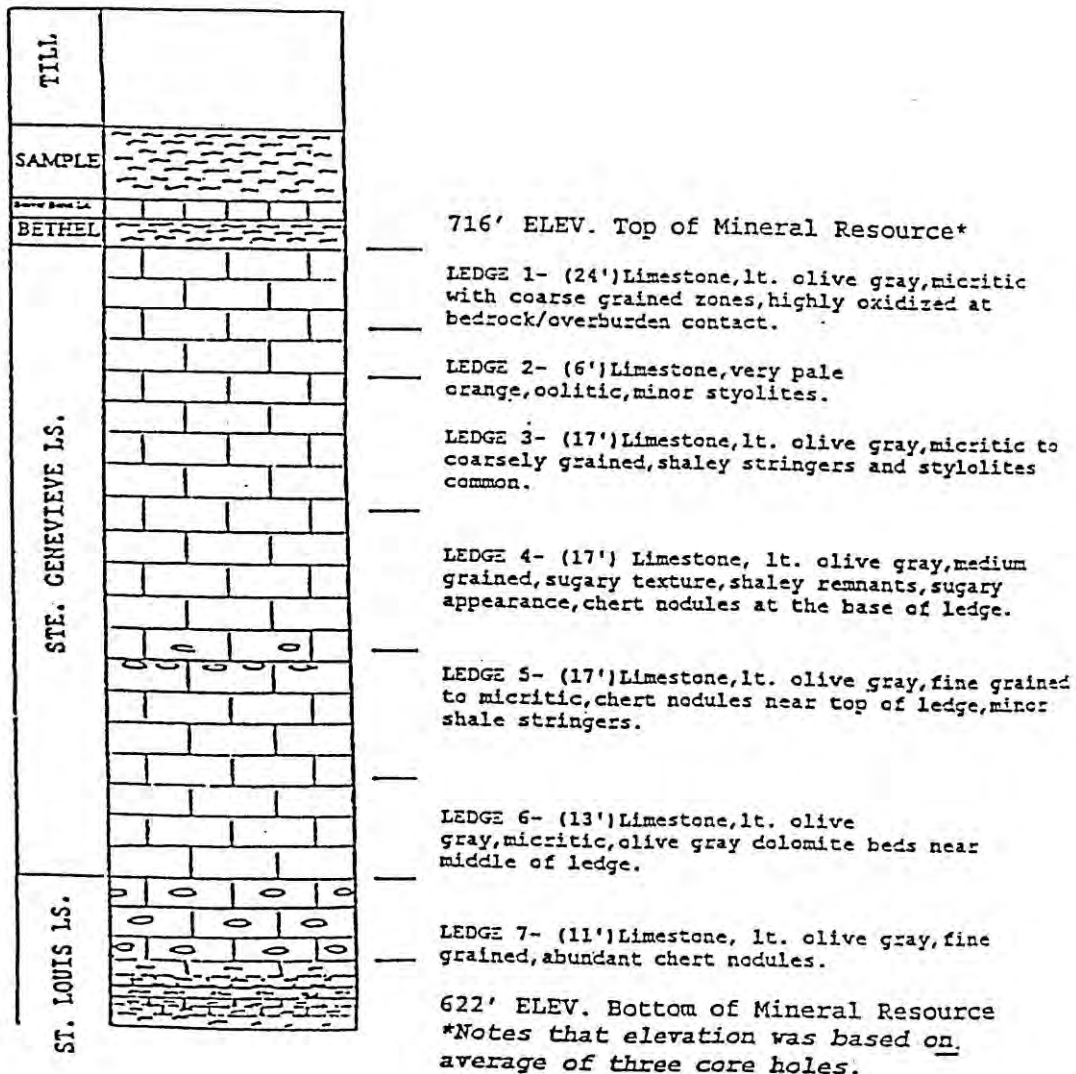
LEDGE	THICKNESS	GEOLOGIC FORMATION
1	20'	LISTON CREEK
2	40'	MISSISSINEWA
3	5'	WALDRON
4	35'	LOUISVILLE
5	38'	LIMBERLOST

MARKLE PLANT – #2799

12/95

# LIMEROCK QUARRIES, INC. MARKLE PLANT

## GENERALIZED STRATIGRAPHIC SECTION MARCH, 1996



## APPENDIX B

### AP PRODUCTION CONTROL PLAN



AP PRODUCTION CONTROL PLAN  
LIMEROCK QUARRIES, INC.  
INDOT SOURCE #2799

Limerock Quarries, Inc. is approved for Class AP aggregate, and produces and controls stone meeting this classification in accordance with the following:

1. The production area for the AP stone is Ledges 4 and 5.
2. AP stone will be processed in accordance with the procedures listed in Section 6.
3. The final production gradation for the AP stone will be INDOT size #8 gradation.
4. The AP stockpile will be identified by a sign indicating AP No. 8, Ledges 4 and 5, and the location and color of the size will be in accordance with Section 11. The No. 8 and AP 8 stockpiles will be separated by another stockpile of a different size.
5. AP stone delivered to concrete plants will be so identified on the aggregate weigh tickets.

## APPENDIX C

### FORMS



CAPP DIARY - ADDITIONAL REMARKS

SOURCE #:

DATE:

SUPERINTENDENT'S (OR REPRESENTATIVE) REMARKS - CONTINUED

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INITIALS

CAT'S REMARKS - CONTINUED

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(PRINTED NAME)

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(SIGNATURE)

RANDOM NUMBERS				PAGE 1
PLANT _____	MONTH _____	YEAR _____	SIZE _____	

**INSTRUCTIONS:**

- 1) RANDOMLY SELECT A STARTING ROW AND COLUMN. CIRCLE THIS VALUE.
- 2) USE THE NUMBER AT THAT POSITION TO CALCULATE THE TONNAGE AT WHICH A SAMPLE WILL BE PULLED.
- 3) AFTER SELECTING A STARTING VALUE, SUBSEQUENT VALUES WILL BE DERIVED BY SELECTING THE NEXT NUMBER IMMEDIATELY BELOW AND IN THE SAME COLUMN.
- 4) WHEN THE BOTTOM OF ANY COLUMN IS REACHED, ADDITIONAL NUMBERS WILL BE DERIVED BY MOVING TO THE TOP OF THE NEXT COLUMN IMMEDIATELY TO THE RIGHT OF THE CURRENT COLUMN.
- 5) WHEN THE LAST NUMBER IN THE LOWER RIGHT-HAND CORNER HAS BEEN USED, ADDITIONAL NUMBERS WILL BE DERIVED BY MOVING TO THE UPPER LEFT-HAND CORNER OF THE TABLE.
- 6) WHEN THE PRODUCTION RUN IS COMPLETED, REPEAT STEPS 1-6 FOR NEXT RUN.

**RANDOM NUMBER TABLE**

.84	.28	.26	.14	.77	.03	.76	.89	.55	.86	.92
.78	.37	.17	.13	.79	.73	.85	.75	.25	.67	.96
.95	.19	.94	.11	.98	.71	.64	.81	.38	.62	.23
.49	.08	.58	.65	.69	.57	.83	.44	.51	.04	.16
.31	.40	.15	.35	.43	.91	.48	.50	.74	.80	.24
.63	.02	.42	.05	.97	.87	.30	.39	.12	.66	.10
.54	.90	.56	.68	.29	.22	.21	.72	.33	.47	.60
.61	.45	.18	.41	.70	.01	.20	.93	.32	.52	.06
.82	.53	.99	.88	.36	.46	.09	.59	.27	.35	.07

**TONNAGE CALCULATIONS**

(F) = FREQUENCY = \_\_\_\_\_ (TONS)

(NOTE: (F) < 99 X (TC))

(TC) = HAUL-TRUCK CAPACITY \_\_\_\_\_ (TONS)

TABLE VALUE (V)	SAMPLE TONNAGE CALCULATIONS (V) X (F) = ST	FREQUENCY RUNNING TOTAL (FRT)	ACCUMULATED SAMPLE TONS (ST)+(FRT)= (AST)	SAMPLE LOAD NUMBER = (AST) / (TC)
		00000000000000		

FN:RANDNUM

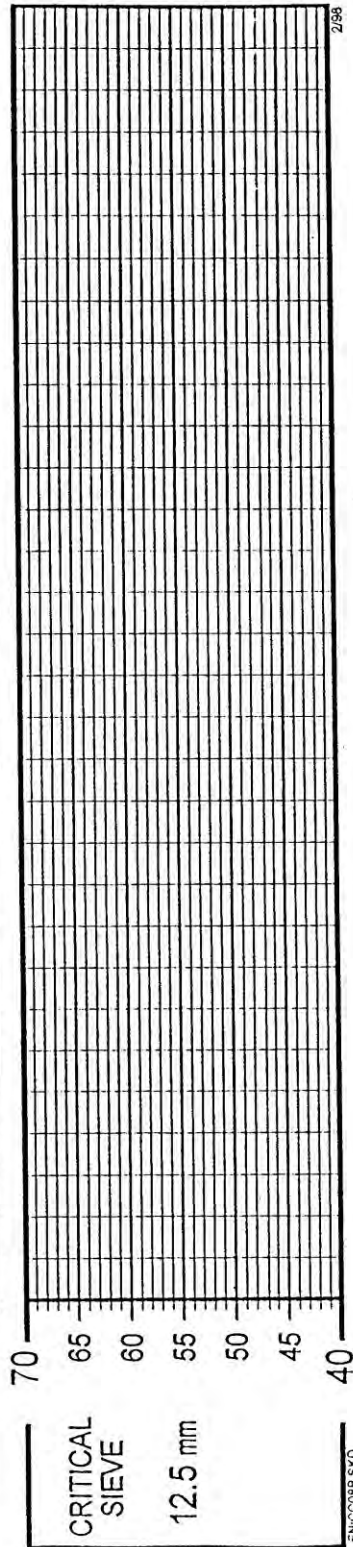
GRADATION ANALYSIS FOR SIZE 8																
SOURCE _____										ORIGIN _____					MONTH / YEAR _____	
DATE															WHEN CONTROL CHARTING IS REQUIRED, PLACE A CIRCLE AROUND THE ENTRY IN THE *TEST TYPE* BOX	
TEST#															◁	
* TYPE																
SIEVE															AVE SPEC RANGE	
25.0 mm															100	
19.0 mm															75-95	
12.5 mm															40-70	
9.5 mm															20-50	
4.75 mm															0-15	
2.36 mm															0-10	
75 μm																
DECANT															0-1.5	
															*SAMPLE TYPES	
CHERT															S = START OF PRODUCTION FREQUENCY	
SOFT															N = NORMAL PRODUCTION FREQUENCY	
LA															L = LOADOUT FREQUENCY	
% ABS															M = MISCELLANEOUS	
BSGR															R = RESAMPLE	
CRUSH															STATISTICS	
															UCL = _____	
															LCL = _____	
															$\bar{X}_t =$ _____	
5-POINT AVERAGE																

0497

PM 1504-100

# PROCESS CONTROL CHART - 8

SIEVE SIZE SPEC. RANGE  
 DATE/TEST# PLANT NUMBER MONTH/YEAR



EXPLANATION			
— — — —	TARGET MEAN ( $\bar{X}_T$ ) (RED INK)	○ — ○ — ○	NORMAL PRODUCTION DATA (RED INK)
— — — —	CONTROL LIMITS. (+) and (-) TWO	□ — □ — □	*LOAD-OUT DATA (BLACK INK)
— — — —	STANDARD DEVIATIONS ( $C_{n-1}$ )	△ — △ — △	5-POINT MOVING AVERAGE (BLUE INK)
— — — —	FROM THE TARGET MEAN (RED INK)	* - Load-out data are connected when they are displayed on a graph separate from the production data.	
TARGET MEAN ( $\bar{X}_T$ ) = _____		UPPER CONTROL LIMIT (UCL) = _____	
		LOWER CONTROL LIMIT (LCL) = _____	





## AUTHENTICATION

APPROVAL

SUBMISSION

\_\_\_\_\_  
Chief, Materials & Tests Division

\_\_\_\_\_  
Management Representative

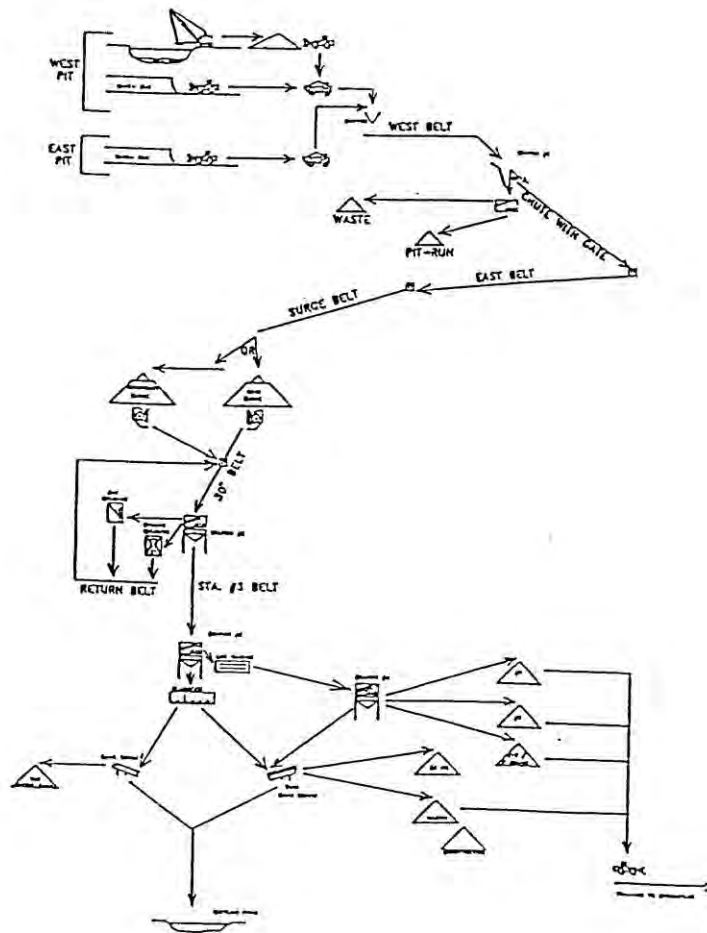
\_\_\_\_\_  
Date of Approval

\_\_\_\_\_  
Date of Submission

# QUALITY CONTROL PLAN

INDIANA QUALITY SAND & GRAVEL, INC.

INDOT SOURCE # 2000



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County  
Township  
Range  
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Locations  
Sampling Devices and Techniques

### **SECTION 8 - TESTING PLAN**

Gradation  
Decantation  
Crushed Particles  
Deleterious Material  
Non-Conforming Materials

### **SECTION 9 - GRADATION CONTROL**

Critical Sieves  
Target Mean Values  
Standard Deviations

### **SECTION 10 - PROCESS CONTROL TECHNIQUES**

## **SECTION 11 - DOWNSTREAM CONTROL**

- Identification of Stockpiles
- Stockpile Construction
- Material Retrieval

## **SECTION 12 - LABORATORY**

- Location
- Equipment
- Calibration

## **SECTION 13 - DOCUMENTATION PLAN**

- Reference Documents
- Diary
- Test Data
- Control Charts
- Material Shipment Record

## **SECTION 14 - ADDENDA**

### **APPENDIX A - AP PRODUCTION CONTROL PLAN**

### **APPENDIX B - GENERAL INFORMATION**

- Location Map - General
- Stockpile Map
- Pit Area Maps

### **APPENDIX C - FORMS**

- Daily Diary
- Gradation Analysis Form
- Control Chart
- Shipping Record Form

## **AUTHENTICATION**

## SECTION 1

### **PLANT LOCATION**

Indiana Quality Sand and Gravel, Quality Plant (2000) is classified as a Plant/Redistribution Terminal for CAPP purposes. The location of the plant is indicated on the map in Appendix B. The address and other pertinent information is as follows:

Indiana Quality Sand and Gravel  
(INDOT #2000)  
6593 Gravel Rd.  
Marion, IN 46713

Latitude - 40E 30, 33"  
Longitude - 85E 37, 05"  
County - Grant  
Township - T 24 N  
Range - R 8 E  
Sections - 21, 27 and 28

Office Phone No. - 317-675-3421  
Fax - 317-675-8221  
Lab - 317-992-7681

The Indiana Quality Sand and Gravel pit is situated on both sides of Gravel Road, north of Gas City. To locate the pit follow First Street north approximately 2 miles from the junction of SR 22/35. The pit entrance is on the west side of the road.

Indiana Quality Sand and Gravel is owned by Gravel City, Inc. which is located at the following address:

Gravel City, Inc.  
618 S. 7th St.  
Vincennes, IN 47591  
812-886-4871  
FAX: 812-886-4881

## **SECTION 2**

### **ORGANIZATIONAL STRUCTURE**

#### **MANAGEMENT REPRESENTATIVES**

The Superintendent for Indiana Quality Sand and Gravel and Management Representative for this pit is Ron Limestone. He is responsible for all production for this site.

#### **CERTIFIED TECHNICIANS**

The CAPP Certified Aggregate Technician responsible for the location is Tony Shale. His duties include testing and reporting results from this site as well as assisting with the CAPP duties at several other sites within the Indiana Quality Sand and Gravel organization. Mr. Shale communicates all CAPP concerns to Mr. Limestone who then takes appropriate actions.

Other Quality Control Personnel include Mary Slag and Gary Siltstone. Ms. Slag's duties include sampling and reporting test results for three sites within Indiana Quality Sand and Gravel. She may also assist at this site as the need arises. Mr. Siltstone is a CAPP Certified Aggregate Technician who will substitute for Mr. Shale when necessary.

### SECTION 3

#### MINERAL DEPOSITS

The gravels and sands that are mined at this location are all derived from quaternary sediments that were reworked from earlier glacial deposits. These types of sediments are consistent with deposits that would be found on the inside curve of a large meander loop (in this case from the Mississinewa River). Since the entirety of this deposit is situated on the floor of the Mississinewa valley, this is probably correct. In addition, the proximity of Walnut Creek would lead one to suspect that it is (in part) responsible for some of the reworking and sorting of this deposit. The dry-run areas are mined to a depth of approximately 25' and the wet-run areas can be mined an additional 40'.

The gravels and sands produced at this source may come from areas on the west side of Gravel Road or a newly-opened area on the east side of Gravel Road (see diagrams of pits - Appendix B).

Materials originating from the west side of Gravel Road may come from any combination of different points of extraction. Materials can be selectively taken from these areas as a first measure in controlling the gradation. Depending on what products we are making, we will remove materials from any of these areas.

Materials originating from the east side of Gravel Road are from one area only. This is a newly opened area and has not been developed very much at this point. This side, so far, is bank-run.

#### **SECTION 4**

##### **AP AGGREGATE**

The gravel at this source is approved for Class AP aggregate production as indicated on the approval letter dated October 21, 1994. The AP Production Control Plan is included in Appendix A.



## SECTION 5

### **MATERIAL CATEGORIES**

#### **STANDARD SPECIFICATION**

Aggregates that are categorized as Standard Specification materials include the following INDOT sizes:

- #AP8 Gravel
- #23 Natural Sand
- #8 Stone

This site is a Redistribution Terminal for #8 stone, which is used and stockpiled at the concrete plant on site owned by Indiana Quality Sand and Gravel. The stone comes from Stone City, Bedrock, Indiana (#2805) which is not a Certified Aggregate producer.

#### **ALTERNATE**

Aggregates that are categorized as Alternate materials include the following materials:

- #4 Gravel
- #12 Gravel
- P-Gravel
- Processed Pit-Run
- Mason Sand
- Construction Sand
- Top-Dressed Sand

## SECTION 6

### **PRODUCTION FLOW DIAGRAM**

Aggregates produced at this source are from two main areas - East Pit and West Pit. The East Pit is newly opened and presently consists of one main mining area which is dry-run. The West Pit may be mined at several points - one of which is dry-run and the others are wet-run.

The East-Pit is presently restricted to dry-run materials. All materials removed from this area are placed onto haul trucks and transported to the processing plant on the west side of Gravel Road.

The West-Pit is divided into dry and wet-run areas. Materials removed from the dry-run areas are loaded directly into haul trucks. Materials removed from the wet-run areas are allowed to drain before being placed onto haul trucks. Once both materials are placed onto haul trucks, they are transported to the processing plant.

Materials arriving at the processing plant, from any of the mining areas, are all processed the same. The plant consists of four stations (see overall Flow Diagram on the following pages). Each station is set up to perform specific tasks within the overall process.

#### **STATION #1**

This station is used only when we are making processed pit-run materials. Materials are initially placed over a Deister 5' x 10', 1-deck vibratory screening unit. Oversized materials are dropped off to the side where a loader removes them to the waste area. Materials passing through the screen are directed to the pit-run stockpile by the processed pit-run conveyor as a finished material.

When not making any processed pit-run materials the aggregate is by-passed onto the east conveyor, which transports the material to either the main surge pile or the secondary surge pile by way of the surge belt.

#### **STATION #2**

Station #2 is used to pre-screen and crush the materials before they are sent on to Station #3. Materials arriving at this station come from either or both surge piles by way of the 30" belt. The materials coming from the main surge pile are placed directly on the 30" conveyor while the material from the secondary surge pile is placed onto the 30" belt at a transfer point.

Materials at this station are put over a Deister 5' x 10', 2-deck vibratory screening unit. This screening unit sits above a single bin that merely funnels materials onto the Station #3 conveyor.

Oversized materials are directed to a Lippmann 15" x 36" jaw crusher. Here they are crushed and closed-circuited back to Station #2. They first drop onto the return belt which then drops materials onto the 30" belt at a transfer point.

Materials passing the top deck and retained on the bottom deck are directed to a Symons 3' standard cone crusher. Here they are crushed and closed-circuited back to Station #2. They first drop onto the return belt which then drops materials onto the 30" belt at a transfer point.

Materials that pass through both screens drop into the bin and are then directed to Station #3.

### **STATION #3**

Station #3 is used to separate most of the sand from the coarser materials that are going on to Station #4. Materials arriving at this station are placed over a Deister 5' x 12', 2-deck vibratory screening unit. This station is also a wash station and includes a log washer and a classifier.

Depending on what size will be produced on the bottom deck at Station #4, the sand panels on the bottom deck of this station will be set up in one of the following two ways.

1. When making #12 gravel, the panels on the bottom deck will be configured to produce mason/construction sand.
2. When making P-gravel, the panels on the bottom deck will be configured to produce #23 sand.

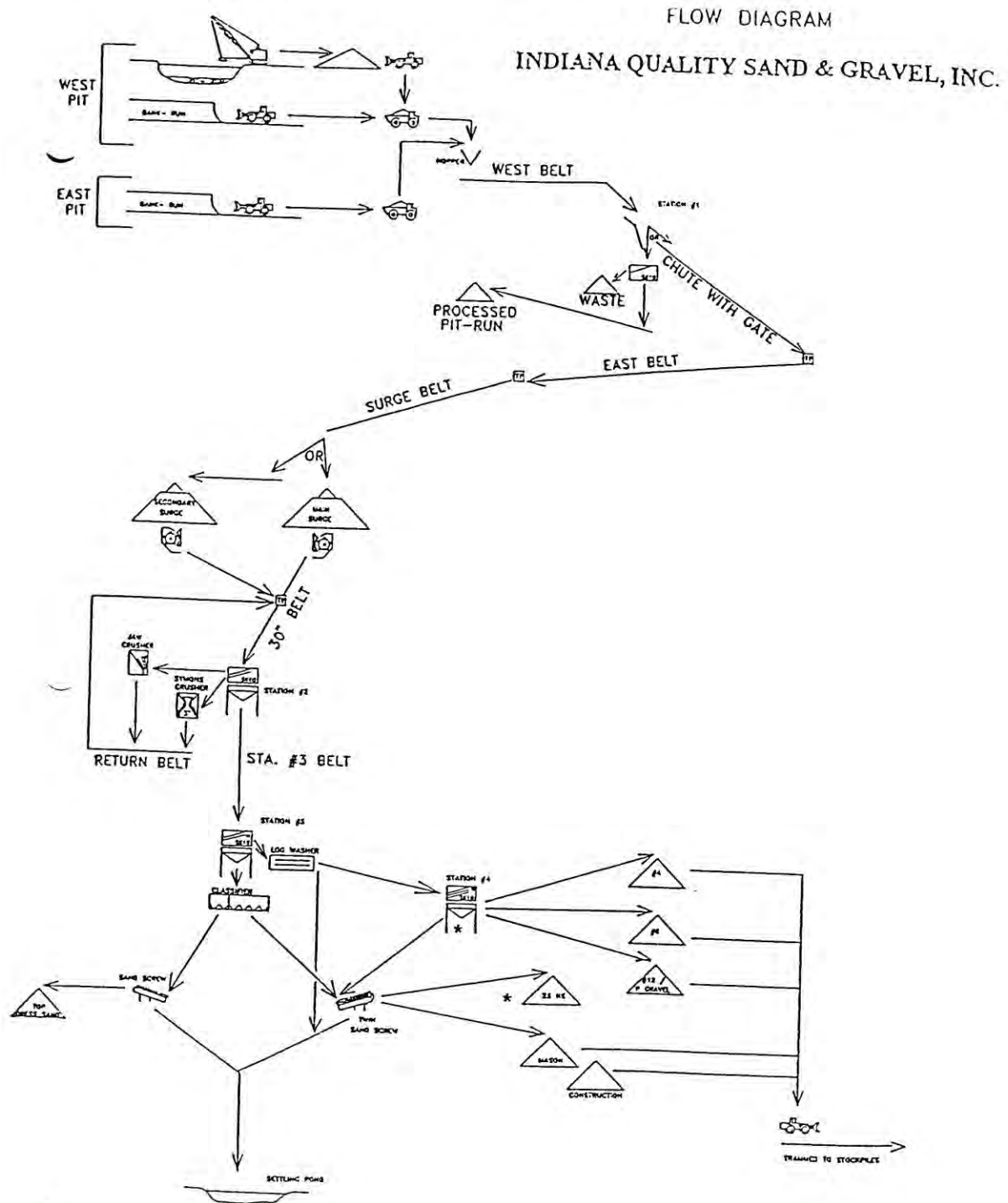
All materials that do not pass through the bottom deck are directed to the Eagle Iron Works log washer. At the log washer, materials are pre-washed in preparation for their arrival at Station #4. Waste-water exits the log washer and is directed to the twin screw at Station #4.

All materials that pass through the bottom deck are directed to the Eagle Iron Works classifier. At the classifier, fine materials are sent to the McClanahan 36" single sand screw and come off as top-dress sand. At the classifier, coarse materials are sent to the Eagle Iron Works 36" twin sand screw and come off as either #23 or mason/construction sand. This is the same screw to which materials from Station #4 will be directed. From either screw, the waste water is directed to the settling ponds.

#### STATION #4

Washed products are made at Station #4. Materials arriving at this station are put over a Deister 5" x 10', 3-deck vibrating screening unit. Since this material has been pre-screened, each bin partition will contain the following finished products:

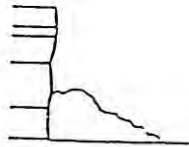
1. Top-sized materials are size #4 and drop into the first bin partition. They are scooped up with the front-end loader and trammed to the stockpile.
2. Materials passing the top deck and retained on the middle deck are size #8 and drop into the middle bin partition.
3. Materials passing the middle deck and retained on the bottom deck are #12 size or P-gravel and drop into the third bin partition. When set up to make mason/construction sand at Station #3, this station will make #12 gravel. When set up to make #23 sand at Station #3, this station will make P-gravel.
4. Material passing through all of the screens is sent to the sand screw. At the sand screw, one of three types of sand may be produced. When making #23 sand, the sand coming off the screw drops onto a conveyor belt and is taken directly to the #23 sand stockpile. When making mason sand or construction sand, the sand coming off the screw is diverted from the #23 conveyor and is dropped off onto the mason sand belt and sent to the tram pile to await stockpiling. Waste water from the sand screws is sent to the settling ponds.



11/95

\* Point of Sampling

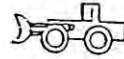
# ALTERNATE TEMPLATE



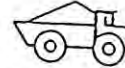
SHOT PILE



SETTLING POND



LOADER



HAUL TRUCK



IMPACT CRUSHER



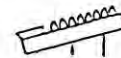
GYRATORY CRUSHER



SYMONS CRUSHER



SURGE FEEDER



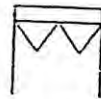
SAND SCREW



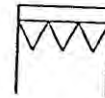
5x14  
3-DECK  
SCREEN



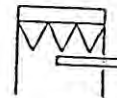
5x14  
3-DECK  
WASH  
SCREEN



2-BIN  
STATION



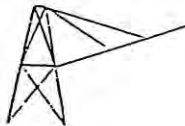
3-BIN  
STATION



3-BIN  
STATION  
WITH SIDE  
DISCHARGE



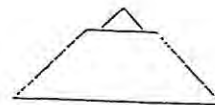
CONVEYOR  
TOWER



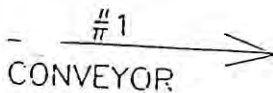
RADIAL  
STACKER



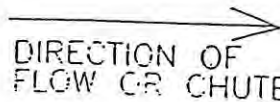
STOCKPILE



SURGE PILE

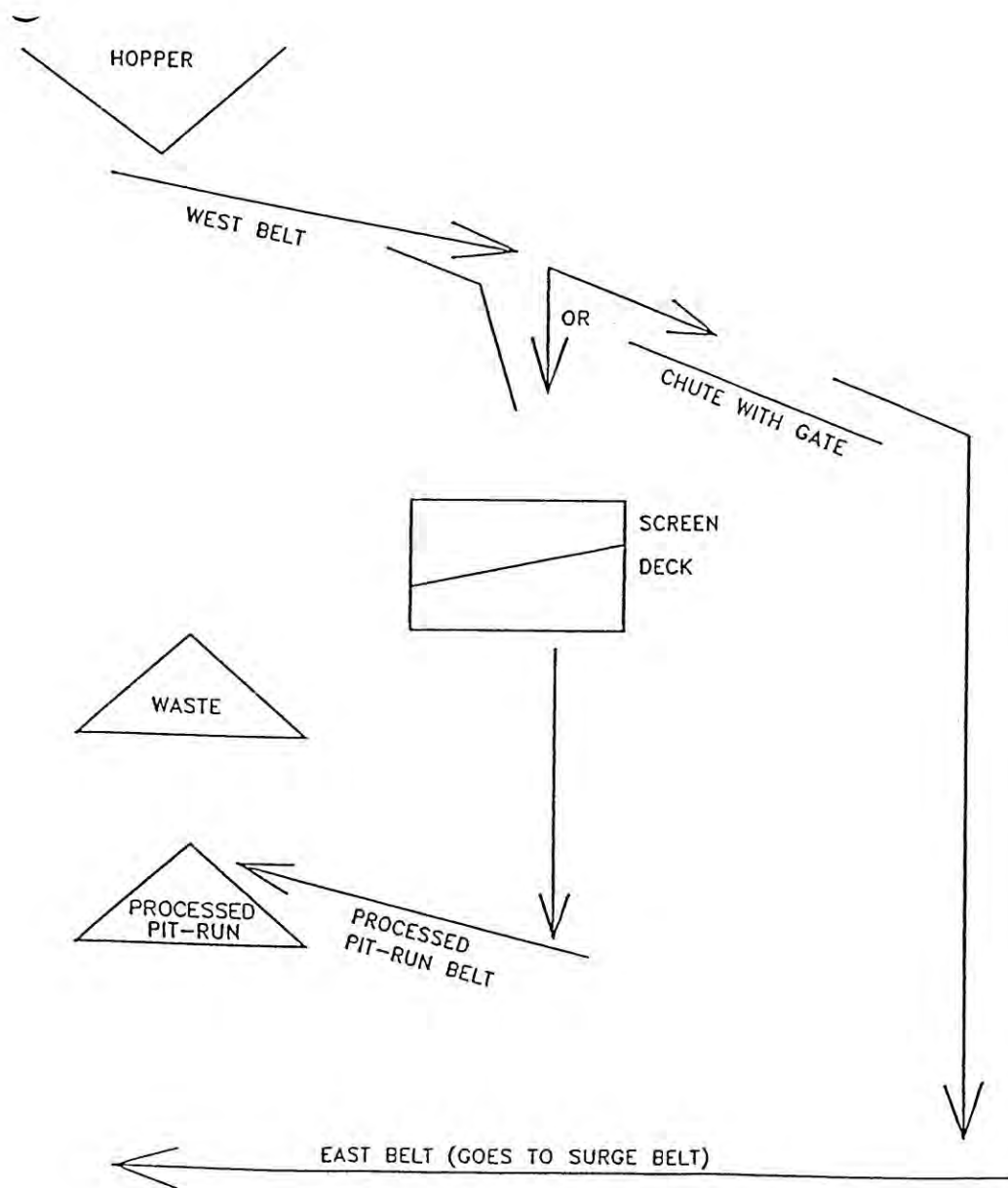


CONVEYOR



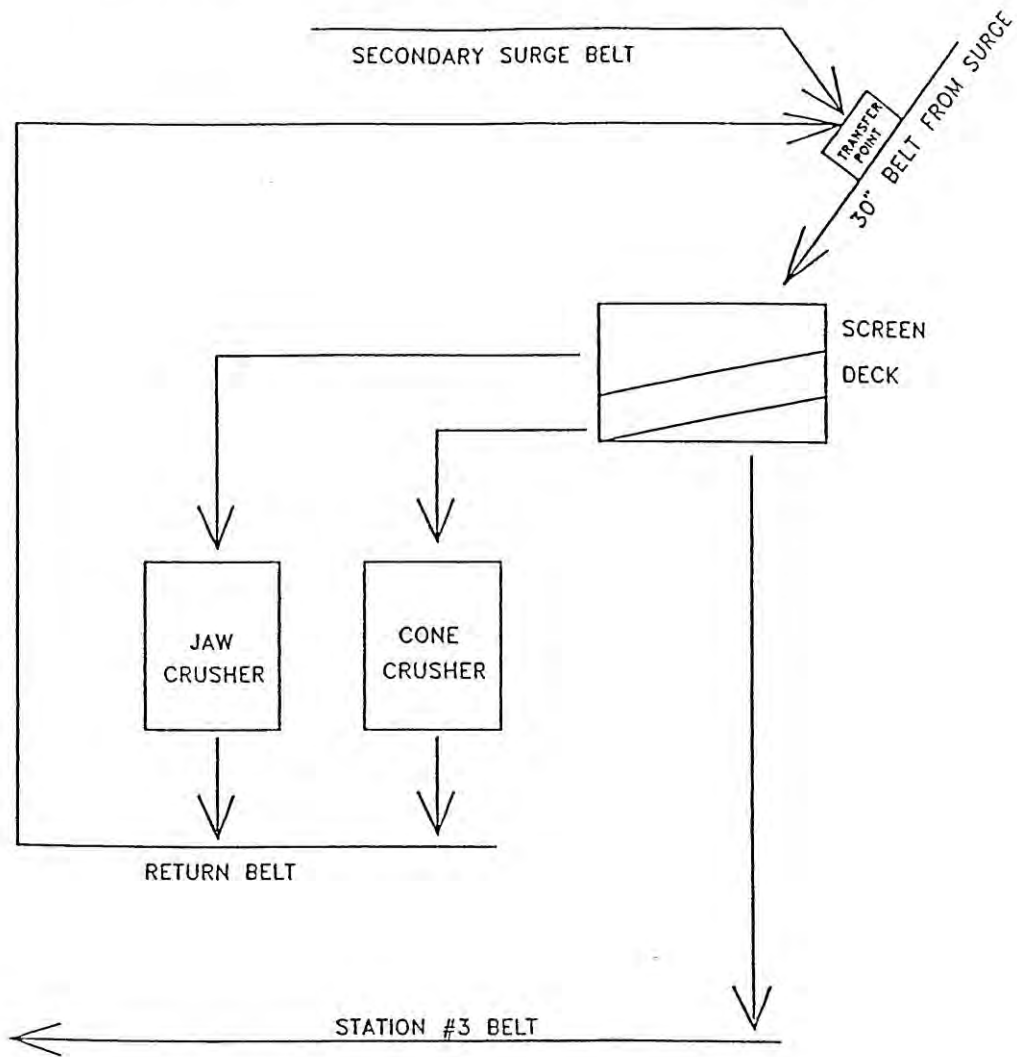
DIRECTION OF  
FLOW OR CHUTE

# FLOW DIAGRAM FOR #1 STATION



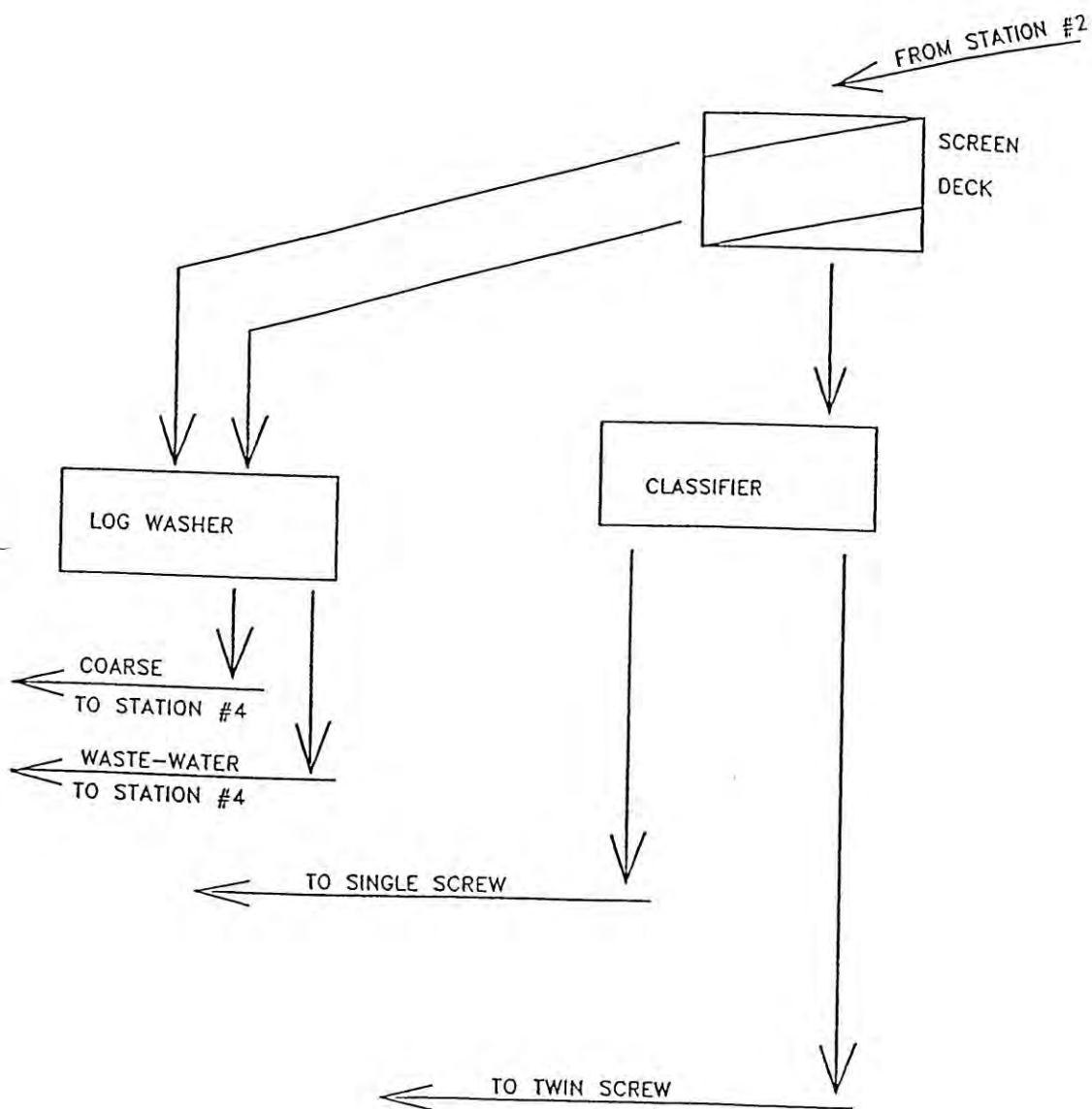
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# FLOW DIAGRAM FOR #2 STATION



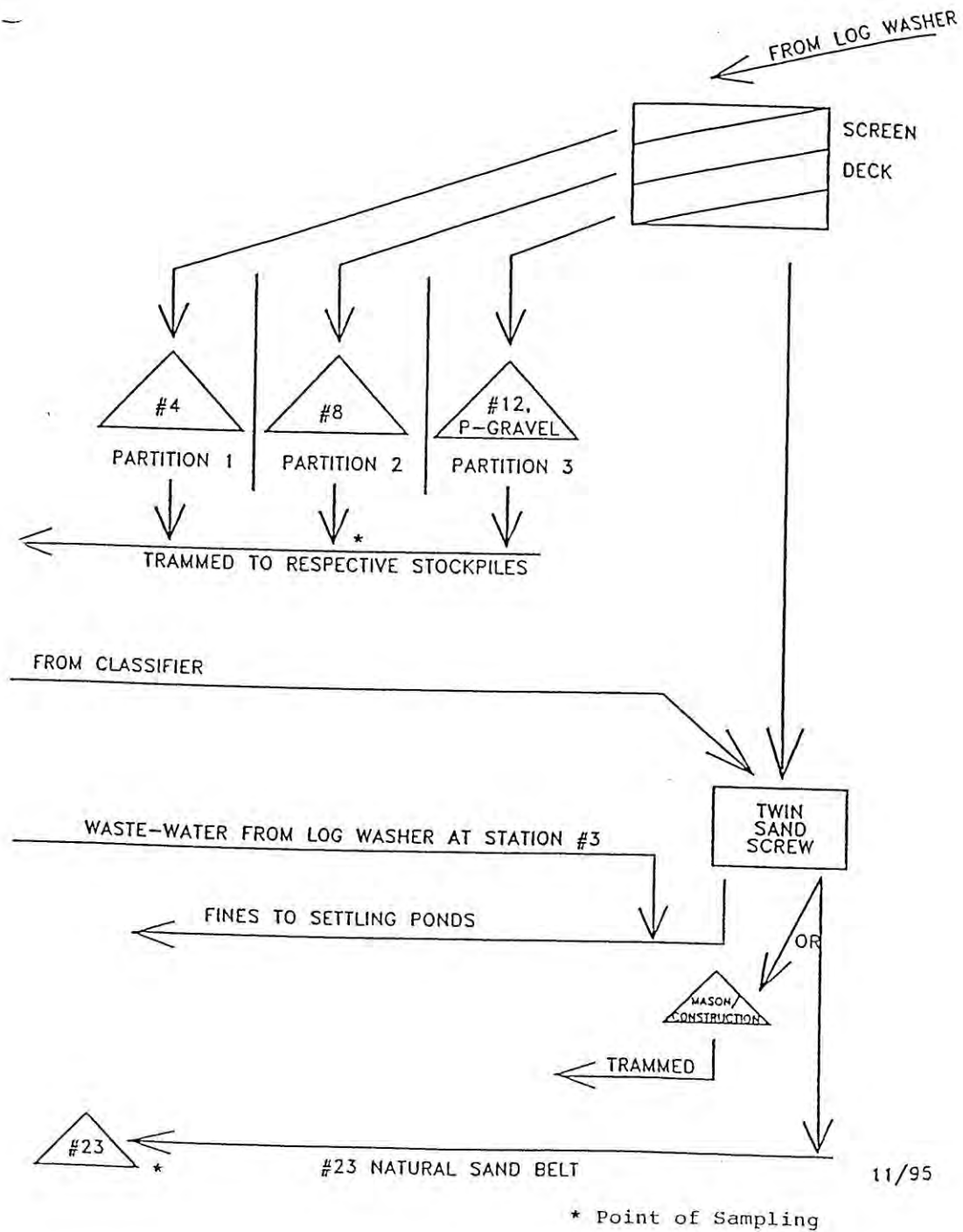


FLOW DIAGRAM FOR #3 STATION



11/95

# FLOW DIAGRAM FOR #4 STATION



## **SECTION 7**

### **SAMPLING PLAN**

Indiana Quality Sand and Gravel has developed a coding scheme to distinguish the 5 different types of samples that are obtained. The type of samples, frequency of sampling, location of samples and sampling procedures of any Certified Materials are included as follows.

#### **TYPE OF SAMPLES AND FREQUENCY**

(S) Start of Production. After a seasonal shutdown or when producing a new material, start of production samples shall be obtained once every 1000 t for the first 5000 t, but shall not exceed 2 per calendar day.

(N) Normal Production. After the start of production samples have been completed for each material, normal production samples shall be obtained. The frequency of these samples shall be once every 2000 t, but shall not exceed 2 per calendar day.

(L) Load-Out. Load-out samples shall be taken from material that is shipped. The frequency of these samples shall be once every 8000 t; however, there shall be at least one sample taken each month for any Certified Material shipped that exceeds 1000 t.

(M) Miscellaneous. Miscellaneous samples are taken at our own discretion for information purposes outside the start of production or normal production samples.

(R) Resample. When there is a failing normal production or load-out test a resample shall be taken.

#### **MEANS OF TRACKING SAMPLES**

Start of production and normal production samples shall be taken from uniform tonnage increments in an unbiased manner. The belt feed rates shall be used to estimate the quantity produced. The Superintendent shall be responsible for communicating with production staff as to when to obtain samples.

Shipping tonnages shall be kept by the office bookkeepers to determine when the load-out samples are to be obtained. The bookkeepers shall inform the Superintendent when a sample is required.

## **SAMPLE LOCATIONS**

All start of production and normal production samples shall be taken from the following locations.

1. #8 GRAVEL - stockpiles made by a front-end loader with material from the bin partitions.
2. #8 STONE - the active area of the finished stockpile.
3. #23 SAND - the active area of the finished stockpile.

Load-out samples shall be taken from the Certified Material stockpiles.

The points of sampling for all samples are indicated on the flow diagram on page 6-4.

## **SAMPLING PROCEDURES**

The #8 stone and #23 sand samples shall be obtained using the procedure for stockpile sampling as set out in the ITM 207. Samples of the #8 gravel from the bin partition shall be obtained by first emptying the bin and then passing a container through the discharge stream.

## **SECTION 8**

### **TESTING PLAN**

#### **GRADATION**

Gradation analysis shall be performed in accordance with AASHTO T 27 on all start of production, normal production and load-out samples. A gradation test shall be performed on resample and miscellaneous samples when necessary.

#### **DECANT**

Decant tests shall be performed in accordance with AASHTO T 11 on all load-out samples.

#### **CRUSHED PARTICLES**

Crushed particle content for the #8 gravel shall be determined in accordance with ASTM D 5821 at least once per week for the start of production and normal production samples. No test shall be performed if the week's production is less than 100 t.

#### **DELETERIOUS**

The percent of deleterious materials shall be determined in accordance with AASHTO T 112, ITM 206, and the Standard Specifications at least once per week for each size of material for the start of production and normal production samples. No test shall be performed if the week's production is less than 100 Mg.

Previous testing has indicated that there are high levels of light-weight chert at this source. For this reason, when the total chert exceeds the specification limit for light-weight chert the District Materials and Tests Engineer shall be notified and samples shall be obtained for INDOT to test for light-weight chert.

#### **NON-CONFORMING MATERIAL**

Any time there is a failing normal production or load-out test the Superintendent shall be notified immediately and a resample test taken. Typically, retests shall be accompanied by a visual check for any problems at the plant. All actions shall be documented in the Daily Diary.

In the event that a second consecutive normal production sample fails, the materials will be diverted until the problem is corrected as follows:

1. No. 8 gravel and No. 8 stone shall be taken to the scrap pile and wasted.
2. No. 23 sand shall be diverted to the mason/construction sand conveyor.

In the event that a second consecutive load-out sample fails, shipping from that stockpile shall cease. The stockpile problem area shall be checked to determine if the stockpile can be remixed and restored within the quality control limits as verified by the resample tests. If the problem area cannot be remixed, the material shall be removed and taken to the scrap pile.

## SECTION 9

### GRADATION CONTROL

#### NO. 8 GRAVEL

1/2 in Critical Sieve

$\bar{x} = 49.0\%$

$\sigma_{n-1} = 5.0$

Upper Control Limit = 59.0%

Lower Control Limit = 39.0%

#### NO. 8 STONE

1/2.in. Critical Sieve

$\bar{x} = 47.5\%$

$\sigma_{n-1} = 4.0$

Upper Control Limit = 55.5%

Lower Control Limit = 39.5%

## SECTION 10

### **PROCESS CONTROL TECHNIQUES**

Los Angeles abrasion and absorption tests may be performed when deemed necessary and shall be posted on the Gradation Analysis Form (Appendix C). A visual check of all stockpiles is an ongoing daily procedure.



## **SECTION 11**

### **DOWNSTREAM CONTROL**

#### **IDENTIFICATION OF STOCKPILES**

All stockpiles shall be marked using signs in front of each stockpile that indicate the size of each material. For Standard Specification stockpiles, the signs shall be blue with white lettering and for Alternate stockpiles, the signs shall be red with white lettering.

#### **STOCKPILE CONSTRUCTION**

The #8 gravel stockpile is constructed by tramming materials with a front-end loader from the bin partitions.

The #8 stone stockpile is constructed by unloading truck loads side by side and then stacking the material only as high as the front-end loader can place the material.

The #23 sand is deposited directly onto the stockpile from the #23 sand belt.

#### **MATERIAL RETRIEVAL**

The entire front face of each stockpile shall be worked by a front-end loader from side to side when loading the truck. The sides of the face shall be occasionally mixed with the center to prevent segregation of the stockpile.

## SECTION 12

### **LABORATORY**

#### **LOCATION**

The laboratory is located in the southwest corner of the shop building, which is situated east of the plant. The following verified equipment is maintained in the laboratory:

#### **EQUIPMENT**

##### Sieve Analysis

Gilson TS-1 shaker

15 in. x 23 in. screens (2 in. (50 mm), 1½ in. (37.5 mm), 1 in. (25 mm), ¾ in. (19.0 mm), ½ in. (12.5 mm), ⅜ in (9.5 mm), No. 4 (4.75 mm) and pan)

Gilson Ro-Tap shaker

8 in. round sieves (¾ in. (9.5 mm), No. 4 (4.75 mm), No. 8 (2.36 mm), No. 16 (1.18 mm), No. 30 (600 µm), No. 50 (300 µm), No. 100 (150 µm), No. 200 (75 µm) and pan)

##### General

Humboldt oven

Ohaus IP12KS Digital Electronic Balance  
(12000 g capacity)

Mettler H10 Electronic Balance  
(160 g capacity)

#### **CALIBRATION**

The balances, mechanical shakers, oven, and sieves shall be verified in accordance with the following:

<u>Equipment</u>	<u>Minimum Frequency</u>	<u>Procedure</u>
Balances	12 mo.	ITM 910
Mechanical Shakers	12 mo.	ITM 906
Sieves	12 mo.	ITM 902

## **SECTION 13**

### **DOCUMENTATION PLAN**

Several forms have been developed for the CAP program and all information regarding the CAPP shall be entered on these forms. Examples of these forms may be found in Appendix C.

#### **REFERENCE DOCUMENTS**

The following documents are on file at the lab:

1. INDOT Certified Aggregate Producer Program (ITM 211)
2. INDOT Standard Specifications and Current Supplemental Specifications
3. INDOT Inspection and Sampling Procedures for Fine and Coarse Aggregate
4. Indiana Quality Assurance Certified Aggregate Technician Training Manual for Producer Technicians
5. Summary of Production Quality Results
6. AP Aggregate Letter
7. Quality Control Plan

#### **DIARY**

The diary is located in the Superintendent's office. One page is devoted to each day of the year that there is a material related operation, and all pages are maintained in a 3-ring binder.

#### **AGGREGATE INSPECTOR RECORD BOOK**

Each aggregate inspector working for this company is issued a number which is unique to that individual. Test data is recorded in the Aggregate Inspector Record Book and is traceable to any inspector through the identification number. This document is located in the laboratory.

#### **GRADATION ANALYSIS FORM**

This form is used for a quick visual comparison of up to 16 separate gradations of like materials. There is a different version of this form for each size of CAPP material including a generic version that may be used for any other material. This document is located in the laboratory.

#### **CONTROL CHARTS**

Control charts for each size material are posted on the wall in the laboratory.

## **INDOT MATERIAL SHIPPING RECORD FORM**

This form was developed to assist our bookkeepers in keeping track of how many tons of each material are shipped from the Certified Material stockpiles. This form is readily available to INDOT personnel at the office.

## **SECTION 14**

### **ADDENDA**

Each page in the Quality Control Plan that is revised shall have the source number, date of revision, and a vertical line in the left margin indicating the paragraph that was revised.

Revisions to the QCP shall be maintained on an Addenda Summary Sheet or QCP Annex in the Appendix until such time that the revisions are incorporated into the QCP. Addenda shall be submitted at the close-out meeting for an annual audit. Any outstanding revisions will also be submitted in January of each year.

## APPENDIX A

### AP PRODUCTION CONTROL PLAN

AP PRODUCTION CONTROL PLAN  
INDIANA QUALITY SAND & GRAVEL, INC.  
INDOT SOURCE #2000

Indiana Quality Sand and Gravel, Inc. is approved for Class AP aggregate, and produces and controls gravel meeting this classification in accordance with the following:

1. The production area for the AP gravel is the west side of Gravel Road. A diagram of the pit is included in Appendix B.
2. AP gravel will be processed in accordance with the procedures listed in Section 6.
3. The final production gradation for the AP gravel will be INDOT size #8 gradation.
4. The AP stockpile will be identified by a sign indicating AP No. 8, and the location and color of the sign will be in accordance with Section 11.
5. AP gravel delivered to concrete plants will be so identified on the aggregate weigh tickets.

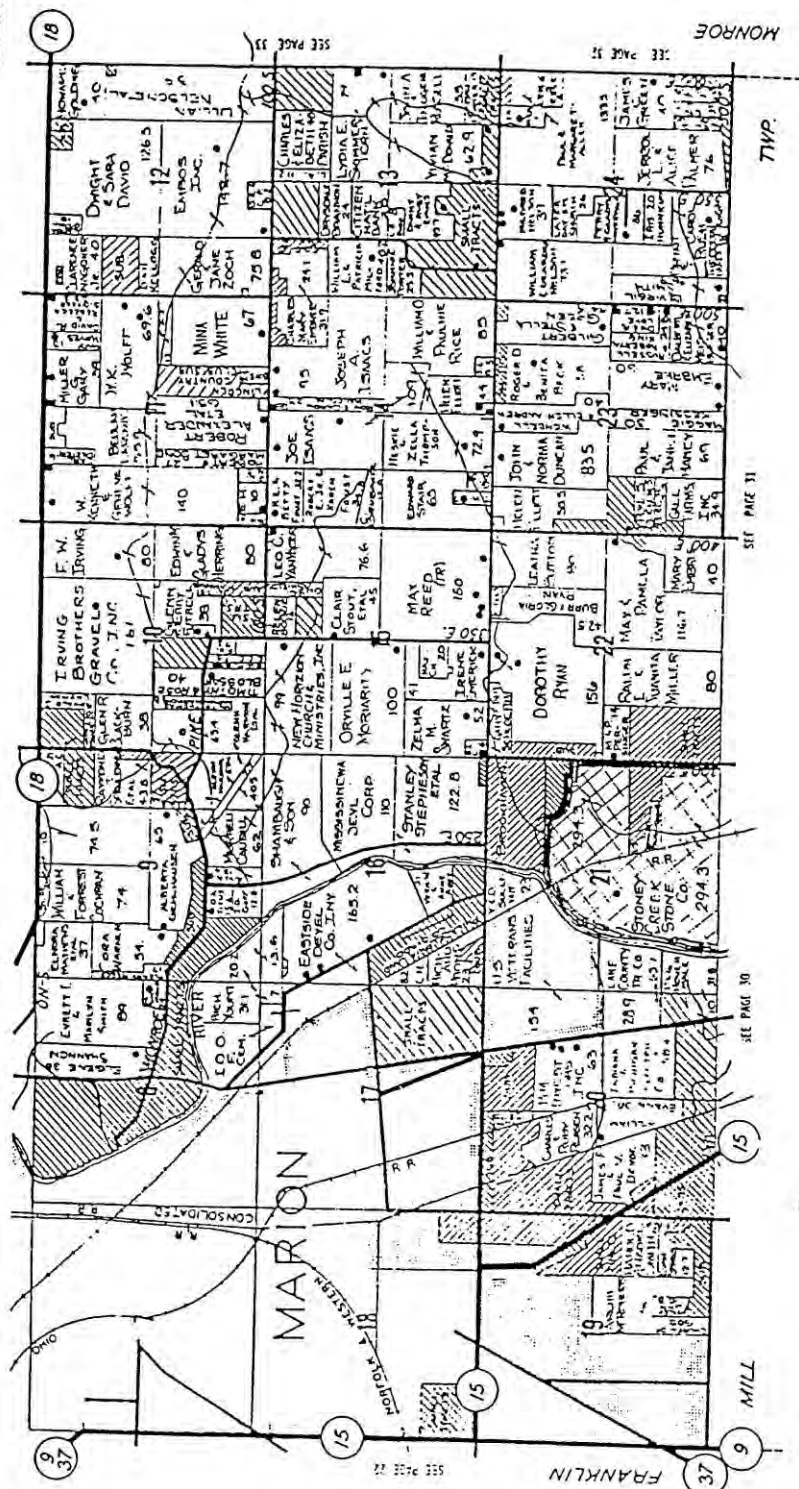
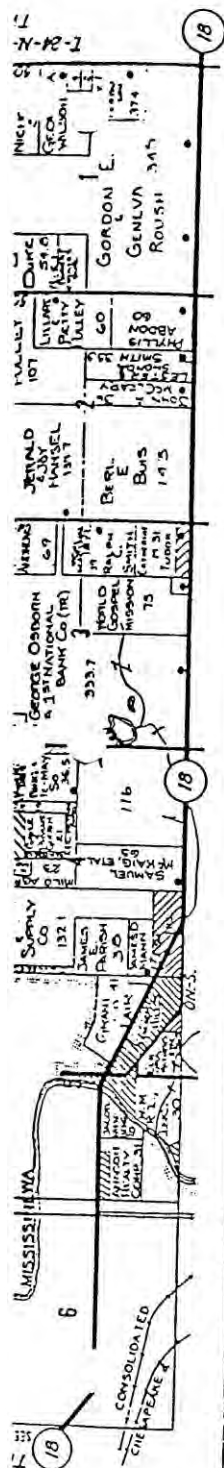
## APPENDIX B

### GENERAL INFORMATION



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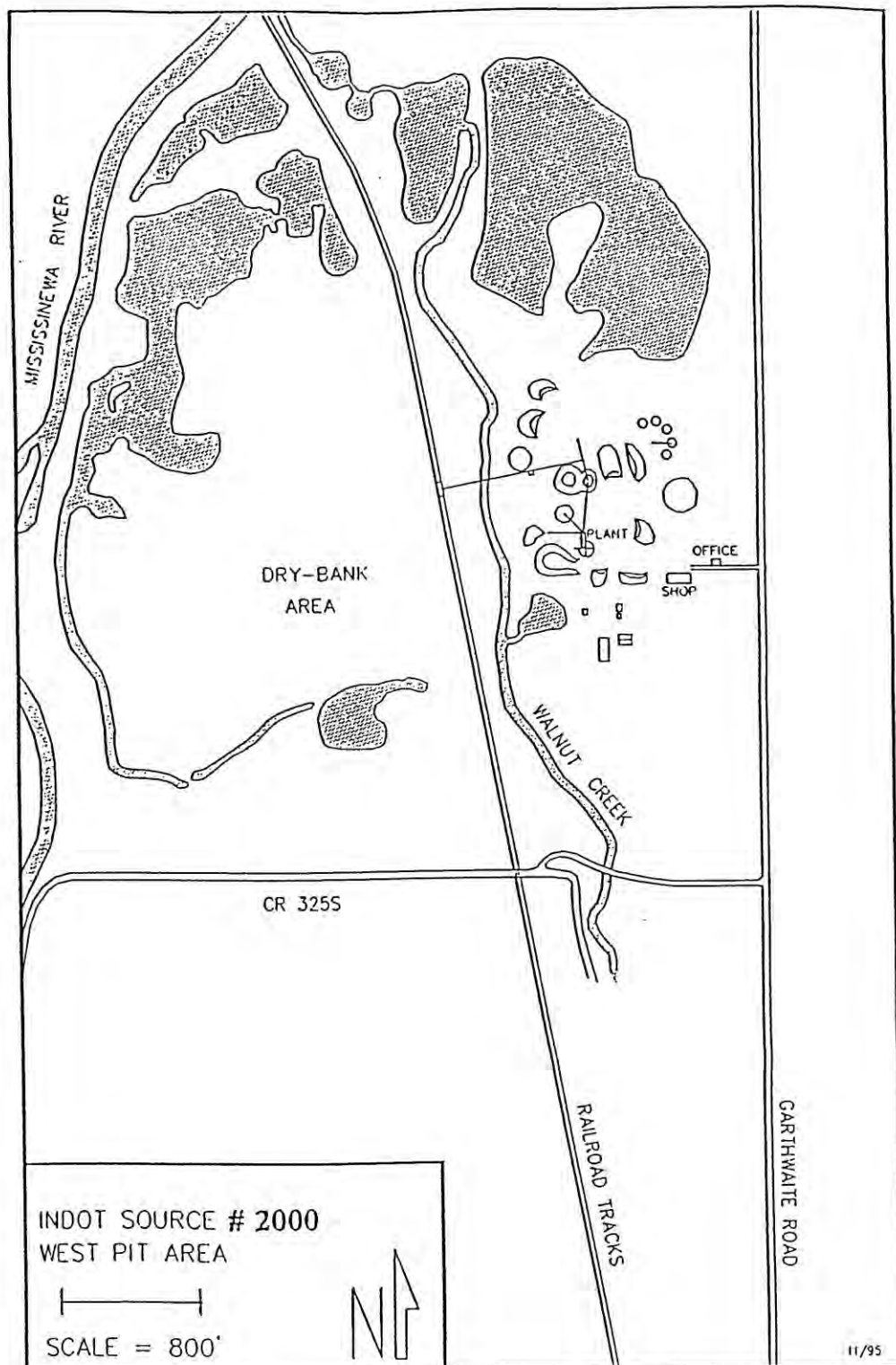
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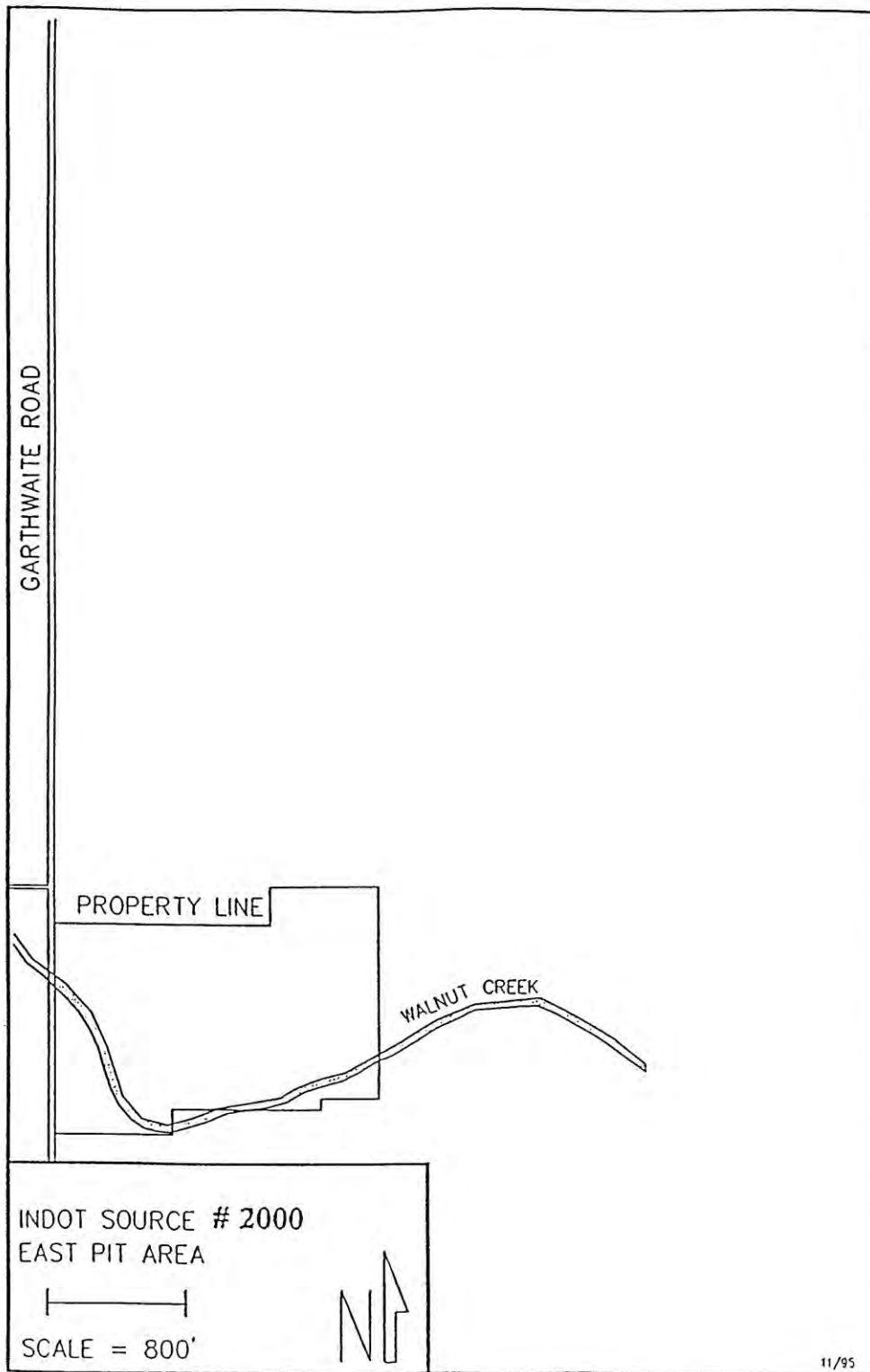


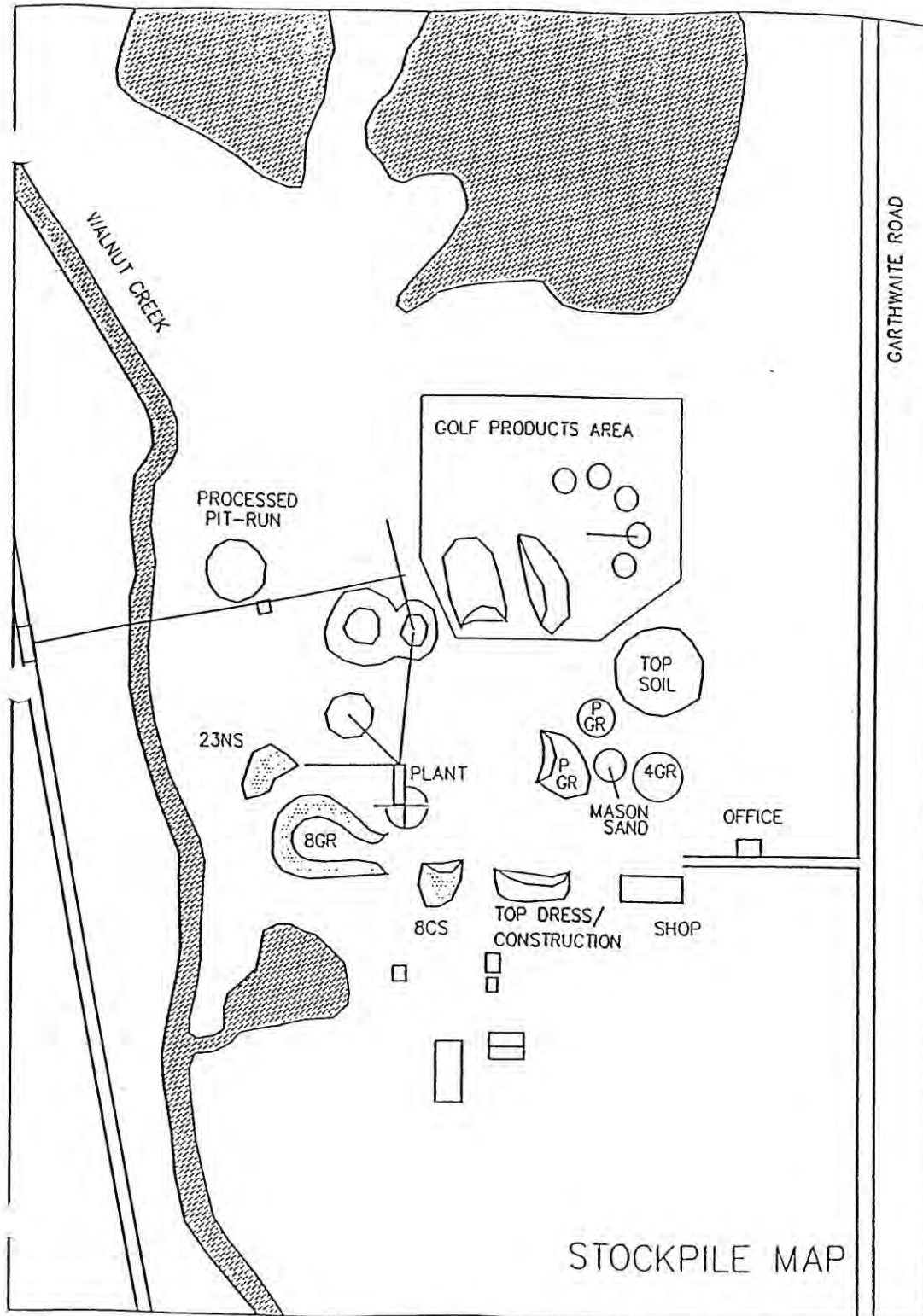
LOCATION MAP

GRANT COUNTY, INDIANA PAGE 1

56/11







## APPENDIX C

### FORMS



CAPP DIARY - ADDITIONAL REMARKS

SOURCE #:

DATE:

SUPERINTENDENT'S (OR REPRESENTATIVE) REMARKS - CONTINUED

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INITIALS

CAT'S REMARKS - CONTINUED

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(PRINTED NAME)

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(SIGNATURE)

08/95



GRADATION ANALYSIS FOR SIZE 8																
SOURCE _____										ORIGIN _____					MONTH / YEAR _____	
DATE															WHEN CONTROL CHARTING IS REQUIRED, PLACE A CIRCLE AROUND THE ENTRY IN THE * TEST TYPE * BOX ◁	
TEST#																
* TYPE																
SIEVE															AVE SPEC RANGE	
25.0 mm															100	
19.0 mm															75-95	
12.5 mm															40-70	
9.5 mm															20-50	
4.75 mm															0-15	
2.36 mm															0-10	
75 µm																
DECANT															0-1.5	
															* SAMPLE TYPES	
CHERT															S = START OF PRODUCTION FREQUENCY N = NORMAL PRODUCTION FREQUENCY L = LOADOUT FREQUENCY M = MISCELLANEOUS R = RESAMPLE	
SOFT																
LA																
% ABS																
BSGR																
CRUSH															STATISTICS	
															UCL = _____	
															LCL = _____	
5-POINT AVERAGE															$\bar{X}_r =$ _____	

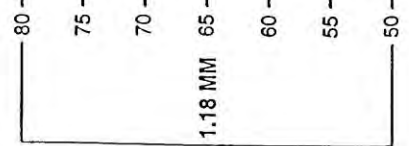
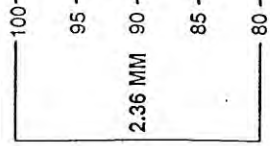
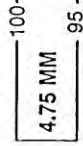
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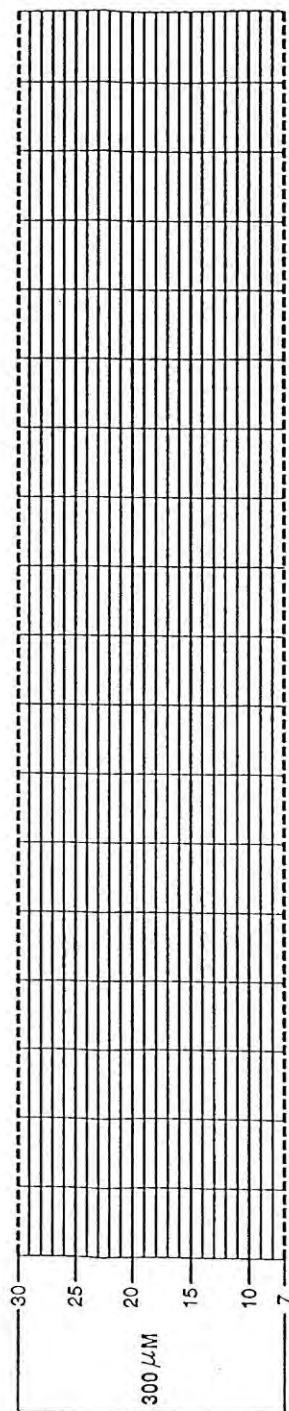
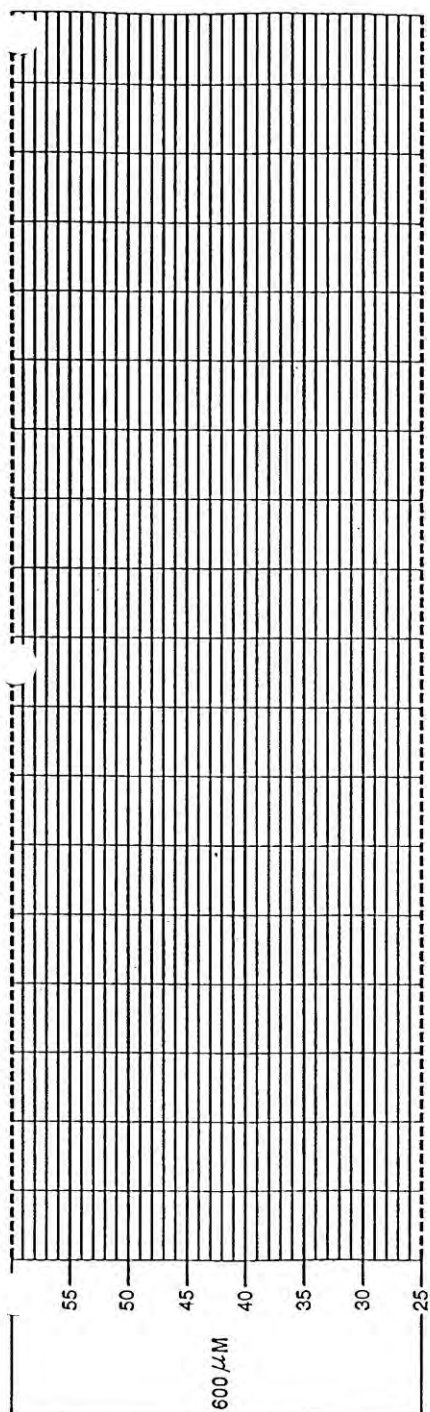
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## AUTHENTICATION

APPROVAL

SUBMISSION

\_\_\_\_\_  
Chief, Materials & Tests Division

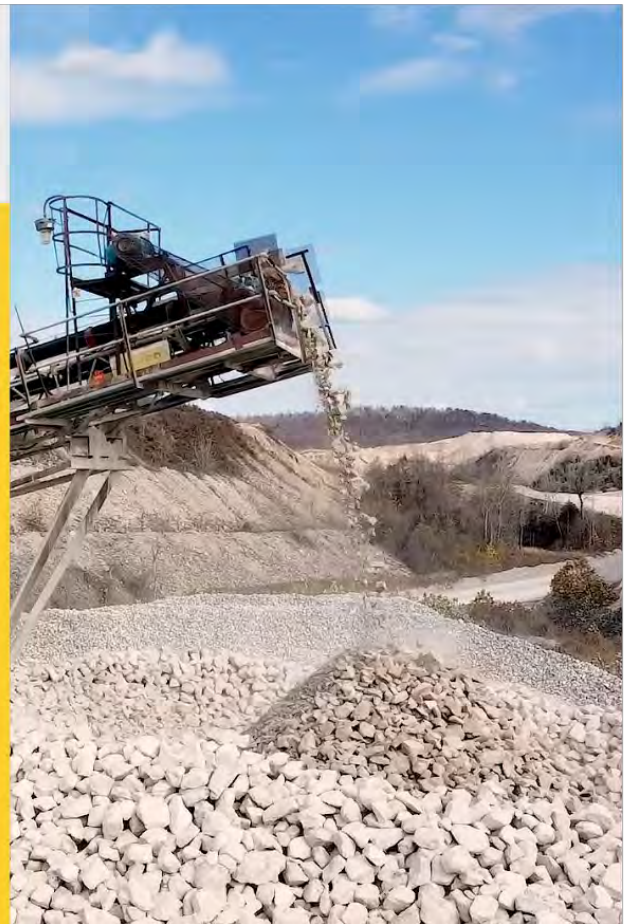
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Date of Submission

# AASHTO & ASTM DOCUMENTS

## CHAPTER 13



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# AASHTO & ASTM REFERENCE

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**AASHTO R 90**

**AASHTO T 11**

**AASHTO T 27**

**AASHTO T 84**

**AASHTO T 85**

**AASHTO T 112**

**AASHTO T 255**

**AASHTO R 76**

**AASHTO T 304**

**ASTM D 5821**



# **SAMPLING OF AGGREGATES**

## **AASHTO R 90**

### **AGGREGATE STREAMFLOW**

Before taking a sample, you must first assemble all the equipment you will need to obtain the sample. To obtain a sample using the aggregate streamflow, you will need the following:

1. Sampling device designed for use at each particular plant. This device consists of a pan of sufficient size to intercept the entire cross section of the discharge stream and retain the required quantity of material without overflowing. In some situations, a set of rails may be necessary to support the pan as it is passed through the streamflow.
2. Safety equipment such as hard hat, glasses, etc.
3. Sample containers, tags, etc.

### **Sampling Procedure**

Pass the sampling device through the streamflow, being sure to cut through the entire cross section of the material as the aggregate is being discharged (Figure 1). Care must be taken to pass the device through the stream rapidly enough to prevent any overflow of material during the sampling procedure. Obtain a minimum of three increments for each sample. Be sure to obtain equal increments. Obtain the appropriate weight to accommodate all tests to be performed on the sample. Allow an amount of time to elapse between passes to better get a representative sample of the material. When sampling aggregate from a loaded bin, increments should not be obtained when the belt first starts or when the bin is nearly empty to avoid the natural segregation that may occur as the material exists in the bin.



**Figure 1**  
**Streamflow Sampling**



## CONVEYOR BELT

The equipment to sample from a conveyor belt is somewhat different than that used for sampling from a streamflow. The following is the equipment needed to secure a proper sample from a conveyor belt:

1. A template constructed to conform to the shape of the loaded belt. An adjustable spacer between the two ends of the template is helpful to allow for adjustment of the device to the amount of aggregate on the belt.
2. A scoop or trowel to aid in removing the aggregate from the stopped belt.
3. A brush or broom to aid in removing the fine particles of the increment from the belt surface.
4. Sample containers, tags, etc.
5. Safety equipment such as hard hat, gloves, glasses, etc.

### Sampling Procedure

Insert the template into the aggregate on the stopped conveyor belt being sure the template passes through the aggregate and rests on the surface of the belt as close as practicable (Figure 2). Do not sample the portions of material first discharged on the belt or material discharged as the bin empties. These areas are normally segregated and the sample will not be representative. Using the small scoop or hand, remove as much of the aggregate from the belt as possible. Brush the remaining fines into the sample container. A dustpan may be useful in some applications to collect the fines. Obtain at least three increments for each field sample being sure to collect the minimum weight needed to perform all applicable tests. When practicable, allow the belt to run awhile between each increment. This will aid in obtaining a sample more representative of the material being tested.



**Figure 2**  
**Conveyor Belt Sampling**

There are automatic belt sampling devices that have the advantage of sampling the aggregate without stopping the belt (Figure 3). These devices sweep the belt with a small scoop and this increment of the belt is deposited into a sampling container. The number of sweeps of the belt is determined by the required size of the sample.



**Figure 3**  
**Automatic Sampling Device**

# **MATERIALS FINER THAN No. 200 (75 $\mu$ m) SIEVE IN MINERAL AGGREGATES BY WASHING**

## **AASHTO T 11**

### **SCOPE**

Aggregates are used in all phases of highway construction from bases, pavement mix, granular shoulders, and granular surfacing, as well as, erosion control. In order to ensure the aggregate performs as intended for the specific use, a variety of tests must be performed on the aggregate. One such test is determining materials finer than No. 200 (75  $\mu$ m) sieve in mineral aggregates by washing. Fine materials such as clay particles or water soluble particles removed by washing, can cling to larger particles and do not dislodge readily. This test washes the fine particles through the No. 200 (75  $\mu$ m) sieve to give an accurate determination of fine materials in the sample. The determination of minus No. 200 (75  $\mu$ m) material is used to compare material performance with gradation specifications, and indirectly to gauge such properties as plasticity, permeability, and soils classifications. Such knowledge helps in determining whether a material is frost susceptible or not, and whether permeability (measurement of material capacity to allow water flow through the aggregate) will be affected.

### **SUMMARY OF TEST**

A known amount of material is placed in a wash container and covered with water, agitated to suspend the fine size particles in the water, and then poured through a No. 200 (75  $\mu$ m) sieve (Figure 1). After thorough rinsing, the portion remaining on the No. 200 (75  $\mu$ m) sieve is transferred to a pan, dried and weighed. The percentage passing through the No. 200 (75  $\mu$ m) sieve is then calculated.



**Figure 1**

Fines suspended in the water are washed over a No. 8 (2.36 mm) and a No. 200 (75  $\mu$ m) sieve

## Apparatus

Balance, general purpose G<sub>2</sub> (AASHTO M231).

Sieves, a No. 8 (2.36 mm) or No. 16 (1.18 mm) and a No. 200 (75 µm).

Container, of sufficient size to properly agitate the sample without losing material.

*Mechanical Washing Device (Optional) - to aid in the washing process.*

Oven, capable of maintaining a temperature of  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ). When tests are performed in the field where ovens are not available, test samples may be dried in suitable containers over an open flame or electric hot plates with sufficient stirring to prevent overheating.

Wetting agent, dispersing material such as dish washing soap.

## Sample Preparation

Determine the proper dried sample weight from Table 1 based on the nominal maximum size of the sample to be tested. If the sample is to be tested for gradation in accordance with AASHTO T 27, the minimum weight of that test method shall apply. If the sample is not tested for gradation in accordance with AASHTO T 27 and the nominal maximum size of aggregate to be tested is not listed in Table 1, the next larger size shall be used to determine the sample size. *Reduce the sample to testing size according to AASHTO R76*

**Table 1-Sample Weight Requirements**

Nominal Maximum Size in. (mm)	Minimum Weight of Sample (gm)
No. 4 (4.75 mm)	300
3/8 in.(9.5 mm)	1000
3/4 in.(19.0 mm)	2500
1 1/2 (37.5 mm)	5000

## Procedure

1. Dry sample to a constant weight. Record this as the dry weight of the material to the nearest 0.1 g. Allow sample to air cool until cool to the touch.
2. Place sample into a wash container large enough to permit mixing the sample with water (Figure 2). Cover the sample with water (and optionally, at the discretion of the technician, add a small amount of wetting agent) and agitate the sample with sufficient movement so that the particles finer than the No. 200 (75  $\mu\text{m}$ ) sieve become suspended in the water. Stirring and agitating the sample may be necessary and may be accomplished with any stirring or agitating instrument. Care should be taken not to lose any portion of the sample or the fines suspended in the water.



**Figure 2**  
Washing Sample

3. Pour the water with the suspended fines through a No. 200 (75  $\mu\text{m}$ ) sieve (Figure 3). Occasionally inspect the No. 200 (75  $\mu\text{m}$ ) sieve for cracks along the seam or holes in the screen, as any imperfections will effect the final wash sieve results. Take care to pour only the water with suspended fines and not the sample itself, since samples with larger size aggregates might damage or clog the fine screen on the No. 200 (75  $\mu\text{m}$ ) sieve (Figure 3). Nesting sieves with larger openings a No. 8 (2.36 mm), or a No. 16 (1.18 mm) above the No. 200 (75  $\mu\text{m}$ ) sieve might help to prevent inadvertent clogging.



**Figure 3**  
Pouring Water through Sieves

4. Continue washing the sample with additional water and agitate until a majority of the fines suspended in the water have been washed through. When the washed sample is near completion, the water should be relatively clear compared with the initial water color of the wash sample. If you can see the sample beneath the water, then the sample is probably adequately washed. *If a Mechanical Washing Device is used limit the washing to a maximum of 10 minutes.*
5. Give the sample a final rinse, pouring as much of the remaining water as possible out of the sample and into the No. 200 (75  $\mu$ m) sieve. Put the sample remaining in the washing bowl into a pan for oven drying.
6. Any suspended fines remaining on the No. 200 (75  $\mu$ m) sieve must be included in the sample for drying. Rinsing any suspended fines to one side of the sieve (Figure 4) and then tapping those fines into the pan is one way of accomplishing this. Be sure to include all fines suspended on the No. 200 (75  $\mu$ m) sieve in the final sample for drying.



**Figure 4**  
Material Retained on the No. 200 (75 $\mu$ m) sieve

A rinsing bottle (Figure 5) may be used to remove the fines sticking to the No. 200 (75  $\mu$ m) sieve once the sample has been washed.



**Figure 5**  
Rinsing Fines on No. 200 (75 $\mu$ m) Sieve

7. Place the washed sample into an oven set at  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ), into an electric skillet, or onto an open flame and dry to a constant weight. Record the dry weight.

### Calculations

Calculate the total % passing the No. 200 (75  $\mu$ m) sieve (A) by dividing the difference of the original dry sample weight (B) and the weight of sample after washing and drying (C) to a constant weight by the original dry sample weight (B) and multiplying by 100.

$$A = \frac{(B - C)}{B} \times 100$$

Where: A = Total % passing No. 200 (75  $\mu$ m) sieve  
B = Original dry weight of sample (gms), and  
C = Dry weight of sample after washing and drying to constant weight (gms)

### Example

B = 532.2 gms

C = 521.6 gms

Formula:  $A = \frac{(B - C)}{B} \times 100$

$$A = \frac{(532.2 - 521.6)}{532.2} \times 100$$

A = 2.0%

Report the percentage of material finer than the No. 200 (75  $\mu$ m) sieve to the nearest 0.1%.

# **SIEVE ANALYSIS OF FINE AND COARSE AGGREGATES**

## **AASHTO T 27**

### **SCOPE**

The sieve analysis, commonly known as the gradation test, is a basic essential test for all aggregate technicians. The sieve analysis determines the gradation (the distribution of aggregate particles, by size, within a given sample) in order to determine compliance with design, production control requirements, and verification specifications. The gradation data may be used to calculate relationships between various aggregate or aggregate blends, to check compliance with such blends, and to predict trends during production by plotting gradation curves graphically, to name just a few uses. Used in conjunction with other tests, the sieve analysis is a very good quality control and quality acceptance tool.

NOTE: Accurate determination of material passing the No. 200 (75  $\mu\text{m}$ ) sieve cannot be made with this test alone. This test is recommended to be used in conjunction with AASHTO T 11 to determine the amount of material finer than the No. 200 (75  $\mu\text{m}$ ) sieve.

### **SUMMARY OF TEST**

A known weight of material, the amount being determined by the largest size of aggregate, is placed upon the top of a group of nested sieves (the top sieve has the largest screen openings and the screen opening sizes decrease with each sieve down to the bottom sieve which has the smallest opening size screen for the type of material specified) and shaken by mechanical means for a period of time. After shaking the material through the nested sieves, the material retained on each of the sieves is weighed.

The cumulative method requires that each sieve beginning at the top be placed in a previously weighed pan (known as the tare weight), weighed, the next sieve's contents added to the pan, and the total weighed. This is repeated until all sieves and the bottom pan have been added and weighed.

### **Apparatus**

Balance, general purpose class G<sub>2</sub> (AASHTO M231).

Sieves, mounted on suitable frames, designed not to leak. Sieves shall conform to ~~AASHTO M92~~  
**ASTM E11**

Mechanical sieve shaker, if used, must provide a vertical or lateral and vertical motion to the sieve, causing the particles thereon to bounce and turn so as to present different orientations to the sieving surface. Sieve shakers must provide sieving thoroughness within a reasonable time.



Oven, capable of maintaining  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ). When tests are performed in the field where ovens are not available, test samples may be dried in suitable containers over open flame or electric hot plates with sufficient stirring to prevent overheating.

### Sample Preparation

Samples should be obtained in the field and reduced to test size in accordance with **AASHTO R76**. Samples are dried to a constant weight in an oven set at  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ), in an electric skillet, or over an open flame.

The original sample must be reduced to a test sample size which falls within the minimum and maximum weight in the following table.

**WEIGHT OF TEST SAMPLE**

AGGREGATE SIZE	MINIMUM	MAXIMUM
No.1	68,000 g	90,700 g
No.2	11,300 g	---
No.5, No. 8, and No. 91	6000 g	8000 g
No. 9	4000 g	6000 g
No. 11	2000 g	---
No. 12	1000 g	--
No.53	6000 g	8000 g
No.73	6000 g	8000 g
Structure Backfill, 2 in.	4000 g	6 000 g
Structure Backfill, 1 ½ in. and 1 in.	6000 g	8000 g
Structure Backfill, ½ in.	4000 g	6000 g
Structure Backfill No. 4 (4.75 mm) & No. 30 (600 µm)	300 g	---
Fine Aggregate	300 g	---

### Procedure

1. Weigh the sample to the nearest 0.1 g by total weight of sample. This weight will be used to check for any loss of material after the sample has been graded. Select suitable sieve sizes in accordance with the specifications.
2. Nest the sieves in order of decreasing size from top to bottom and begin agitating and shaking the sample for a sufficient amount of time.

For coarse aggregate, the large tray shaker is most commonly used (Figure 1). This device provides a clamping mechanism which holds the sieve in place during agitation. Shakers of this make need to be run 5 minutes for size 9 or larger and 10 minutes for sizes smaller than size 9.

For fine aggregate, round 8" (203.2 mm) or 12" (304.8 mm) sieves are commonly used (Figure 2). These sieves are self-nesting and supported in a shaking mechanism at the top and bottom by a variety of clamping and/or holding mechanisms. Small shakers of this type require shaking times of 15 minutes to adequately grade the fine aggregate sample.



**Figure 1**  
Large Tray Shaker



**Figure 2**  
Small Sieve Shaker

NOTE: Every effort should be made to avoid overloading the sieves. AASHTO defines overloading large sieves as weight retained in excess of 2.5 times the sieve opening in in. (mm), as expressed in  $\text{gm/in.}^2$  ( $\text{kg/m}^2$ ). For fine aggregate, no weight shall be in excess of  $4 \text{ gm/in.}^2$  ( $7 \text{ kg/m}^2$ ).

### 3. Coarse Aggregates

After the material has been sieved, remove each tray, weigh each size, and record each weight to the nearest 0.1 g. Be sure to remove any aggregate trapped within the sieve openings by gently working from either or both sides with a trowel or piece of flat metal until the aggregate is freed. Banging the sieve on the floor or hitting the sieve with a hammer will damage the sieve. The final total of the weights retained on each sieve should be within 0.3% of the original dry sample weight prior to grading. Particles larger than 3 in. (75 mm) should be hand-sieved. When passing large stones through sieves, do not force the aggregate through the sieve openings.

4. Fine Aggregates

Weigh the material retained on each sieve size to the nearest 0.1 g. Ensure that all material entrapped within the openings of the sieve are cleaned out and included in the weight retained. This may be done using brushes to gently dislodge entrapped materials. The 8 in. (203 mm) or 12 in. (304.8 mm) round sieves need to be handled with special care due to the delicate nature of their screen sizes. As a general rule, use coarse wire brushes to clean the sieves down through the No. 50 (300  $\mu$ m) sieve (Figure 3). Any sieve with an opening size smaller than the No. 50 (300  $\mu$ m) should be cleaned with a softer cloth hair brush (Figure 4). The final total of the weights retained on each sieve should be within 0.3% of the original dry sample weight prior to grading.



**Figure 3**  
Use Wire brush on Coarse Sieve



**Figure 4**  
Use Hair Brush on Fine Sieves

# **SPECIFIC GRAVITY AND ABSORPTION OF FINE AGGREGATES**

## **AASHTO T 84**

### **GLOSSARY**

**Absorption:** The increase in mass due to water in the pores of the material.

**Bulk Specific Gravity (also known as Bulk Dry Specific Gravity):** The ratio of the weight in air of a unit volume of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**Bulk SSD Specific Gravity:** The ratio of the weight in air of a unit volume of aggregate, including the weight of water within the voids filled to the extent achieved by submerging in water for approximately 15 hours, to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**Apparent Specific Gravity:** The ratio of the weight in air of a unit volume of the impermeable portion of aggregate (does not include the permeable pores in aggregate) to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**SSD - Saturated, Surface Dry.** The condition in which the aggregate has been soaked in water and has absorbed water into its pore spaces. The excess, free surface moisture has been removed so that the particles are still saturated, but the surface of the particle is essentially dry.

### **SCOPE**

Specific Gravity is the ratio of the weight of a given volume of aggregate to the weight of an equal volume of water. Water, at a temperature of 73.4°F (23°C) has a specific gravity of 1. Specific Gravity is important for several reasons. Some deleterious particles are lighter than the "good" aggregates. Tracking specific gravity can sometimes indicate a change of material or possible contamination. Differences in specific gravity may be used to separate the deleterious particles from the good using a heavy media liquid.

Specific gravity is critical information for the Hot Mix Asphalt Design Engineer. This value is used in calculating air voids, voids in mineral aggregate (VMA), and voids filled by asphalt (VFA). All are critical to a well performing and durable asphalt mix. Water absorption may also be an indicator of asphalt absorption. A highly absorptive aggregate may result in a low durability asphalt mix.

In Portland Cement Concrete the specific gravity of the aggregate is used in calculating the percentage of voids and the solid volume of aggregates in computations of yield. The absorption is important in determining the net water-cement ratio in the concrete mix. Knowing the specific gravity of aggregates is also critical to the construction of water filtration systems, slope stabilization projects, railway bedding and many other applications.

This test method determines the specific gravity of fine aggregates that have been soaked for a period of 15-19 hrs. The determinations that may be made from this procedure are identical to those made in AASHTO T 85 (Specific Gravity and Absorption of Coarse Aggregate).

## SUMMARY OF TEST

### Apparatus

Balance, conforming to the requirements of M 231, Class G2

Pycnometer, a flask or other suitable container into which the fine aggregates may be readily introduced (Figure 1). Volume content for the container needs to be reproduced within  $\pm 100 \text{ mm}^3$ . The volume of the container filled to the mark shall be at least 50 percent greater than the space required to accommodate the test sample.

Mold, metal in the form of a frustum of cone with acceptable dimensions of  $40 \pm 3 \text{ mm}$  inside diameter at top,  $90 \pm 3 \text{ mm}$  inside diameter at the bottom, and  $75 \pm 3 \text{ mm}$  in height. The metal thickness is a minimum of 0.8 mm.

Tamper, metal having a mass of  $340 \pm 15\text{g}$  and having a flat circular tamping face of  $25 \pm 3 \text{ mm}$  in diameter.



**Figure 1**  
Fine Aggregate Specific Gravity Apparatus

### Procedure

1. Thoroughly mix the sample and reduce the sample to the required size in accordance with AASHTO **R76** (Reducing Field samples of Aggregates to Test Size). The sample size for this procedure is approximately 1000g of material passing the No. 4 (4.75 mm) sieve.
2. Dry test samples to constant weight in an oven set at  $230 \pm 9^\circ\text{F}$  ( $110 \pm 5^\circ\text{C}$ ). Cool the sample at room temperature for 1 to 3 hours. After the cooling period, immerse the sand in water at room temperature for a period of 15 to 19 hours.

Instead of completely immersing the sand in water, AASHTO considers sand to be "soaked" if the sand is maintained at a moisture content of at least 6% for the prescribed period. This is the recommended procedure to eliminate the need to decant excess water from the sand prior to testing. The decantation process is time consuming and difficult, since great care must be taken to avoid decanting some of the sample along with the water. Additionally, the sand will be much closer to the SSD condition when soaked at 6% moisture, which expedites the dry procedure.

3. Decant water from the sample, avoiding loss of fines. Spread the sample on a flat, non-absorbent surface. Stir the sample occasionally to assist in homogeneous drying. A current of warm air may be used to assist drying procedures (Figure 2); however, fine particles may be lost with this procedure if not careful.



**Figure 2**  
A current of air being used  
to achieve SSD condition.

4. Determine the SSD condition of the sand using the Cone Test.

Note: Throughout the process of drying in Step 3, test the sand for SSD condition using the cone method. Place the cone with the large diameter down on a glass plate. Fill cone to overflowing with drying sand. Lightly tamp the fine aggregate into the mold with 25 light drops of the tamper (Figure 3). Each drop should start about 1/5 in. above the top surface of the fine aggregate. Remove loose sand from base and carefully lift the mold vertically. If surface moisture is still present, the fine aggregate will retain the molded shape. When the sand achieves an SSD condition, the sand will slump (Figure 4).



**Figure 3**  
Tamping sand using the cone  
method to determine SSD

If on the first trial the sand slumps, moisture must be re-added and the drying process repeated. Record the weight of the sand as SSD mass when the sand slumps to the nearest 0.1 g.



**Figure 4**  
Sand at SSD condition will slump  
once the cone is removed

5. Calibrate a specific gravity flask pycnometer by filling with water at  $73.4 \pm 3^{\circ}\text{F}$  ( $23 \pm 1.7^{\circ}\text{C}$ ) to the calibration line. Record this weight as the weight of the pycnometer filled with water to the nearest 0.1 g.
6. Place the SSD sand into the pycnometer (Figure 5) and fill with water (set at  $73.4 \pm 3^{\circ}\text{F}$  ( $23 \pm 1.7^{\circ}\text{C}$ )) to 90% of pycnometer capacity.



**Figure 5**  
Pouring sand into pycnometer  
once SSD is achieved

Manually roll, invert, and agitate the pycnometer to eliminate air bubbles (Figure 6). This procedure should be repeated several times to ensure that any entrapped air is eliminated. Agitation of the pycnometer does not have to be constant.



**Figure 6**  
Agitating the pycnometer



7. Bring the pycnometer to the pycnometer calibrated capacity with additional water (Figure 7).



**Figure 7**  
Adding Water to Calibrated Capacity

If bubbles prevent the proper filling of the pycnometer, adding a few drops of isopropyl alcohol is recommended to disperse the foam. Place the pycnometer in a water bath at the regulated temperature and allow the sample to equalize.

8. Determine the total weight of pycnometer, specimen, and water. Record the weight to the nearest 0.1g as Weight of Pycnometer with sample and water.

### Calculations

Determine calculations based on appropriate formula for desired result as follows:

- A. Bulk Specific Gravity (Gsb): The ratio of the weight in air of a unit volume of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at a stated temperature.

$$Gsb = A / (B - C)$$

Where: A = Oven dry wt.    B = SSD wt.    C = Wt. in water

- B. Bulk SSD Specific Gravity (Gsb SSD): The ratio of the weight in air of a unit volume of aggregate, INCLUDING the weight of water within the voids filled to the extent achieved by submerging in water for approximately 15 hours, to the weight in air of an equal volume of gas-free distilled water at a stated temperature.

$$Gsb \text{ SSD} = B / (B - C)$$

- C. Apparent Specific Gravity (G<sub>sa</sub>): The ratio of the weight in air of a unit volume of the IMPERMEABLE portion of aggregate (does not include the permeable pores in aggregate) to the weight in air of an equal volume of gas-free distilled water at a stated temperature.

$$G_{sa} = A / (A - C)$$

- D. Absorption (% Abs): The increase in weight of aggregate due to water in the pores of the material, but not including water adhering to the outside surface of the particles.

$$\% \text{ Abs} = [(B - A) / A] \times 100$$

### Example

Trial	Wet Weight	Dry Weight	Wet - Dry	% Absorbed
1	118.11	117.42	0.69	0.59
2	158.10	157.13	0.97	0.62
3	172.81	171.12	1.09	0.64

Trial	S	A	B	C	B + S - C	B + A - C
1	500.05	497.1	670.7	983.8	186.9	184.0
2	499.77	496.7	679.6	992.4	187.0	183.9
3	499.61	496.5	671.6	984.1	187.1	184.0

Trial	Bulk SSD S/B+S-C	Bulk A/B+S-C	APPARENT A/B+A-C
1	2.675	2.660	2.702
2	2.673	2.656	2.701
3	2.670	2.654	2.698
Average	2.673	2.657	2.700

A = Weight of Oven Dry Specimen in Air

B = Weight of Pycnometer filled with water

C = Weight of Pycnometer with specimen and water to calibration mark

S = SSD Weight

# **SPECIFIC GRAVITY OF COARSE AGGREGATE**

## **AASHTO T 85**

### **GLOSSARY**

**Absorption:** The increase in weight due to water contained in the pores of the material.

**Bulk Specific Gravity (also known as Bulk Dry Specific Gravity):** The ratio of the weight in air of a unit volume of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**Bulk SSD Specific Gravity:** The ratio of the weight in air of a unit volume of aggregate, including the weight of water within the voids filled to the extent achieved by submerging in water for approximately 15 hours, to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**Apparent Specific Gravity:** The ratio of the weight in air of a unit volume of the impermeable portion of aggregate (does not include the permeable pores in aggregate) to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**SSD - Saturated, Surface Dry.** The condition in which the aggregate has been soaked in water and has absorbed water into its pore spaces. The excess, free surface moisture has been removed so that the particles are still saturated, but the surface of the particle is essentially dry.

### **SCOPE**

Specific Gravity is the ratio of the weight of a given volume of aggregate to the weight of an equal volume of water. Water, at a temperature of 73.4°F (23°C) has a specific gravity of 1. Specific Gravity is important for several reasons. Some deleterious particles are lighter than the good aggregates. Tracking specific gravity can sometimes indicate a change of material or possible contamination. Differences in specific gravity may be used during production to separate the deleterious particles from the good using a heavy media liquid.

Specific gravity is critical information for the Hot Mix Asphalt Design Engineer. The value is used in calculating air voids, voids in mineral aggregate (VMA), and voids filled by asphalt (VFA). All are critical to a well performing and durable asphalt mix. Water absorption can also be an indicator of asphalt absorption. A highly absorptive aggregate may lead to a low durability asphalt mix.

In Portland Cement Concrete the specific gravity of the aggregate is used in calculating the percentage of voids and the solid volume of aggregates in computations of yield. The absorption is important in determining the net water-cement ratio in the concrete mix. Knowing the specific gravity of aggregates is also critical to the construction of water filtration systems, slope stabilization projects, railway bedding and many other applications.

This test method determines the specific gravity of coarse aggregates that have been soaked for a period of 15 hours (Figure 1). There are four determinations that may be made from this procedure. They are as follows:



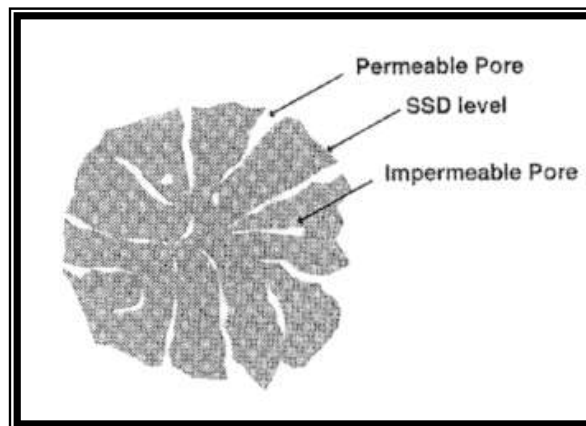
**Figure 1**  
Coarse Aggregate Gravity Apparatus

A. Bulk Specific Gravity (Gsb) (also known as Bulk Dry Specific Gravity)

The ratio of the weight in air of a unit volume of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at a stated temperature (Figure 2). This unit volume of aggregates is composed of the solid particle, permeable voids, and impermeable voids.

$$G_{sb} = A / (B - C)$$

Where:      A = Oven dry weight.  
              B = SSD weight.  
              C = Weight in water.



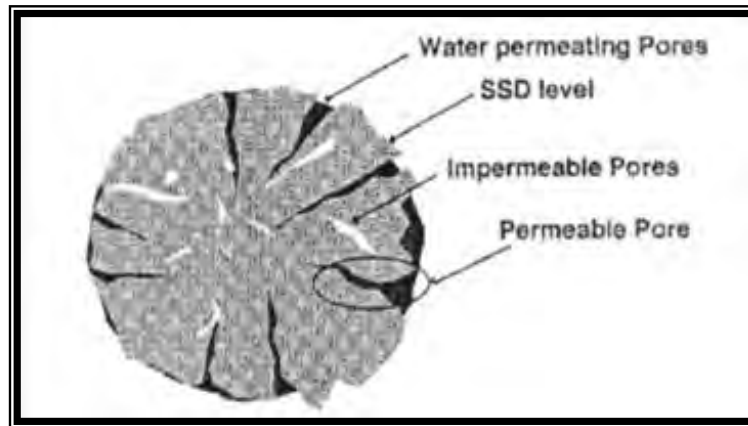
**Figure 2**  
Diagram of Bulk Specific Gravity

B. Bulk SSD Specific Gravity (Gsb SSD)

The ratio of the weight in air of a unit volume of aggregate, INCLUDING the weight of water within the voids filled to the extent achieved by submerging in water for approximately 15 hours, to the weight in air of an equal volume of gas-free distilled water at a stated temperature (Figure 3).

$$G_{sb\ SSD} = B / (B - C)$$

Where:      B = SSD weight.  
              C = Weight in water.



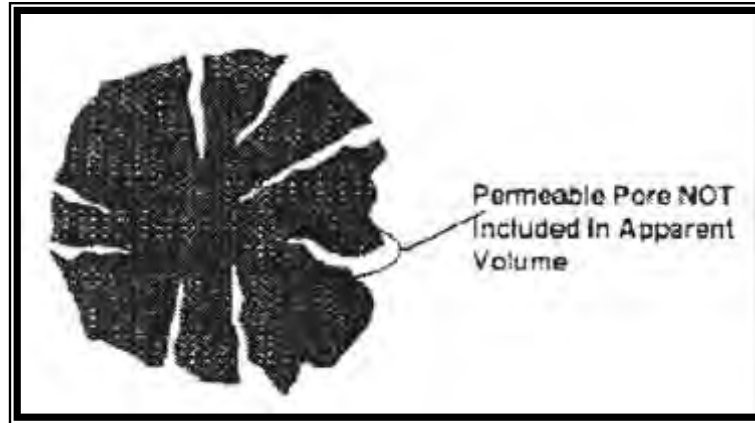
**Figure 3**  
Diagram of Bulk SSD Specific Gravity

C. Apparent Specific Gravity (Gsa)

This ratio of the weight in air of a unit volume of the IMPERMEABLE portion of aggregate (does not include the permeable pores in aggregate) to the weight in air of an equal volume of gas-free distilled water at a stated temperature (Figure 4).

$$G_{sa} = A / (A - C)$$

Where:      A = Oven dry weight.  
              C = Weight in water



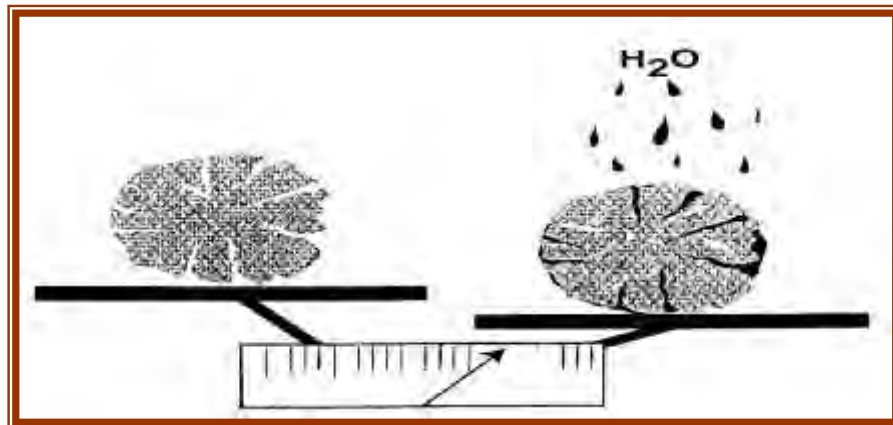
**Figure 4**  
Diagram of Apparent Specific Gravity

D. Absorption (% Abs.)

The increase in weight of aggregate due to water in the pores of the material, but not including water adhering to the outside surface of the particles (Figure 5).

$$\% \text{ Abs.} = [ (B - A) / A ] \times 10$$

Where:      A = Oven dry weight.  
              B = SSD weight.



**Figure 5**  
Increase in Mass due to Absorption of Water

## SUMMARY OF TEST

### Apparatus

Balance, conforming with class G5 (AASHTO M231)

Note: Not an official AASHTO Document

Sample container, wire basket of No. 6 (3.35 mm) or less mesh wire cloth, with a capacity of 1 to 1 3/4 gal. (4 to 7 L) to contain aggregate with a nominal maximum size of 1 1/2 in. (37.5 mm) or smaller; larger basket for larger aggregates.

Water tank, watertight and large enough to completely immerse aggregate and basket, equipped with an overflow valve to keep water at a constant level.

Suspended Apparatus, wire used to suspend apparatus with the smallest practical diameter. A hi-test fishing leader or other thin wire with utility hook can be used with a small hook attached to the handle of the basket or sample container.

Sieves, No. 4 (4.75 mm) or other size as needed, conforming to **ASTM E11**.

## Procedure

1. Thoroughly mix the sample and reduce the sample to the required size (Figure 6) in accordance with AASHTO T248 (Reducing Field Samples of Aggregate to Test Size). Use sample sizes as indicated in Table 1.



**Figure 6**  
Reducing Sample to Test Size

TABLE 1

Nominal Maximum Size	Minimum Sample Weight
1/2 in. (12.5 mm)	4.4 lbm (2 kg)
3/4 in. (19 mm)	6.6 lbm (3 kg)
1 in. (25 mm)	8.8 lbm (4 kg)
1 1/2 in. (37.5 mm)	11 lbm (5 kg)
2 in. (50 mm)	18 lbm (8 kg)
2 1/2 in. (63 mm)	<b>25</b> lbm (12 kg)
3 in. (75 mm)	40 lbm (18 kg)



2. Dry sieve the sample through a No. 4 (4.75 mm) sieve and discard any material that passes the sieve (if a substantial amount of material passes the No. 4 (4.75 mm) sieve, you may need to use a No. 8 (1.18 mm) sieve instead of the No. 4 (4.75 mm), or you may need to perform a specific gravity on the minus No. 4 (4.75 mm) material). Wash the aggregate retained on the No. 4 (4.75 mm) sieve.
3. Dry test sample to constant weight in an oven regulated at  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ). Cool sample at room temperature for 1 to 3 hr. After the cooling period, immerse the aggregate in water at room temperature for a period of 15 hr.
4. Place entire sample in a container and weigh in water maintained at  $73.4 \pm 3^{\circ}\text{F}$  ( $23 \pm 1.7^{\circ}\text{C}$ ). Shake container to release any entrapped air and weigh on minimum diameter wire suspended below scale apparatus. The water level in the bath should be maintained at the overflow depth to obtain a constant water level throughout the test (Figure 7). Record to the nearest 1.0 g or 0.1% of total weight, whichever is greater, as the weight in Water (C).



**Figure 7**  
Water Overflow Outlet

5. Remove the sample from the container and drain any excess water from the aggregate. Using an absorbent cloth (an absorbent towel usually works best), roll the aggregate until the surface water has been removed (Figure 8). Rolling up the aggregate into the towel and then shaking and rolling the aggregate from side to side is also an effective procedure in reducing the sample to an SSD (saturated, surface-dry) condition.



**Figure 8**  
Removing Excess Water

An SSD condition is one in which the aggregate has no FREE water on the surface of the aggregate. If the test sample dries past the SSD condition, immerse the sample in water for 30 minutes and resume the process of surface-drying.

6. Weigh SSD sample to nearest 1.0 g or 0.1% of the total weight, whichever is greater and record this as SSD weight.
7. Dry the sample in a pan to a constant weight in an oven set at  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ). Cool in air at room temperature for 1 to 3 hr, or until the aggregate can be comfortably handled. Record weight to nearest 1.0 g or 0.1%, whichever is greater, as oven dry weight.

### Calculations

Determine calculations based on appropriate formula.

A = Oven dry weight

B = SSD weight.

C = Weight in water

Bulk Specific Gravity (Gsb)

$$\text{Gsb} = A / (B - C)$$

Bulk SSD Specific Gravity (Gsb SSD)

$$\text{Gsb SSD} = B / (B - C)$$

Apparent Specific Gravity (Gsa)

$$\text{Gsa} = A / (A - C)$$

Absorption (% Abs)

$$\% \text{ Abs.} = [(B - A) / A] \times 100$$

Example

<b>Trial</b>	<b>A</b>	<b>B</b>	<b>C</b>	<b>B-C</b>	<b>A-C</b>	<b>B-A</b>
<b>1</b>	<b>2030.9</b>	<b>2044.9</b>	<b>1304.3</b>	<b>740.6</b>	<b>726.6</b>	<b>14.0</b>
<b>2</b>	<b>1820.0</b>	<b>1832.5</b>	<b>1168.1</b>	<b>664.4</b>	<b>651.9</b>	<b>12.5</b>
<b>3</b>	<b>2035.2</b>	<b>2049.4</b>	<b>1303.9</b>	<b>745.5</b>	<b>731.3</b>	<b>14.2</b>

<b>Trial</b>	<b>Bulk SSD B/B-C</b>	<b>Bulk A/B-C</b>	<b>Apparent A/A-C</b>	<b>Abs. (B-A/A)100</b>
<b>1</b>	<b>2.761</b>	<b>2.742</b>	<b>2.795</b>	<b>0.691</b>
<b>2</b>	<b>2.758</b>	<b>2.739</b>	<b>2.792</b>	<b>0.698</b>
<b>3</b>	<b>2.749</b>	<b>2.730</b>	<b>2.783</b>	<b>0.698</b>
<b>Ave.</b>	<b>2.756</b>	<b>2.737</b>	<b>2.790</b>	<b>0.693</b>

A = Weight of Oven Dry Specimen in Air

B = Weight of SSD Specimen in Air

C = Weight of SSD Specimen in Water

These calculations demonstrate the relationship between Gsb, Gsb SSD, and Gsa. The Gsb (bulk specific gravity) will always be the lowest value since the volume calculated includes voids permeable to water. The Gsb SSD (bulk specific gravity at SSD) will always be the intermediate value, and the Gsa (apparent specific gravity) will always be the highest, since the volume calculated includes only the "solid" aggregate particle (does not include those voids permeable to water). When conducting this test, check to make sure the values calculated make sense in relation to one another.

# **CLAY LUMPS AND FRIABLE PARTICLES IN AGGREGATE**

## **AASHTO T 112**

### **SCOPE**

To ensure the aggregate used performs as intended for highway construction, several tests are performed to determine the physical characteristics of the material. One of these tests is the determination of Clay Lumps and Friable Particles in Aggregate.

Excessive clay lumps in a processed aggregate intended for use in a Portland Cement or Hot Mix Asphalt may interfere with the bonding between the aggregate and cementitious material. This will result in spalling, raveling, or stripping and create weak points and pop-outs if the material is incorporated into the pavement or structure.

Aggregate intended to perform as a drainable base or subbase may also be adversely affected when excess amounts of clay and friable particles are present. This type of material tends to fill the void spaces intended for drainability, eventually contributing to pavement failure.

Attaining a reasonably accurate determination of the amount of clay lumps and friable particles in the processed aggregate is dependent on properly obtained representative samples.

### **SUMMARY OF TEST**

There are two test methods for determining the clay lumps and friable particles in aggregates. There is a method for coarse aggregate and one for fine aggregate. The test methods are similar, but there are differences, so always be sure to follow the correct method for the type of aggregate being tested.

The material is sampled, dried, and soaked according to testing instructions. The clay lumps and friable particles are broken down by manipulation, using the thumb and forefinger. The material is washed, dried, and sieved according to the correct test procedure.

The materials are weighed and the calculations for the percent of clay lumps and friable particles are performed.

### **FINE AGGREGATE**

#### **Apparatus**

Balance, sufficient capacity to determine the weight of the test samples, accurate to 0.1 percent of the weight of the sample to be tested, and conforms to the requirements of AASHTO M 231.

Containers, rust-resistant of a size and shape that will permit the spreading of the sample on the bottom in a thin layer.

Sieves, conforming to **ASTM E11**.

Oven, capable of providing free circulation of air and of maintaining a temperature of  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ).

### Sample Preparation

First subject the test sample to AASHTO T 11, Amount of Material Finer Than the No. 200 (75  $\mu\text{m}$ ) Sieve in Aggregate.

The sample shall be dried to a constant dry weight at a temperature of  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ).

Remove the material smaller than No. 16 (1.18 mm) sieve by thoroughly sieving the original sample over the No. 16 (1.18 mm) sieve. The weight retained on this sieve is the test sample and must be at least 25 grams.

### Procedure

1. Weigh the test sample and spread the sample in a thin layer on the bottom of an appropriately sized, rust-resistant container, cover the sample with **potable** water and allow the sample to soak for a period of  $24 \pm 4$  hours.
2. Decant the excess water from the sample after soaking. Roll and squeeze individual particles between the thumb and forefinger to attempt to break the particle into smaller pieces. Do not use fingernails, nor press the particles against hard surfaces or each other in the attempt to break the particles.
3. After all discernable clay lumps and friable particles have been broken, sieve the sample on a No. 20 (850  $\mu\text{m}$ ) sieve and then place the sample in a suitable drying pan. Dry the sample to a constant dry weight at a temperature of  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ). Allow the sample to cool and weigh the sample to the required accuracy specified for the balance in AASHTO M 231.

### Calculations

Calculate the percent of clay lumps and friable particles in fine aggregate using the following formula:

$$P = \frac{M-R}{M} \times 100$$

Where:

P = percent of clay lumps and friable particles

M = weight of test sample retained on the No. 16 (1.18 mm) sieve

R = weight of material retained on the No. 20 (850  $\mu\text{m}$ ) sieve

## COARSE AGGREGATES

### Apparatus

The same apparatus is used for the coarse aggregate test method that is used for the fine aggregate test method.

### Sample Preparation

Subject the sample to be tested to AASHTO T 11, Amount of Material Finer Than No. 200 (75  $\mu$ m) Sieve.

The aggregate sample shall be dried to a constant weight at a temperature of  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ).

Separate the coarse aggregate sample into individual fractions using the following sieves to obtain the minimum weights as shown in Table 1:

Table 1

Sizes of Particles Making Up Test Sample	Min. Weight of Individual Test Sample, Grams
Under No. 4 (Under 4.75 mm)	300 to 325
No. 4 to 1.2 in. (4.75 to 12.5 mm)	750 to 800
1/2 to 1 1/2 in. (12.5 to 37.5 mm)	1500 to 1600
Over 1 1/2 in. (37.5 mm)	3000 to 3100

Note: To provide the minimum required individual weight as indicated in Table 1, combining the material from more than one test by AASHTO T 11 may be necessary. If the original grading of the sample has less than 10% of material retained on any of the above individual sizes, do not test that size.

## Procedure

1. Weigh each fraction size and spread the individual samples in rust-resistant pans to form a thin layer. Cover the samples with **potable** water and soak for  $24 \pm 4$  hours (Figure 1).



Figure 1  
Sample Soaking

2. After soaking, decant the excess water from the samples. Roll and squeeze suspect particles between the thumb and forefinger to attempt to break the particles into smaller sizes (Figure 2). Do not use fingernails to break the particles, or press the particles against a hard surface or each other.



Figure 2  
Attempting to Break Particles

3. After all recognizable clay lumps and friable particles have been broken, remove the undersized material from each tested fraction by wet-sieving. The wet-sieving is to be accomplished by placing the sample on the appropriate size sieve for the size of the individual fraction (Table 2) and passing water over the sample while manually agitating the sieve, until all undersize material has passed the required sieve.

Table 2

Size of Particles Making Up the Sample	Sieve Size for Removing Residue of Clay Lumps and Friable Particles
Under No. 4 (Under 4.75 mm)	No. 20 (850-um)
No. 4 to 1/2 in. (4.75 mm to 12.5 mm)	No. 8 (2.36 mm)
1/2 to 1 in. (12.5 mm to 25.0 mm)	No. 4 (4.75 mm)
1 to 1 1/2 in. (25.0 mm to 37.5 mm)	3/8 in (9.5 mm)
Over 1 1/2 in (Over 37.5 mm)	3/8 in (9.5 mm)

The material that can be broken down and removed from the sample by wet-sieving is classified as clay lumps and friable particles.

4. Remove the retained particles carefully from the sieve. Dry the sample to a constant dry weight at  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ), and allow the material to cool.
5. Weigh the record the weight of the material to the accuracy specified for the balance in AASHTO M 231.

Note: Combined aggregates (those containing a substantial amount of coarse and fine material) are separated into two fractions using the No. 4 (4.75 mm) sieve and then prepared as appropriate for the correct size of the material (i.e., coarse or fine aggregate).

In most cases, only the plus No. 4 (4.75 mm) fraction of coarse aggregate is required to be evaluated by this test method regardless of the amount of minus No. 4 (4.75 mm) material present. However, the amount of material between the No. 16 (1.18 mm) and No. 4 (4.75 mm) sieves is included in the weight of the test sample when calculating the percent of clay lumps and friable particles.



## Calculations

Calculate the percent of clay lumps and friable particles in the individual sizes as follows:

$$P = \frac{M-R}{M} \times 100$$

where:

P = percent of clay lumps and friable particles

M = weight of test sample (this is the weight of each size increment prepared for test)

Note: include the weight of the plus No. 16 (1.18 mm) to minus No. 4 (4.75 mm) when needed, if the aggregate contains both coarse and fine particles.

R = Weight of particles retained on a designated sieve

The percent of clay lumps and friable particles in coarse aggregate is an average based on the percent of clay lumps and friable particles in each sieve size fraction weighed in accordance with the grading of the original sample, or preferably the average grading of the entire lot. When the sample contains less than 5 % of the total material in a given size, based on the original grading of the aggregate sample, the increment is considered to have the same percent of clay lumps and friable particles as the next larger or smaller fraction, whichever is present (see Table 3).

Table 3

Particle Size	Original Sample Percent Retained	Percent Clay Lumps and Friable Particles	Weighted Average Percent
No. 4 to 3/8 in. (4.75 mm to 9.5 mm)	24	13	3.12
3/8 in. to 3/4 in. (9.5 mm to 19.0 mm)	15	8	1.20
3/4 in. to 1 1/2 in. (19.0 mm to 37.5 mm)	4	8*	0.32
Total Percent in aggregate			4.64
* the percent of material retained on the fraction from 3/4 in. (19.0 mm) to 1 1/2 in. (37.5mm) is less than 5 %, therefore the percent of clay lumps and friable particles found to be in the next smaller size increment (8 %) is used in the weighted average.			

# **TOTAL MOISTURE CONTENT OF AGGREGATE BY DRYING**

## **AASHTO T 255**

### **SCOPE**

The moisture content in aggregate is used to determine the binder content for HMA during production of the mixture in a plant. The procedure requires that a known amount of aggregate be obtained, the aggregate heated to remove the moisture, and the percentage of moisture determined. Ovens, hot plates, heat lamps or microwave ovens are used for heating the sample.

### **SUMMARY OF TESTS**

#### **Apparatus**

Balance, general purpose class G<sub>2</sub> (AASHTO M 231)

Source of Heat, oven capable of maintaining a temperature of  $239 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ), electric or gas hot plate, electric heat lamps, or microwave oven

Sample Container, suitable for method of heating

#### **Heat Sources for Aggregate Drying**

There are several alternatives to choose from when drying aggregates.

##### Hot Plate

The hot plate is an excellent choice for heating when in a hurry; however, care should be taken to avoid excessive localized overheating and fracturing of aggregates. If fracturing of the aggregate occurs, another sample is required. When a hot plate is used, the sample is stirred repeatedly while observing the state of the aggregate. Some types of aggregate will not tolerate the high localized heat and may fracture despite the best of care. In this case, an oven should be used.

##### Oven

The most common heat source is an oven set at  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ). An oven is a good choice when time is not of the essence. Samples dried in the oven, depending on the type of container used and the moisture content of the sample, may take anywhere from one to several hours to dry to a constant weight. The benefit of using an oven is that sensitive aggregates will not likely overheat and fracture.

## Microwave

The microwave oven is a quicker solution than a hot plate, except that microwave drying will often fracture and pop the aggregate particles. Some experimentation will be necessary to ensure the best settings for the material, to avoid this situation. The microwave should not be used where there is metal or metal oxides present in the aggregate.

## **Sample**

The aggregate sample shall be obtained in accordance with AASHTO T 2 and protected against loss of moisture prior to determining the weight. An air-tight container or plastic bag is best for this purpose. The size of sample shall be as follows:

<b>Nominal Maximum Size</b>	<b>Minimum Sample Weight lb(kg)</b>
No. 4 (4.75 mm)	1.1 (0.5)
3/8 in. (9.5 mm)	3.3 (1.5)
1/2 in. (12.5 mm)	4.4 (2)
3/4 in. (19.0 mm)	6.6 (3)
1 in. (25.0 mm)	8.8 (4)
1 1/2 in. (37.5 mm)	13.2 (6)
2 in. (50.0 mm)	17.6 (8)

## **Procedure**

1. Weigh the sample and record the weight (W)
2. Dry the sample until there is less than 0.1% change in weight over subsequent weighings.
3. Record the weight (D) of the sample after the sample has cooled sufficiently not to damage the balance.

## Calculations

The calculation for moisture content (P) is as follows:

$$P = 100 \frac{(W - D)}{D}$$

where:

P = moisture content of sample, %

W = original wet weight of sample, gms

D = dry weight of sample, gms

Example:

$$W = 546.2 \text{ gms. } D = 541.2 \text{ gms}$$

$$P = 100 \frac{(546.2 - 541.2)}{541.2}$$

$$P = \frac{500}{541.2}$$

$$P = 0.92\%, \text{ record as } 0.9\%$$

The moisture content is calculated to the nearest first decimal place (0.0).

# **REDUCING SAMPLES OF AGGREGATE TO TESTING SIZE**

## **AASHTO R 76**

### **GLOSSARY**

Nominal Maximum size - The smallest sieve opening through which the entire amount of the aggregate is permitted to pass.

Saturated Surface Dry (SSD) - An aggregate is considered to be in a saturated surface dry condition when there is no free moisture present but the aggregate is in a nonabsorbent state.

Air Dry - When the aggregate appears to be dry but still has some absorbed moisture in the pore structure.

### **SCOPE**

The field samples of aggregate must generally be reduced to an appropriate size for testing to determine physical characteristics, such as, sieve analysis, soundness, hardness, etc. The methods described in this test method are intended to minimize variations in the aggregate characteristics between the smaller test sample and the larger field sample.

Several methods of sample reduction will be described. The technician must be sure to use the appropriate technique dependent on such factors as aggregate size and moisture content.

The reduction methods include:

Method A - Mechanical Splitter

Method B - Quartering

Method C - Miniature Stockpile

In some circumstances, reducing the field sample prior to testing is not recommended. Substantial differences may unavoidably occur during sample reduction, i.e., in the case of an aggregate having relatively few large size particles in the sample. These few particles may be unequally distributed among the reduced size test samples. If the test sample is being examined for certain contaminants occurring as a few discrete particles in a small percentage, the reduced test sample may not be truly representative of the total aggregate as produced. In these cases, the entire original field sample should be tested.

Failure to carefully follow the procedures in these methods of sample reduction may result in providing a nonrepresentative sample for subsequent testing, resulting in inaccurate test results, and ultimately, failure of the aggregate to perform as intended.

## SUMMARY OF PROCEDURE

Aggregate and other materials sampled in the field need to be reduced to appropriate sizes for testing. It is, therefore, necessary to reduce field samples while minimizing the chance of variability during handling. In some instances a few particles on a given sieve might effect a gradation significantly enough to alter an interpretation of the field sample and subsequently the entire material's compliance with specifications.

The appropriate field sample reduction method is dependent chiefly on the nominal maximum size of the aggregate, the amount of free moisture in the sample, and the equipment available.

The following chart should be used in selecting the appropriate reduction method for the aggregate to be tested.

<b>Mechanical Splitter</b>	<b>Quartering</b>	<b>Miniature Stockpile</b>
Fine Aggregates - Air Dry	Fine Aggregates –Free Moisture on the Particle Surface	Fine Aggregates – Free Moisture on the Particle Surface
Coarse Aggregates	Coarse Aggregates	Not Permitted for Coarse Aggregates
Combined Aggregates - Dry	Combined Aggregates – Dry or Free Moisture on the Particle Surface	Not Permitted for Combined Aggregates

## METHOD A -- MECHANICAL SPLITTER

### Apparatus

The mechanical sample splitter shall have an even number of equal width chutes, not less than eight for coarse or combined aggregate, or twelve for fine aggregate. The chutes shall discharge alternately to each side of the splitter. For coarse and combined aggregate the width of the individual chutes shall be approximately twice the largest size particle in the sample to be reduced. For dry fine aggregate in which the entire sample will pass the 3/8 in. (9.5 mm) sieve, the minimum width of the chutes shall be at least fifty percent larger than the largest particles in the sample with a maximum width of 3/4 in. (20 mm).

The splitter shall be equipped with at least two receptacles (catch pans) to hold the two halves of the sample during splitting. It shall also be equipped with a hopper or straight-edge pan with a width equal to or slightly less than the overall width of the assembly of chutes, by which the sample may be fed at a controlled rate into the chutes.

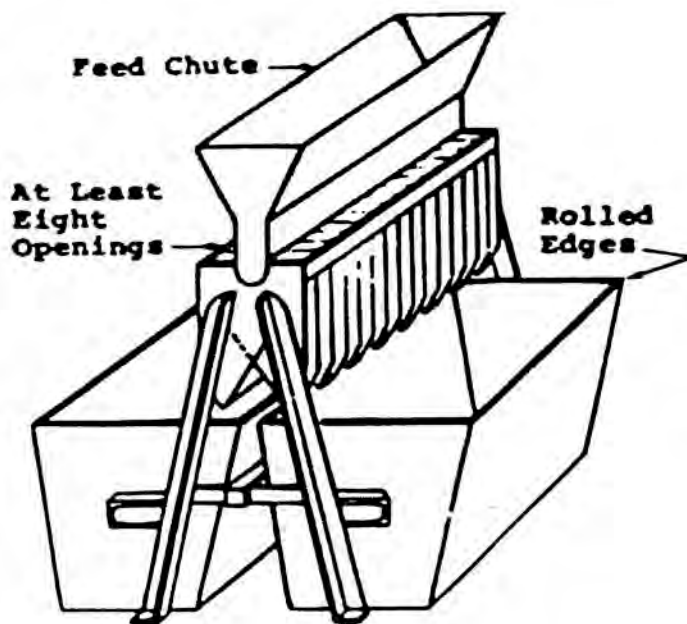
The splitter and accessories shall be designed to allow the sample to flow smoothly without restriction or loss of material.

Mechanical splitters are commonly available in sizes adequate for aggregate having the largest particle size not over 1 1/2 in. (37.5 mm).

## Procedure

1. If the mechanical splitter procedure is desired and the sample is damp or shows free water, the sample may be dried until the sample appears dry or until clumps may be easily broken by hand. The dryness of the sample may be tested by tightly squeezing a small portion of the sample in the palm of the hand. If the cast crumbles readily, the correct moisture range has been obtained.
2. Place the original sample, or portion thereof, in the hopper or pan and uniformly distribute it from edge to edge being sure the sample appears homogenous (well-blended). Carefully introduce the sample into the chutes in a manner to allow the aggregate to flow freely through the openings and into the catch pans. Continue this procedure until the entire sample has been halved, being careful that catch pans do not overflow.
3. Remove the catch pans and set aside. Continue splitting one half of the material. Follow this procedure, being sure to split entire increments, until the desired test sample size is obtained. Retain the unused material until all desired tests are performed in case a retest is needed.

Note: Sometimes a significant amount of fines may be lost in the splitting process if the sample is extremely dry and the action of pouring the sample through the splitter chutes creates a large dust cloud, suspending the fines in the air above the splitter. If this is a serious concern, then add a small amount of water to the original sample and mix thoroughly before splitting the sample. The extra moisture will prevent many of the fines from becoming suspended in the air and drifting off. Remember to not add so much water that the moisture content ends up being at or greater than the SSD condition, in which case the mechanical splitting method would no longer be valid. In any case, be sure to perform the splitting procedure in a well-ventilated area while wearing a suitable dust mask.

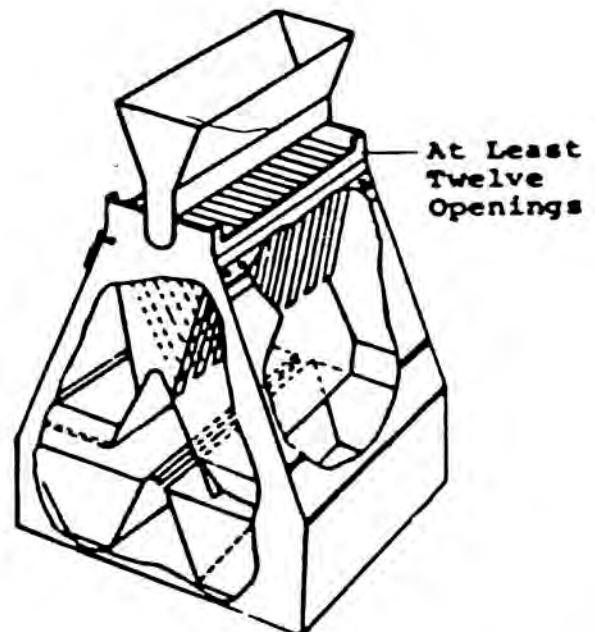
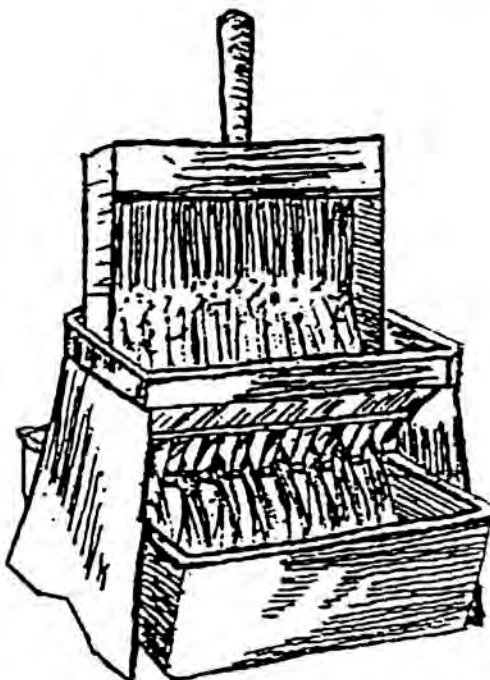


**Riffle Sample Splitter**



**Riffle Bucket and  
Separate Feed Chute Stand**

(a) Large Riffle Samplers for Coarse Aggregate.



NOTE—May be constructed as either closed or open type. Closed type is preferred. (b) Small Riffle Sampler for Fine Aggregate.

**Figure 1**  
Sample Splitters



## MECHANICAL SAMPLE SPLITTER



**Mechanical Splitter**



**Sample in Splitter**



**Sample Being Split**

## **METHOD B -- QUARTERING**

### **Apparatus**

Straight-edged scoop.

Flat-edged shovel or trowel.

Broom or brush.

Alternate method only - canvas blanket measuring approximately 6 ft. x 8 ft (2 m by 2.5 m).

### **Procedure**

1. Place the original sample on a hard, clean, level surface. Mix the material thoroughly by turning the entire sample over with the shovel three times. With the last turning, shovel the entire sample into a conical pile by depositing each shovelful on top of the preceding one. Carefully flatten the conical pile to a uniform thickness and diameter by pressing down the apex with the shovel so that each quarter section of the resulting pile will contain the material originally in the pile. The pile diameter should be approximately four to eight times the thickness.
2. Divide the flattened pile into four equal quarters with the shovel or trowel. Remove two diagonally opposite quarters, including all fine material. Brush the cleared spaces clean. Successively mix and quarter the remaining material in the same fashion as the original sample. Continue this process until the desired quantity is obtained.

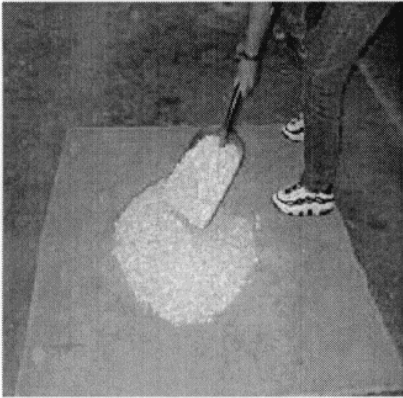
Save the unused portion of the original field sample until all testing is completed in case a retest is needed.

## **METHOD B -- ALTERNATIVE**

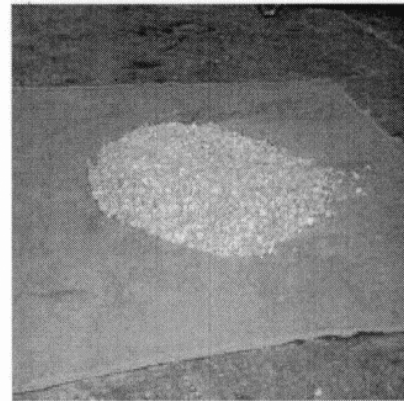
As an alternative to Method B, when the floor surface is uneven, the field sample may be placed on a canvas blanket and mixed with a shovel, or by alternatively lifting each corner of the blanket and pulling the blanket over the sample toward the diagonally opposite corner causing the material to be rolled. Flatten and divide the pile as described in Method B, or if the surface beneath the blanket is too uneven, insert a stick or pipe dividing the pile into two equal parts. Remove the stick leaving a fold in the canvas between the sample halves. Slide the stick under the canvas blanket again at a right angle to the first division and dissecting the two halves of the sample through their centers. Lift the stick evenly from both ends dividing the sample into equal quarters. Remove two diagonal parts including the fine material and clean the area. Successively mix and quarter the remaining material until the desired sample size is obtained.

Note: The quartering method is fairly time intensive and thus is generally used in situations where an adequate mechanical splitter is unavailable. Diligence and care is required to ensure that the samples obtained by quartering remain representative of the entire field sample.

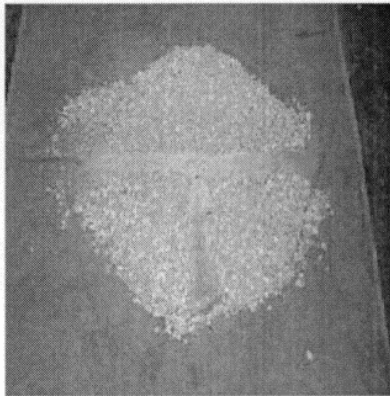
## METHOD B



**Mix by Forming New Cone**

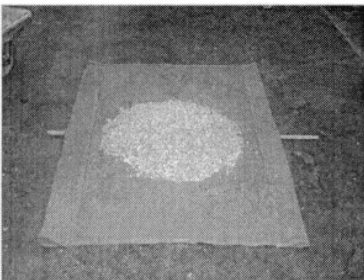


**Flatten Cone**

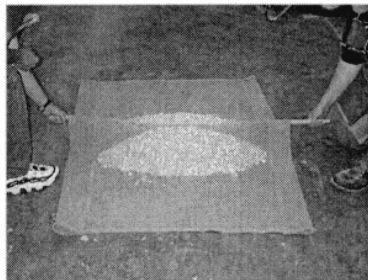


**Divide Sample Into  
Quarters**

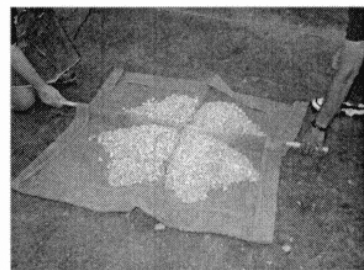
**Method B ( alternative)**



**Stick Placed Under  
Flattened Sample**



**Sample Divided in Half**



**Sample Divided  
Into Quarters**

## METHOD C -- MINIATURE STOCKPILE

### Apparatus

Straight-edge scoop.

Shovel or trowel (for mixing the aggregate).

Small sampling thief, small scoop, or spoon.

### Procedure

This method is for damp, fine aggregate only.

1. Place the field sample on a hard, clean, level surface where there will be no loss of material or contamination. Mix the sample by turning the entire sample over three times with a shovel. With the last turning, shovel the entire sample into a conical pile by depositing each shovelful on top of the preceding one.
2. Obtain a sample for each test to be performed by selecting at least five increments of material at random locations from the miniature stockpile using a sample thief, small scoop, or spoon.



Miniature Stockpile



Taking One of at Least Five Samples

# **UNCOMPACTED VOID CONTENT OF FINE AGGREGATE**

## **AASHTO T 304**

### **GLOSSARY**

**Voids** - Difference between the total volume and the volume occupied only by the aggregate particles. The amount of void space (or air space) is a function of the aggregate gradation, particle shape and texture, and the amount of compaction of the material.

**Uncompacted Voids** - The amount of void space present when the material is in an uncompacted, unconsolidated state.

**Bulk Dry Specific Gravity** - The ratio of the weight in air of a unit volume of aggregate at a stated temperature to the weight in air of an equal volume of gas-free distilled water at the stated temperature.

**Angularity** - A description of the degree of roughness, surface irregularities or sharp angles of the aggregate particles (i.e. particle shape).

### **SCOPE**

This method determines the loose uncompacted void content of a sample of fine aggregate. When performed on an aggregate sample of a known, standard grading (Method A), this measurement provides an indication of particle shape. The material angularity, roundness or surface texture relative to other materials of the same standard grading is indicated by the percent of voids determined by this test. The Superpave asphalt mix design method sets minimum requirements for void content that vary depending on traffic loads and depth from the surface of the asphaltic concrete pavement.

In this method, the prepared sample is allowed to free-fall through a standard funnel of a specified diameter from a specified height into a small cylinder of known volume (nominal 100 mL). The material is then leveled with the top of the calibrated cylinder and weighed. Because the volume and weight of the cylinder are known, the weight of the sample contained in the cylinder may be calculated. Using the Bulk Dry Specific Gravity (as determined by AASHTO T 84), the volume of the material in the cylinder is calculated. By subtracting the calculated volume of material from the calibrated volume of the test cylinder, the volume of voids may be calculated.

## SUMMARY OF TEST

A sample of sand is prepared in accordance with one of three methods. Method A, a standard gradation, is the most common used. The sample is allowed to free-fall from a funnel into a cylinder of known volume. Using the Bulk Dry Specific Gravity of the sample as determined by AASHTO T 84, the percent of void space in the cylinder is calculated. This value is known as the Fine Aggregate Angularity Value or FAA.

### Apparatus

Cylindrical measure, approximately 1.56 in. (39 mm) in diameter, 3.44 in. (86 mm) deep with a capacity of approximately 100 mL.

Funnel, conforming to Figure 2 in AASHTO T 304.

Funnel Stand, conforming to Figure 2 in AASHTO T 304.

Glass Plate, for calibrating cylindrical measure.

Pan, large enough to contain funnel stand and to catch overflow material. The pan shall not be warped so as to prevent rocking of the apparatus during testing.

Metal spatula, with a straight edge approximately 4.0 in. (100 mm) long and 0.8 in. (20 mm) wide.

Balance, accurate and readable to 0.1 grams.

**Procedure** - Only Method A will be discussed in this procedure. For the other methods consult AASHTO T 304

1. Wash representative sample in accordance with T 11. The size of this sample is dependent on the gradation of the sample. Generally 500 grams to 700 grams is sufficient to yield the necessary size fraction quantities.
2. Dry washed sample material in a  $230 \pm 9^{\circ}\text{F}$  ( $110 \pm 5^{\circ}\text{C}$ ) oven to a constant weight.

Sieve material in accordance with AASHTO T 27. Remove the following size fractions from the sieves and retain in separate, labeled containers:

Passing No. 8 (2.36 mm) - Retained on No. 16 (1.18 mm)  
Passing No. 16 (1.18 mm) - Retained on No. 30 (600  $\mu\text{m}$ )  
Passing No. 30 (600  $\mu\text{m}$ ) - Retained on No. 50 (300  $\mu\text{m}$ )  
Passing No. 50 (300  $\mu\text{m}$ ) - Retained on No. 100 (159  $\mu\text{m}$ )

3. Weigh individual size fractions and combine them in accordance with the following:

<u>Size Fraction</u>	<u>Weight, grams</u>
No. 8 (2.36 mm) x No. 16 (1.18 mm)	44
No. 16 (1.18 mm) x No. 30 (600 $\mu\text{m}$ )	57
No. 30 (600 $\mu\text{m}$ ) x No. 50 (300 $\mu\text{m}$ )	72
No. 50 (300 $\mu\text{m}$ ) x No. 100 (150 $\mu\text{m}$ )	<u>17</u>
Total	190

4. Mix combined sample thoroughly with spatula.
5. Place finger under opening in funnel to seal opening. Pour mixed sample into funnel (Figure 1).



**Figure 1**  
Pouring Sample into Funnel

6. Quickly remove finger from funnel and allow sample to free-fall into the calibrated cylinder.
7. Take care not to vibrate or unnecessarily disturb the material in the cylinder to avoid further consolidation. Strike off the excess material above the lip of the cylinder with the spatula edge, held in a vertical position, using one continuous motion.
8. After striking off, remove any excess sand from the outside of the cylinder using a small brush. At this point, additional compaction of the material in the cylinder will not affect the test results and will aid in handling.

9. Weigh the cylinder with the sample and record to the nearest 0.1g (Figure 2). Retain and recombine all materials for the next trial.



**Figure 2**  
Weighing the Cylinder

10. Repeat test using the recombined sample. Calculate and report the average of at least two trials.

### Calculations

Calculate the uncompacted voids content as follows:

$$U = \frac{V - (F/G)}{V} \times 100$$

Where:

V = Volume of calibrated cylinder in mL (cubic centimeters)

F = Net Weight of Sample in cylinder (gross weight minus weight of empty cylinder)

G = Bulk Dry Specific Gravity as determined by AASHTO T 84

U = Uncompacted Voids in Percent (reported to nearest 0.1%)

### Example:

Volume of Cylinder: 99.92 mL

F = 156.4 g

G = 2.643

$$U = \frac{99.92 - (156.4 / 2.643)}{99.92} \times 100 = 40.8$$



# FLAT PARTICLES, ELONGATED PARTICLES, OR FLAT AND ELONGATED PARTICLES IN COARSE AGGREGATE

## ASTM D 4791

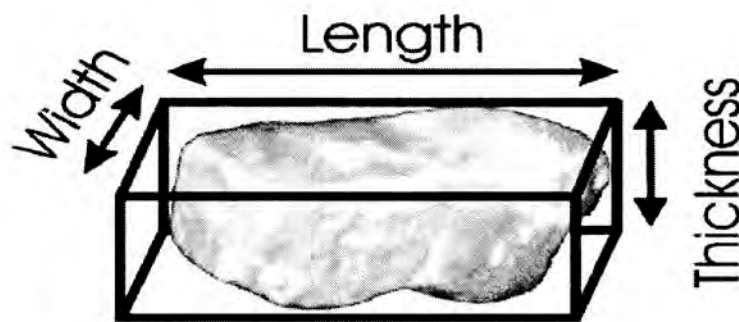
### GLOSSARY

Flat and Elongated Particles of Aggregate - Those particles having a ratio of length to thickness greater than a specified value.

Length - the maximum dimension.

Thickness - the maximum dimension perpendicular to the length and width.

Width - the maximum dimension in the plane perpendicular to the length.



### SCOPE

This test method covers tests for flat particles, elongated particles, or flat and elongated particles in coarse aggregate. In this text only flat and elongated particles in accordance with the procedure designated in Method B will be covered.

Flat and elongated particles of coarse aggregates have a tendency to fracture more easily than other aggregate particles. When the coarse aggregate does fracture, the gradation will likely change which may be detrimental to the mix. Additionally, flat and elongated particles of aggregate, for some construction uses, may interfere with consolidation and may result in harsh, difficult to place mixtures.

## SUMMARY OF TEST

Individual aggregates of specific sieve sizes are tested for ratios of width to thickness, length to width, or length to thickness. The test is performed on a sample of coarse aggregate reduced from a representative field sample. The sample is sieved to separate each size larger than the 3/8 in. (9.5 mm) sieve. Each size is then tested in a proportional caliper device. Particles are weighed to determine a percentage of flat, elongated, or flat and elongated particles in a sample. Superpave specifications require hot mix asphalt to have less than 10% flat and elongated particles using a 5:1 ratio.

### Apparatus

Proportional Caliper Device.

Balance, accurate to 0.5% of the weight of the sample.

Oven or hot plate (if determination is made by weight)

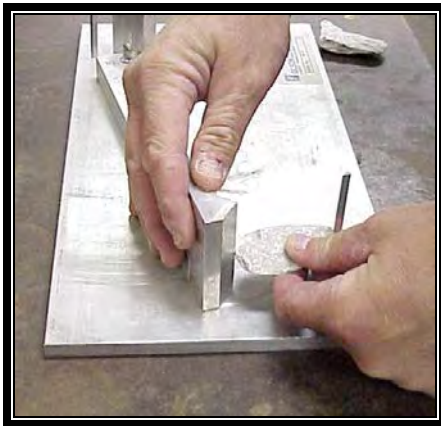
### Sample Size

Set up the test sample according to the following table:

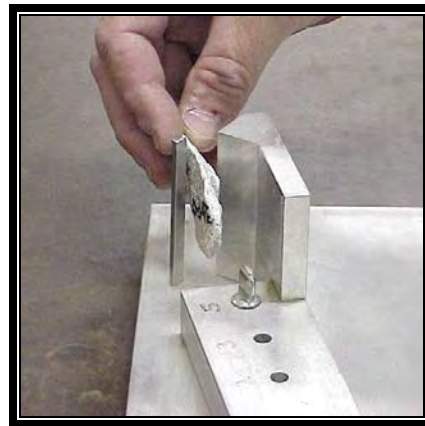
Nominal Maximum Size	Minimum Weight
3/8 in. (9.5 mm)	2 lbm. (1 kg)
1/2 in. (12.5 mm)	4 lbm. (2 kg)
3/4 in. (19.0 mm)	11 lbm. (5 kg)
1 in. (20.5 mm)	22 lbm. (10 kg)
1 1/2 in. (37.5 mm)	33 lbm. (15 kg)
Note: This is the entire sample. Only test the sieve sizes that are present in the amount of 10% or more of the original sample.	

## Procedure

1. If determination by weight is required, oven dry the sample to a constant weight at a temperature of  $230^{\circ} \pm 9^{\circ}\text{F}$  ( $110^{\circ} \pm 5^{\circ}\text{C}$ ). If determination is by particle count, drying is not necessary.
2. Sieve the sample of coarse aggregate to be tested in accordance with test method AASHTO T 27. Reduce each size fraction larger than the 3/8 in. (9.5 mm) sieve that is present in the amount of 10% or more of the original sample in accordance with method AASHTO T 248 until approximately 100 particles are obtained.
3. Use the proportional caliper device positioned at the 5:1 ratio.
4. Set the larger opening equal to the particles longest dimension. The particle is considered flat and elongated if the particles thinnest dimension passes through the smaller opening. (Figures 1 and 2)



**Figure 1**  
Checking Elongation



**Figure 2**  
Checking Flatness

5. Test each of the particles in each size fraction and place them into one of two groups: (1) Particles with longest to thinnest ratios over 5:1 and (2) Particles with longest to thinnest ratios less than 5:1
6. After the particles have been classified into the two groups, determine the proportion of the sample in each group by either count or by weight as required.

## Calculations

Calculate the percentage of flat and elongated particles to the nearest 1% for each sieve size greater than 3/8 in. (9.5 mm).

### Example

3/4 in. (19.0 mm) Stone

Sieve	1 in. (25.0 mm)	3/4 in. (19.0 mm)	1/2 in. (12.5 mm)	3/8 in. (9.5 mm)
% Passing	100	99.4	75.7	46.4
% Retained	0	0.6	23.7	29.3

No test is performed on the 3/4 in. (19.0 mm) size aggregate because this material is less than 10 % of the total sample. Assume that the 3/4 in. (19.0 mm) particles have the same percentage of flat and elongated as the next sieve which is the 1/2 in. (12.5 mm) sieve.

The 1/2 in. (12.5 mm) size material totaled 715.3 grams after reducing to approximately 100 particles. 6.9 grams were classified as flat and elongated, therefore, the percent flat and elongated on the 1/2 in. (12.5 mm sieve) is:

$$\frac{6.9}{715.3} \times 100 = 1.0\% \approx 1\%$$

Likewise, the 3/8 in (9.5 mm) size totaled 239.7 grams after reduction and 12.2 grams were classified as flat and elongated. The percent flat and elongated on the 3/8 (9.5 mm) sieve is:

$$\frac{12.2}{239.7} \times 100 = 5.1\% \approx 5\%$$

The percentage of flat and elongated particles on each sieve is reported to the nearest whole percent.

# **DETERMINING PERCENT OF FRACTURED PARTICLES IN COARSE AGGREGATE**

## **ASTM D 5821**

### **GLOSSARY**

**Fractured Face** - A fractured face is defined as being caused either by mechanical means or by nature and should have sharp or slightly blunted edges. Natural fractures, to be accepted, must be similar to fractures produced by a crusher. A broken surface constituting an area equal to at least 25% of the projected area of the particle, as viewed perpendicular to (looking directly at) the fractured face, is considered an acceptable fractured face.

**Fractured Rock Particle** - A rock particle having at least one or two fractured faces, as required for that class/type of aggregate in the specifications.

### **SCOPE**

This test procedure determines the amount (percent) of fracture faced rock particles, by visual inspection that meets specific requirements. The fractured face of each rock particle must meet a minimum cross-sectional area. Specifications contain requirements for percentage of crushed aggregate particles, with the purpose of maximizing shear strength in either bound or unbound aggregate mixtures. This method may be used in determining the acceptability of coarse, dense-graded, and open-graded aggregates with respect to such requirements. This procedure is used primarily for hot mix asphalt aggregates.

### **SUMMARY OF TEST**

#### **Apparatus**

Sieves, appropriate for the sample type

Balance, appropriate for the size of sample and in accordance with AAASHTO M 231 for a class G<sub>2</sub> balance

Spatula or similar tool to aid in sorting the aggregate particles

Paper containers

## Sample Preparation

Air-dry the representative sample prior to the coarse gradation process so that there is a clean separation of the particles. A total + No. 4 (4.75 mm) sample could be used for testing but more commonly the + No. 4 (4.75 mm) material will be split into representative fractions. The minimum size of samples shall be as follows:

Nominal Maximum Sieve Sizes*	Minimum Sample Size lbm (g)
3/8 in. (9.5 mm)	0.5 lbm(200 g)
1/2 in. (12.5 mm)	1 lbm (500 g)
3/4 in.(19.0 mm)	3 lbm (1500 g)
1 in.(25.0 mm)	6 lb,m (3000 g)
1 1/2 in.(37.5 mm)	16.5 lbm (7500 g)

\*Note: Nominal maximum sieve size is defined as the smallest sieve opening through which the entire amount of the aggregate is permitted to pass.

For aggregate with a nominal maximum size of 3/4 in. (19.0 mm) or larger, the test sample may be separated on the 3/8 in. (9.5 mm) sieve. The portion passing the 3/8 in. (9.5 mm) sieve may then be further reduced to a minimum of 0.5 lbm (200 g). This will reduce the number of particles to be separated during the procedure. In this case, percent fractured particles is determined on each portion and a weighted average percentage of fractured particles is calculated.

## Procedure

1. Wash and dry sample to a constant weight. Weigh the test sample to the nearest 0.1g and record as "Test Sample Weight".
2. Spread the test sample on a clean flat surface large enough to permit the material to be spread thinly for careful inspection and evaluation.
3. Using the spatula or a similar tool separate the particles into one of the following three categories:

Crushed Particles, using the criteria of "one or more fractured faces" or "two or more fractured faces" as is consistent with the requirements in the specifications.

Uncrushed Particles

4. Determine the weight of the "Crushed Particles" and record the weights as "Weight of Crushed Particles".

## Calculations

1. Calculate the percentage of crushed particles as follows:

$$\text{Percent Crushed Particles (P)} = \frac{F}{F + N} \times 100$$

Where: F = Weight of crushed particles with at least the specified number of fractured faces, in grams

N = Weight of uncrushed particles, in grams

### Example:

$$\begin{aligned} F &= 730 \\ N &= 1016 \end{aligned}$$

$$P = \frac{730}{730 + 1016} \times 100 = 41.8\%$$

2. For aggregate with a nominal maximum size of 3/4 in. (19.0 mm) or larger, the test sample may be separated on the 3/8 in. (9.5 mm) sieve. The percent fractured particles is determined on each portion and a weighted average percentage is calculated.

### Example:

3/4 - 3/8 in. (19.0 - 9.5 mm) Material	=	3766g
3/8 - No. 4 (9.5 - 4.75 mm) Material	=	7314g
		-----
Total + No. 4 (4.75 mm) Material	=	11080g

$$\text{Percent } 3/4 - 3/8 \text{ in. (19.0 - 9.5 mm)} = \frac{3766}{11080} \times 100 = 34\%$$

$$\text{Percent } 3/8 \text{ in. - No. 4 (9.5 - 4.75 mm)} = \frac{7314}{11080} \times 100 = 66\%$$

Total Percent Crushed Particles =

(% Crushed Particles 3/4 to 3/8 in. [19.0 - 9.5 mm]) x  
(% of 3/4 to 3/8 in. [19.0 - 9.5 mm] Material)

+

(% Crushed particles 3/8 in. - No.4 [9.5 - 4.75 mm]) x  
(% of 3/8 in. - No. 4 [9.5 - 4.75 mm] Material)

Using the following Data

% Crushed Particles 3/4 - 3/8 in. (19.0 - 9.5 mm) = 35.7%

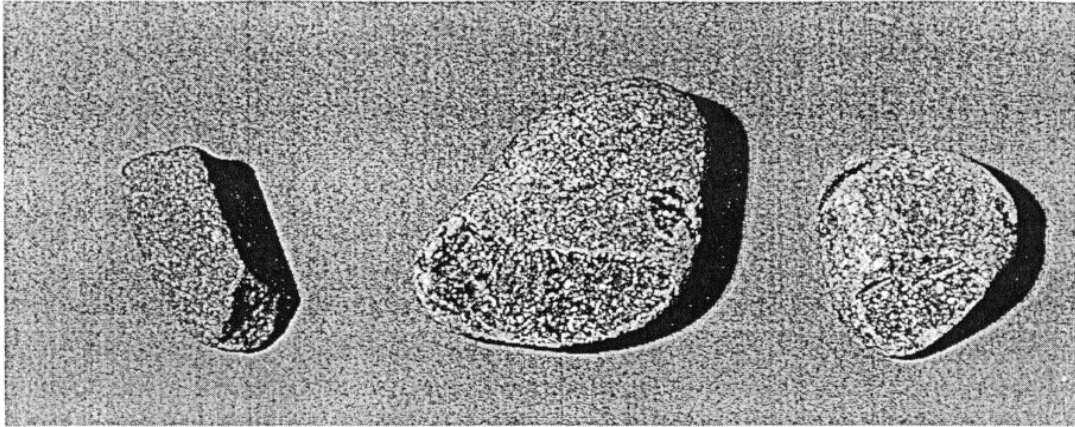
% Crushed Particles 3/8 in. - No. 4 (9.5 - 4.75 mm) = 75.1%

% Crushed Particles = [(0.357 x 0.34) + (0.751 x 0.66)] x 100

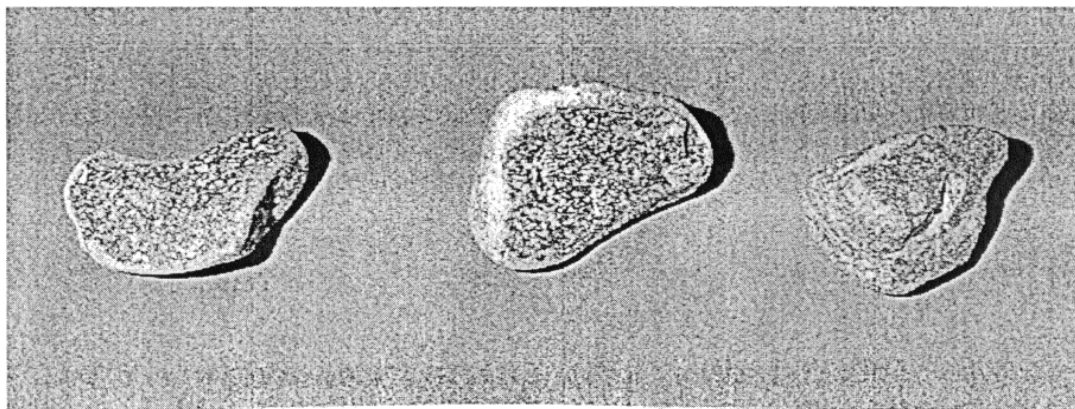
= [(0.121) + (0.496)] x 100

= 61.7%

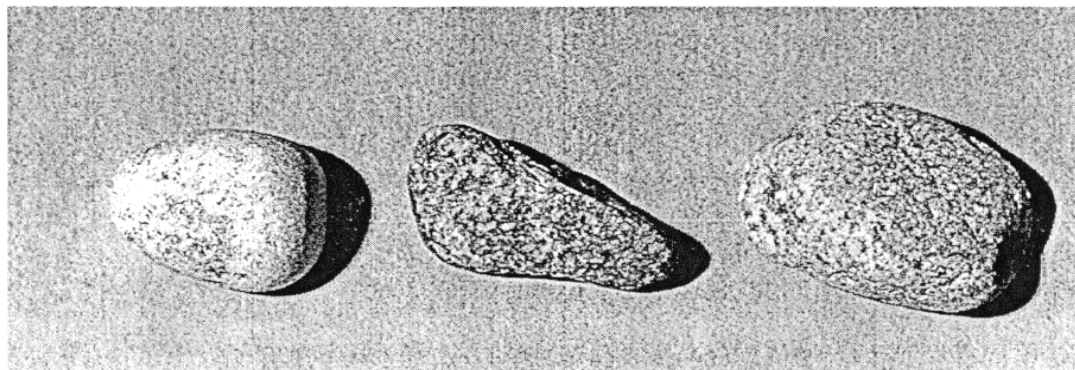




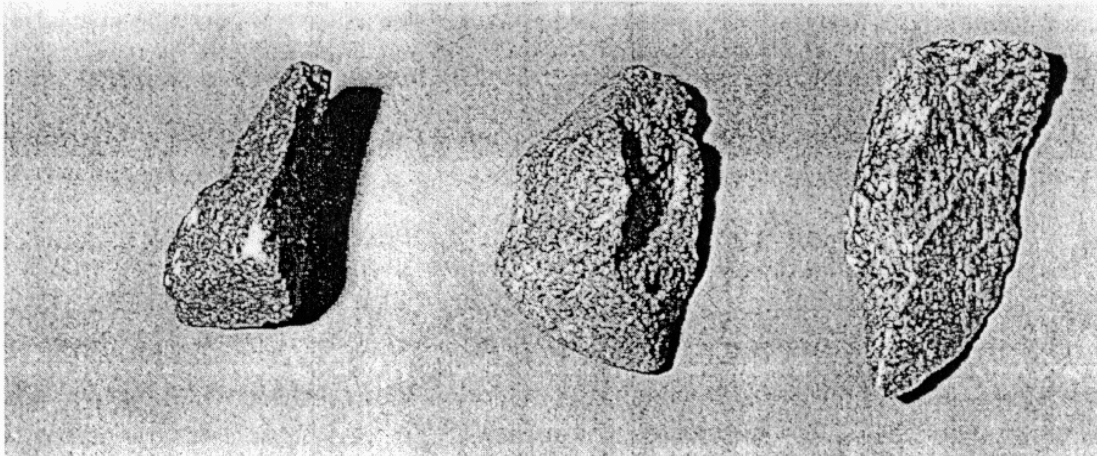
Fractured particle (center) flanked by two non-fractured particles (chipped only).



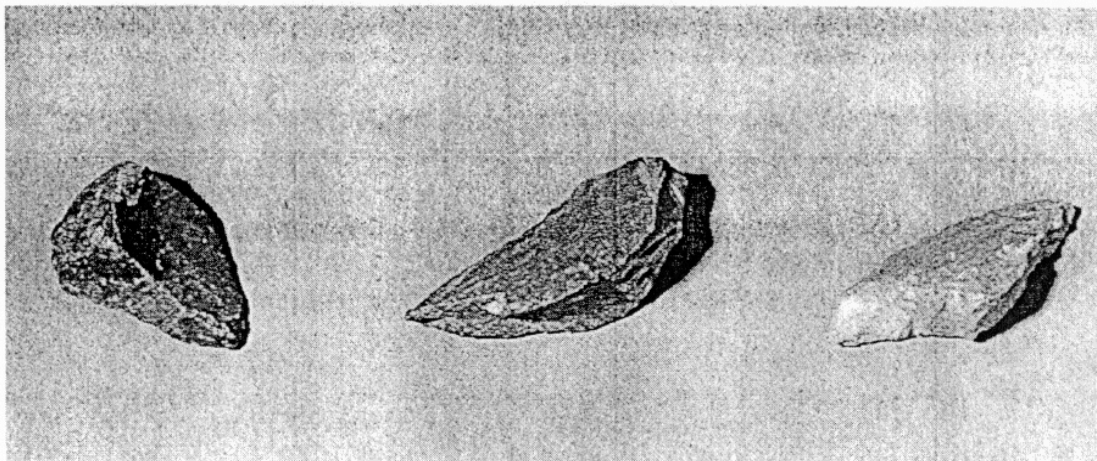
Non Fractured particle (round edges, smooth surfaces



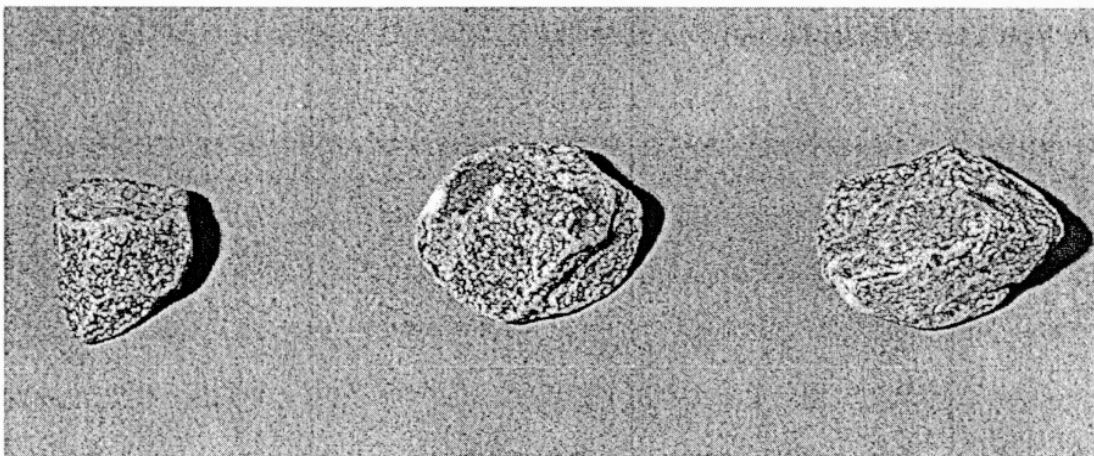
Non Fractured particle (rounded particles, smooth surfaces).



Fractured particle (sharp edges, rough surfaces).



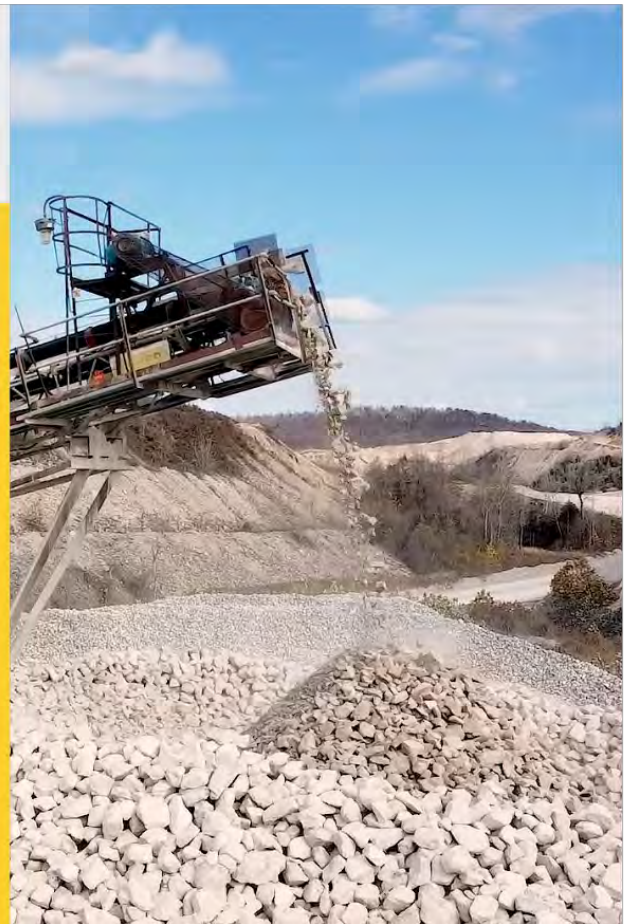
Fractured particle (sharp edges, smooth surfaces)



Fractured particle (round edges, rough surfaces).

# INDIANA TEST METHODS

## CHAPTER 14



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# ITM REFERENCE

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**ITM 202**

**ITM 203**

**ITM 205**

**ITM 206**

**ITM 207**

**ITM 209**

**ITM 210**

**DIRECTIVE 201**

**ITM 211**

**ITM 212**

**ITM 214**

**ITM 219**

**ITM 220**

**ITM 221**

**ITM 224**

**ITM 225**

**ITM 226**

**ITM 902**

**ITM 906**

**ITM 910**



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**ACID INSOLUBLE CONTENT OF FINE AGGREGATES  
ITM No. 202-25**

**1.0 SCOPE.**

- 1.1** This test method covers the procedure for quantitative determination of the acid insoluble content of fine aggregates used in HMA.
- 1.2** The fine aggregate is crushed or ground to a fineness sufficient to pass through a No. 30 (600  $\mu$ m) sieve. A dried sample is dissolved and digested in dilute acid, filtered, and the washed and dried residue weighed back as the insoluble fraction.
- 1.3** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- M 92 Wire-Cloth Sieves for Testing Purposes
- M 231 Weighing Devices Used in the Testing of Materials

**2.2 ASTM Standards.**

- D1193 Reagent Water
- E960 Laboratory Glass Beakers
- E1406 Laboratory Glass Filtering Flasks
- E145 Gravity-Convection and Forced-Ventilation Ovens

**2.3 OTHER Standards.**

Reagent Chemicals, American Chemical Society Specifications

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specification, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM is used to verify the minimum specification requirements for acid insoluble content of fine aggregates used in HMA.

## **5.0 APPARATUS.**

- 5.1** Sieve, No. 30, conforming to the requirements of AASHTO M 92
- 5.2** Mechanical crusher or mortar and pestle
- 5.3** Drying oven, capable of operation at 210 to 260°F in accordance with ASTM E145
- 5.4** Analytical balance, Class A, conforming to the requirements of AASHTO M 231
- 5.5** Dessicator, glass, with dessicant
- 5.6** 250 mL griffin low form borosilicate glass beakers in accordance with ASTM E960, Type I
- 5.7** Erlenmeyer vacuum flask, borosilicate glass, in accordance with ASTM E1406, Type III, Class 2, with crucible holder
- 5.8** Vacuum pump or other vacuum source
- 5.9** Ceramic or glass gooch filtering crucibles, medium porosity fritted disc, 25 mL minimum capacity
- 5.10** Hot plate, electrical, with heat control
- 5.11** Chemical fume hood

## **6.0 REAGENTS.**

- 6.1 Purity of Reagents.** Reagent grade chemicals shall be used in all tests. Unless otherwise indicated, all reagents shall conform to the specifications of the Committee on Analytical Reagents of the American Chemical Society.
  - 6.1.1** Hydrochloric acid, concentrated
  - 6.1.2** Methanol
- 6.2 Purity of Water.** Unless otherwise indicated, references to water shall be Type II reagent water in accordance with ASTM D1193.

## **7.0 SAMPLE PREPARATION.**

- 7.1** Separate 100 g of the material to be tested on a No. 30 sieve.



- 7.2** Crush or grind the portion retained on the sieve, by means that will not contaminate the sample, until substantially all of the material passes the sieve.
- 7.3** Combine all portions of the original 100 g material.
- 7.4** Quarter this material until approximately 10 g are obtained as a representative sample.
- 7.5** Dry the sample at  $221 \pm 9^{\circ}\text{F}$  for at least 2 h.
- 7.6** Remove the sample from the oven and place in desiccator to cool at least 2 h.

## **8.0 PROCEDURE.**

- 8.1** Weigh two portions of the dried sample of approximately 2.5 g each to 0.0001 g, and transfer separately into 250 mL beakers.
- 8.2** In a chemical fume hood add to each beaker 75 mL of water and 25 mL of concentrated hydrochloric acid.
- 8.3** When the initial effervescent reaction has subsided, stir and heat on a hot plate to a boil.
- 8.4** Remove the beaker from the hot plate and allow to stand at room temperature for 2 h.
- 8.5** Filter the solution on vacuum through a tared, dried, medium porosity, fritted-disc crucible, retaining the filtrate in a clean flask. If the filtrate is cloudy, refilter.
- 8.6** Wash the residue four times with water and once with methanol.
- 8.7** Dry the crucible and residue at  $248 \pm 9^{\circ}\text{F}$  for 3 h, or at  $221 \pm 9^{\circ}\text{F}$  overnight.
- 8.8** Cool the sample at least 2 h in a desiccator and weigh to 0.0001 g.

## **9.0 CALCULATIONS.** The percent of acid insoluble content is calculated by the following formula:

$$\text{Acid Insoluble Content, \%} = \frac{(W_3 - W_2)}{W_1} \times 100$$

where:

$W_1$  = weight (mass) of the dried sample

$W_2$  = weight (mass) of the crucible

$W_3$  = weight (mass) of crucible and residue

**10.0 REPORT.** The average of duplicate determinations shall be reported to the nearest 1%.





**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**CONTROL PROCEDURES FOR  
CLASSIFICATION OF AGGREGATES  
ITM No. 203-21**

**1.0 SCOPE.**

- 1.1** This method covers the control procedures used for the classification of aggregates. This control is accomplished through a rational system of categories and subcategories as set out herein. The tests conducted on ledge and production samples will be in accordance with the applicable requirements of 904.
- 1.2** These procedures will be administered to protect the interests of the Department. When unforeseen situations or interpretation difficulties arise regarding these procedures, the resolution which provides the highest quality of materials to the Department will be considered as the deciding factor.
- 1.3** All sampling and testing of production and ledge quality samples will be conducted by the Department or by an AASHTO Accredited Laboratory acceptable to the Department. Test results from the AASHTO Accredited Laboratory will be reviewed to ensure compatibility with previous test results, if applicable.
- 1.4** The final quality approval of materials for Department use will be contingent on production quality test results. When production quality testing is conducted by an AASHTO Accredited Laboratory, final quality approval will be contingent on point-of-use samples tested by the Department. Test results for other than production quality will be assigned an L rating for information only. Instances where lower classification of ledge test results has been obtained may necessitate a series of production quality samples prior to a source classification and subsequent aggregate usage approval.
- 1.5** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 ITM Standards.**

- 210 Class AP Coarse Aggregate for Concrete Pavement and Slab-on-Grade Concrete

### **3.0 TERMINOLOGY.**

- 3.1** AASHTO Accredited Laboratory. A laboratory which has demonstrated a proficiency in performing quality tests in accordance with the guidelines of the ASHTO Accreditation Program
- 3.2** Aggregate Specialist. An individual qualified to evaluate various aggregate deposits based on training in Earth Sciences. This person will be approved by the Department as being qualified to conduct various functions described in Department documents.
- 3.3** Category. Source classification used to determine the production quality sampling frequency
- 3.4** Certified Aggregate Producer (CAP). A source that has met all of the requirements to supply materials in accordance with 917
- 3.5** Class A. Quality rating assigned to aggregates which meet requirements for all Department uses except for specified slab on grade concrete applications
- 3.6** Class AP. Quality rating assigned to coarse aggregates permitted for use in all Department concrete, in particular, concrete pavement and specified slab-on-grade concrete uses in accordance with ITM 210
- 3.7** Class AS. Quality rating assigned to coarse aggregates permitted for use in SMA mixtures
- 3.8** Classes B, C, D, E, and F. Quality ratings assigned to aggregates with restricted uses
- 3.9** Class G. Quality rating assigned to materials which do not meet requirements for any Department use
- 3.10** Core Drilling Log. A written field description of a rock core sample and the operations
- 3.11** Core Sample. A rock sample obtained with a bit affixed to a barrel with drill rods that are advanced by a rotary drilling machine
- 3.12** Finished Material. Material which has been processed and proposed for use
- 3.13** Geologist. An individual qualified to evaluate various aggregate deposits based on knowledge of the principles of geology, acquired by professional education and practical experience. This person may be employed by either the drilling consultant or aggregate source for the purpose of logging core descriptions and identifying ledges.

- 3.14** Ledge. Any stratigraphic unit which may be separated from adjacent units by lithologic differences
- 3.15** Ledge Sample. Core or face sample taken to represent ledges
- 3.16** New Source. Aggregate source that has never been assigned a Department source number, or does not have past ledge and production quality test results
- 3.17** Point-Of-Use Sample. Production quality sample obtained at the last opportunity prior to incorporation into the end use
- 3.18** Production Quality Sample. An aggregate sample representing finished materials obtained at the aggregate source or the point of use
- 3.19** Source. Facility that processes or handles aggregates. A redistribution terminal will be classified as a source.
- 3.20** Source Map. A map of the quarry showing critical features and operating areas.
- 3.21** Source Sample. Production quality sample representing finished materials that are stored at an aggregate source or redistribution terminal
- 3.22** Subcategory. Source classification based on results of tests conducted on source samples and used to determine the production quality sampling frequency
- 3.23** Rating L. A rating for information only
- 4.0** **SIGNIFICANCE AND USE.** This ITM shall be used to assign the category and subcategory classification, if applicable, for each source, and to establish the guidelines for continued approval of sources supplying aggregates for Department use.
- 5.0** **SOURCE CATEGORY AND SUBCATEGORY CLASSIFICATIONS.**
  - 5.1** **General.** Each source will be assigned to one or more of four categories: GS, I, II, or III. Sources assigned to categories GS, I, or II will also be assigned to one of two subcategories: A or B. The Department reserves the right to reclassify a source at such time that reclassification is considered necessary.
  - 5.2** **Category GS.** Category GS is assigned to all sources supplying gravel, natural sand, manufactured sand from sand and gravel, or slag products.

- 5.3 Category I.** Category I is assigned to crushed stone sources that have finished products with only material from ledges with quality ratings equal to or higher than required for the proposed use, either through selective quarrying or natural occurrence.
- 5.4 Category II.** Category II is assigned to crushed stone sources that include material from ledges with quality ratings lower than required for the proposed use in finished materials. Category II assignment is used to allow the source to include small percentages of materials from lesser quality ledges that cannot be economically removed by selective quarrying, and shall not significantly affect the results of tests conducted on production quality samples. Category II assignment is not intended to allow sources to blend large amounts of lesser quality materials so that the resulting production meets higher quality specifications. Category II is assigned when the source has demonstrated capability of consistently producing material of uniform quality. This capability generally is based on the results of production quality tests conducted over a reasonable period of time, but in some cases may be based on a combination of ledge and production sample quality test results. All new stone sources with no production quality history will be rated Category II, Subcategory B, unless Category III is assigned to the source.
- 5.5 Category III.** Category III is assigned to sources which meet the requirements for Category II, but have not demonstrated a capability to consistently produce uniform products. Sand and gravel, and slag sources may be classified as Category III sources if the requirements of Category GS cannot be met. A Category III assignment is to allow any source to develop a production quality history that may eventually be used to justify a Category II or GS assignment. All Category III materials are processed, stockpiled, sampled, tested, and approved prior to use of the materials. Samples will be taken and tested as the stockpile is being built. Final acceptance will be determined when the stockpile is deemed completed. The source shall not add any material to a stockpile after the final sample has been taken. Material shall not be removed from the stockpile for non-Department use without notification to the Department prior to shipment.
- 5.6 Subcategory A or B.** Subcategory assignment is based on the results of tests conducted on source samples. Sources with production quality test results which consistently fall in the lower 75 percent of the specification range for Class A material in accordance with Table I, are assigned to Subcategory A. Sources with multiple production quality test results which consistently fall above the 75 percent specification range for Class A material in accordance with Table I, or that have quality ratings affected by the deleterious content, are assigned to Subcategory B.

Numeric Values For 75% of the Specification Range (904.03(a))				
Size	Minimum Size, (in. )	Absorption, (%)	Los Angeles Abrasion Loss, (%)	Freeze and Thaw Loss, (water (%))
2	2 1/2	---	33.75	12.0
5	1 1/2	4.50	30.0	9.0
8	1	4.50	30.0	9.0
9	3/4	4.50	30.0	9.0
11	1/2	4.50	30.0	9.0
12	3/8	4.50	30.0	9.0
43	1 1/2	---	30.0	9.0
53	1 1/2	---	33.75	12.0
73	1	---	33.75	12.0
23	3/8	---	---	7.5
24	3/8	---	---	7.5

Table I

## 6.0 FREQUENCY OF SAMPLING FOR TESTING.

**6.1 General.** The minimum frequency for ledge and production quality sampling and testing shall be as outlined within this section.

New and reactivating sources proposing to provide aggregates for Department use shall submit a written request to the appropriate District Testing Engineer. The request shall include the method of sampling and what products they intend to supply.

Sources only supplying aggregates for precast concrete items are not required to have ledge samples. Production quality samples will be required every two years. Point-of-use samples will be obtained when deemed necessary by the Department.

Aggregates for precast prestressed concrete items will require production quality sampling and testing. Point-of-use samples will be obtained when deemed necessary by the Department.

Reclassification of category and subcategory assignments of existing sources will be based on a review of the test results for the ledge and production quality samples, or production quality samples only for GS sources. The source shall request reclassification in writing to the appropriate District Testing Engineer. The Department Geologist will advise the source when a quality problem exists that may necessitate reclassification.

**6.2 Ledge Samples.** Ledge samples are required for the following conditions:

**6.2.1** A new source requests Department approval

- 6.2.2 An existing crushed stone source encounters previously untested ledges
- 6.2.3 Significant changes occur in production quality test results indicating a possible variance within existing production ledges
- 6.2.4 At the request of the Producer to evaluate selective quarrying or reassignment of previous ledge quality test results
- 6.2.5 At such time that the Department implements a new quality control program, such as adoption of new aggregate quality specifications or tests, investigation of failed materials, or other situations that may occur
- 6.2.6 At such time that the Department determines a significant lithologic or stratigraphic change has occurred since the last ledge or production samples were obtained and tested
- 6.2.7 At such time that the Department determines that reactivated areas of approved crushed stone sources are required to be tested.
- 6.3 **Production Quality Samples.** Production quality samples are required for the following conditions:
  - 6.3.1 Source samples.
    - a) In conjunction with ledge samples for crushed stone sources requesting initial Department approval
    - b) Gravel sources requesting initial Department approval
    - c) After initial Department approval in accordance with Table II
  - 6.3.2 Point-of-Use samples. After initial Department approval in accordance with Table II
- 6.4 **Testing Location.** The location of testing for ledge and production quality samples will be as follows:
  - 6.4.1 Ledge samples will be tested at the Division of Materials and Tests or an AASHTO Accredited Laboratory
  - 6.4.2 Production quality samples that require L.A. Abrasion, absorption, crushed particles, and deleterious determination will be tested at the District laboratory. Production quality samples that require soundness determination and Micro-Deval determination, if

applicable, will be tested at the Division of Materials and Tests or an AASHTO Accredited Laboratory.

- 6.4.3** Ledge and production quality samples tested at the AASHTO Accredited Laboratory will be given a SiteManager ID number and the test report from the laboratory will include this sample ID number.

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Frequency for Sampling and Testing			
Source and Point-Of-Use Production Quality Samples		Point-Of-Use Production Qualities	
Category	Subcategory	Source Production Qualities	Point-Of-Use Production Qualities
I	A	Every two years, Department sizes: 2; 5, or 8; 23 or 24 (Notes 1 and 3)	None, if all active ledges are Class A. One per year if selective quarrying and production is used to remove lesser quality materials. (Note 4)
I	B	Every year, Department sizes: 2; 5 or 8; 23 or 24 (Notes 1 and 3)	Two per year of any coarse size used and any fine size used. None in year when no material is used by the Department. (Notes 2 and 4)
II	A	Every year, Department sizes: 2; 5 or 8; 43, 53 or 73; 23 or 24 (Notes 1 and 3)	One per quarter of any coarse size used and any fine size used. None in quarter when no material is used by the Department. (Notes 2 and 4)
II	B	Every year, Department sizes: 2; 5 or 8; 43, 53 or 73; 23 or 24 (Notes 1 and 3)	One per month of any coarse size used and any fine size used. None in month when no material is used by the Department. (Notes 2 and 4)
III	--	Every year, every stockpile	See II B
GS	A	Every two years, Department sizes: 2; 5 or 8; 23 or 24 (Notes 1 and 3)	When deemed necessary by the Department.
GS	B	Every year, Department sizes: 2; 5 or 8; 23 or 24 (Notes 1 and 3)	When deemed necessary by the Department.
Notes: 1. Department approved dolomite and polish resistant aggregate sources will be sampled for size 11 or 1/2 in. material 2. The selection of the size sampled will be at random at each source. 3. Crushed and uncrushed gravel will be tested. Size 2 will be obtained when proposed for use in HMA mixtures. 4. Stone sand size 23 and size 24 will be sampled at the same frequency.			

Table II



## **7.0 LEDGE SAMPLING.**

- 7.1 General.** The source shall be responsible for preparing a ledge sampling plan that is acceptable to the Department. Core sampling shall be the primary method of ledge sampling, with face sampling an allowable alternate. If unsafe conditions appear to exist, the source or the Department has the right to refuse to allow face sampling. The sampling and handling costs shall be the responsibility of the source.
- 7.2 Ledge Sampling Plan.** The ledge sampling plan shall include, as a minimum, the following requirements:
- 7.2.1** The method for obtaining the ledge samples
  - 7.2.2** Sampling locations identified on the latest available copy of the source map from the Annual Aggregate Source Report. Reef structures may necessitate sampling at predetermined mining levels.
  - 7.2.3** Compliance with applicable current Mine Safety and Health Administration regulations
  - 7.2.4** Identification of the Source Safety Officer responsible for enforcing all safety requirements
- 7.3 Ledge Identification.** The rock type, texture, color and bedding shall be as indicated on IT-530Q. The GSA Rock Color Chart may be used as an additional reference.
- 7.4 Core Sampling.** Core sampling shall be conducted in accordance with the following requirements:
- 7.4.1** The Department Geologist shall be notified prior to starting the coring operation. In the event that thin ledges are encountered with questionable ledge contacts, the District Testing Engineer shall be immediately notified if not present.
  - 7.4.2** The Department Geologist will not direct the coring operation.
  - 7.4.3** Cores shall be drilled a minimum of 30 ft apart and represent a maximum of the equivalent of two years of production, unless otherwise approved.
  - 7.4.4** The source Geologist or Aggregate Specialist shall, in coordination with the Department Geologist, select the core locations, preliminarily identify the ledges, and determine the number of cores sufficient to obtain the required quantity of material.

A minimum of 35 lb of core sample shall be required for laboratory processing for each ledge. The approximate yield of a solid core per 1 ft is as follows:

<b>Nominal Core Diameter</b>	<b>Weight (Mass) of Core</b>
1.875 in	3.2 lb
2.000 in	3.6 lb
2.400 in.	5.2 lb
2.500 in.	5.6 lb
3.000 in.	8.1 lb
3.345 in.	10.1 lb
4.000 in.	14.4 lb
5.875 in.	31.1 lb

**7.4.5** A minimum of three cores shall be properly taken, identified and stored.

**7.4.6** The minimum ledge thickness for testing purposes shall be 1 ft.

**7.4.7** The minimum core diameter shall be a nominal size of 1.875 in.

**7.4.8** The source Geologist or Aggregate Specialist shall be responsible for completion of the Core Log TD-539 or other approved form, and the Ledge and Core Sample Description IT-530Q, and for the proper collecting, splitting, packaging, and shipping of the cores to the Division of Materials and Tests.

**7.4.9** Rock core boxes shall be weatherproof and constructed of wood or other durable materials for the protection and storage of cores while en route from the drill site to the Division of Materials and Tests. All core boxes shall be provided with longitudinal separators.

Recovered cores shall be laid in the box from upper left to lower right. The top and bottom of each coring run shall be identified. Spacers, blocks, or plugs shall be marked and inserted into the core column within the separators to indicate the top and bottom of each coring run and zones of no core recovery.

Each box shall be marked on the outside to indicate the core or boring number, source number, numerical position of the box (e.g., hole X, box 2 of 7), depth of the top and bottom of the particular core run contained in that box, date of coring, possible formation and ledge contacts, and any other pertinent information.

The entire core column shall be put into the boxes and transported to a location for splitting. The split core will be reassembled and replaced in the core box in the pre-split position. The split cores shall be submitted to the Division of Materials and Tests after consultation with the Department Geologist.

**7.4.10** The Department Geologist will examine the split cores and core drilling logs as prepared by the source Geologist or Aggregate Specialist. The following items shall be furnished as a minimum:

- a) The core locations and elevations identified by grid coordinate or physical feature reference as marked on the latest available source map from the Annual Aggregate Source Report. The elevation of the bore hole shall be referenced and surveyed from a bench mark that has been previously tied into a U.S.C. & G.S. bench mark or other recognized bench mark.
- b) A detailed description of each ledge for location, thickness, rock type, color, lithology, grain size, texture, bedding characteristics, elevation to the nearest 0.1 ft of the top and base of ledges, and any other pertinent observations. This information shall be indicated on IT-530Q.
- c) Length of particular core run
- d) Percent recovery. This information is used to determine the material that may have been washed away or for locating voids, and is determined as follows:

$$\text{Percent Recovery} = \frac{\text{Length Recovered}}{\text{Length of Core Run}} \times 100$$

- e) Name of the Geologist or Aggregate Specialist, Department representative, and quarry representative present during the coring operation. If a particular boring is not completed during a given day, the date and depth of the drilling shall be noted.
- f) Any miscellaneous information such as loss of water from the core hole, decreased or increased effort in coring by the drilling rig, or change in color of the wash cuttings. This information shall be listed in the remarks column with the approximate depth of occurrence.

**7.4.11** The wall face to be represented by the cores shall be observed by the Department Geologist prior to final ledge designation and approval.

**7.5 Face Sampling.** Face sampling shall be conducted in accordance with the following requirements:

- 7.5.1** The source shall provide a means for the Department Geologist to closely examine the face of the wall to be sampled.
- 7.5.2** Ledges shall be identified in accordance with 7.3
- 7.5.3** A source Geologist or Aggregate Specialist shall participate with the Department Geologist in all face sampling at the site.
- 7.5.4** The source shall provide a minimum of 180 lb of material from each identified ledge.
- 7.5.5** The sample will consist of approximately 60 lb of representative material from each 1/3 of the identified ledge.
- 7.5.6** The Department Geologist will determine the acceptable quantity of material with concurrence of the source Geologist or Aggregate Specialist.

## **8.0 ANNUAL AGGREGATE SOURCE REPORT.**

**8.1 General.** Crushed stone aggregate sources are required to submit an Annual Aggregate Source Report. The report may be submitted electronically or by hard copy. Sources that are not a Certified Aggregate Producer shall submit a report in accordance with 8.2. Certified Aggregate Producers shall submit a report in accordance with 8.2.1, 8.2.4, 8.2.5, 8.2.6, and 8.3, and include the report in the CAP Quality Control Plan.

### **8.2 Report.**

**8.2.1 Format.** The report shall be typed, and if maps are hand drawn they shall be in a professional manner. Limited handwritten words will be acceptable on maps and figures. The Department source number and the report date shall be noted in a consistent location on each page of the report. An example of an acceptable Annual Aggregate Source Report is included in Appendix C.

**8.2.2 Title Sheet.** The report shall contain a title sheet that lists the following information:

- a)** The year for which the report is being submitted
- b)** Source name
- c)** Source number

- d)** Mailing address
- e)** Phone number
- f)** Fax number
- g)** Names of Management and Quality Control Personnel who are responsible for reporting to the Department
- h)** Source location identified by section, township, range, longitude, latitude, and nearest identifiable points such as highways, towns, etc.

**8.2.3 Mineral Deposit Description.** On a separate page, an explanation shall be included for the following:

- a)** Thickness of the current working benches
- b)** Class AP ledges and quality expiration dates
- c)** Ledges meeting dolomite and sandstone requirements and the most recent approval dates
- d)** Ledges requiring special handling procedures
- e)** The Department classification
- f)** Any significant differences in the stratigraphic section measurements and the respective ledge thickness as reported on the most recent Summary of Ledge Quality Results letter

**8.2.4 Source Map.** The report shall include a map of the mineral deposit. If the processing or stockpiling areas are on-site, the areas may be shown on the same map. If the processing or stockpiling areas are at another location, additional maps will be required. The map may be a drawing or an aerial photograph. The source map is required to be submitted once every two years. Each map shall include the following:

- a)** Title Block. The map shall display a title block containing as a minimum the source name, Department source code number, and submittal or revision date.
- b)** North Arrow. Each map shall display a North arrow.

- c) Scale - Each map shall display a scale appropriate to the size of the property or source area; however, no scale shall be less than 1 in. = 100 ft and no greater than 1 in. = 400 ft, unless otherwise approved.
- d) Grid. A grid shall be established using a rectangular coordinate system. The grid shall be indicated on the map either directly or using an overlay. The major grid lines shall be no further apart than 300 ft and incremented at least every 75 ft as measured on the map.
- e) Benchmarks. At least one permanent benchmark shall be established on the property. All other points shall be referenced to the permanent benchmark through the true elevation. Temporary benchmarks shall also be established, and at least one benchmark shall be available for easy access to each ledge.
- f) Control Points. Permanent control points for the grid shall be established within the property perimeter. There shall be enough permanent control points to permit the reestablishment of a portion of the grid within approximately one hour. The control points shall be close enough to the deposit such that all significant features may be conveniently and accurately referred to by standard survey methods.
- g) Quarry Walls. All quarry walls, including the quarry outline and all active and inactive benches, shall be shown on the map. Each bench shall be discernable from other benches.
- h) Proposed Operating Areas. All proposed operating areas for the year represented by the Annual Aggregate Source Report shall be clearly designated in some manner. The source is not committed to production from these areas, but shall designate the areas from which they anticipate production. The map may be revised at any time during the year if production is needed from areas that were not previously marked.
- i) Critical Features. The map shall show all relative critical features including, as a minimum, the office, scales, testing laboratory, stockpile areas, processing plant areas, ramps, sumps, and pertinent quarry roads.

**8.2.5** Legend. On the map or on a separate page, the source shall include a legend and map symbols appropriate to the source map.

**8.2.6 Stratigraphic Section.** On a separate page, the source shall include one or more source-specific stratigraphic sections. The stratigraphic sections shall be prepared to an appropriate scale to graphically and descriptively depict the stratigraphic relationships of the various lithologies within the current or anticipated production benches. The addition or deletion of ledges in a bench will require submittal of a new stratigraphic section when the change in the bench ledges occurs. Each stratigraphic section shall include, as a minimum, the following requirements:

- a) The grid coordinates near where the section was measured
- b) The lithologies above and below the current or anticipated production bench
- c) The Department ledge numbers with the thickness, geologic formations and members names, elevation, and date of measurement given for the top of each bench

**8.3 Submittal.** No later than April 1<sup>st</sup> of each year, each source shall submit an Annual Aggregate Source Report to the appropriate District Testing Engineer. Failure to submit an Annual Aggregate Source Report shall result in rejection of the source to provide materials for Department use until such time that an acceptable report has been received by the District Testing Engineer.

Upon receipt of the report, the District Geologist will verify the contents of the report by field review with the Producer's representative designated in 8.2.2. A summary of this review will be forwarded with a signed cover letter from the District Testing Engineer to the Division of Materials and Tests. The District will retain one copy and forward another copy of the report to the Division of Materials and Tests for incorporation into the source QCP Appendix, if applicable. The source shall retain and include the report in their copy of the QCP, if applicable.

IT-530Q Rev. 10/98		<b>INDIANA DEPARTMENT OF TRANSPORTATION</b> <b>DIVISION OF MATERIALS &amp; TESTS</b> <b>LEDGE AND CORE SAMPLE DESCRIPTION</b>				<div style="display: flex; justify-content: space-around;"> <div><div style="border: 1px solid black; width: 20px; height: 20px;"></div></div> <div><div style="border: 1px solid black; width: 20px; height: 20px;"></div></div> <div><div style="border: 1px solid black; width: 20px; height: 20px;"></div></div> </div> Date Sampled	
<div style="border: 1px solid black; width: 20px; height: 20px;"></div> YEAR	<div style="border: 1px solid black; width: 20px; height: 20px;"></div> LAB	<div style="border: 1px solid black; width: 40px; height: 20px;"></div> SUBMITTER	<div style="border: 1px solid black; width: 40px; height: 20px;"></div> SEQUENCE	Sampled By _____			
SOURCE <div style="border: 1px solid black; width: 40px; height: 20px;"></div>		LOCATION _____					

<b>AREA FROM GRID MAP</b>  X COORDINATE _____  Y COORDINATE _____  Section <div style="border: 1px solid black; width: 20px; height: 20px;"></div> Township <div style="border: 1px solid black; width: 20px; height: 20px;"></div> <div style="border: 1px solid black; width: 20px; height: 20px;"></div> N or S  Range <div style="border: 1px solid black; width: 20px; height: 20px;"></div> <div style="border: 1px solid black; width: 20px; height: 20px;"></div> E or W	<b>LEDGE INFORMATION</b>  LEDGE NO. <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 52  LEDGE THICKNESS <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 53 (0.01 m) ELEV.- Top ledge <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 54 (0.01 m) ELEV.- Bottom <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 55
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<b>COLOR</b> SELECT PREDOMINATE GSA COLOR CODE  _____  WET or DRY Surface (Circle one)	<b>STRATIGRAPHY</b>  SYSTEM <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 72 <div style="border: 1px solid black; width: 20px; height: 20px;"></div> 2  FORMATION <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 74 <div style="border: 1px solid black; width: 20px; height: 20px;"></div> 2  MEMBER / BED <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 73 <div style="border: 1px solid black; width: 20px; height: 20px;"></div> 2  COLOR <div style="border: 1px solid black; width: 40px; height: 20px;"></div> 75 <div style="border: 1px solid black; width: 20px; height: 20px;"></div> 2 <div style="border: 1px solid black; width: 20px; height: 20px;"></div> 5
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**BEDDING** (Ingram 1954)  
 ( Check The Three Prominant Characteristics)  

51 ___ Steeply Dipping	66 ___ Discontinuous	
52 ___ Crossbedded	67 ___ Very Thickly Bedded	>1m
56 ___ Contorted	68 ___ Thickly Bedded	30-100 cm
61 ___ Wavy	69 ___ Medium Bedded	10-30 cm
62 ___ Pinch & Swell	70 ___ Thinly Bedded	3-10 cm
63 ___ Irregular	71 ___ Very Thinly Bedded	1-3 cm
64 ___ Lenticular	72 ___ Thickly Laminated	0.3-1 cm
65 ___ Planar	73 ___ Thinly Laminated	< 0.3 cm

**GRAIN SIZE** (Udden-Wentworth)  
 (Select One)  

014 ___ Cobbles	>64 mm	019 ___ Medium	0.5-0.25 mm
015 ___ Pebbles	64-4 mm	020 ___ Fine	0.25-0.125 mm
016 ___ Granular	4.0-2.0 mm	021 ___ Very Fine	0.125-0.0625 mm
017 ___ Very Coarse	2.0-1.0 mm	022 ___ Silt	0.0625-0.0039 mm
018 ___ Coarse	1.0-0.5 mm	023 ___ Clay	< 0.0039 mm

**ROCK TYPE** 77  2  

001 ___ Limestone	004 ___ Chert
002 ___ Dolostone	005 ___ Shale
003 ___ Sandstone	006 ___ Siltstone

**CRYSTAL SIZE** (Folk 1961)  
 (Select One)  

024 ___ Extremely Coarse	>4.0mm
017 ___ Very Coarse	4.0-1.0mm
018 ___ Coarse	1.0-0.25mm
019 ___ Medium	0.25-0.062mm
020 ___ Fine	0.062-0.016mm
021 ___ Very Fine	0.016-0.004mm
025 ___ Aphanocrystalline	<0.004mm

**CRYSTAL SIZE** (Folk 1961)  
 (Select One)  

024 ___ Extremely Coarse	>4.0mm
017 ___ Very Coarse	4.0-1.0mm
018 ___ Coarse	1.0-0.25mm
019 ___ Medium	0.25-0.062mm
020 ___ Fine	0.062-0.016mm
021 ___ Very Fine	0.016-0.004mm
025 ___ Aphanocrystalline	<0.004mm

**LITHOLOGY** (Check The Four Prominant Characteristics)  

01 ___ Cherty	12 ___ Arenaceous	23 ___ Conglomeritic	34 ___ Micritic	44 ___ Siliceous
02 ___ Clay Pocketed	13 ___ Gypsiferous	24 ___ Coralline	35 ___ Mod. Sorted	45 ___ Silty
03 ___ Fossiliferous	14 ___ Glauconitic	25 ___ Crystalline	36 ___ Mottled	46 ___ Skeletal
04 ___ Pelletal	15 ___ Vuggy	26 ___ Dense	37 ___ Pisolitic	47 ___ Sparry
05 ___ Petroliferous	16 ___ Iron Stained	27 ___ Ferriferous	38 ___ Poorly Indurated	48 ___ Stromatolitic
06 ___ Pyritic	17 ___ Styolitic	28 ___ Fissile	39 ___ Poorly Sorted	49 ___ Sucrosic
07 ___ Oolitic	18 ___ Algal	29 ___ Flaggy	40 ___ Porous	50 ___ Variegated
08 ___ Argillaceous	19 ___ Bioclastic	30 ___ Friable	41 ___ Reefal	51 ___ Vitreous
09 ___ Weathered	20 ___ Blebbly	31 ___ Intraclastic	42 ___ Rubbly	52 ___ Well-Indurated
10 ___ Dolomitic	21 ___ Brecciated	32 ___ Lithographic	43 ___ Shaly	53 ___ Well-Sorted
11 ___ Calcareous	22 ___ Carbonaceous	33 ___ Micaceous		

**LITHOLOGY** 85

**BEDDING** 86



386

**EXAMPLE**

**AGGREGATE SOURCE REPORT**

**TITLE SHEET**

<b><u>Producer Name</u></b>	Limerock Quarries Inc.
<b><u>Address</u></b>	P.O. Box 7-11
<b><u>City, State</u></b>	Markle, IN 47025
<b><u>Phone #'s</u></b>	219-328-7025
<b><u>Fax</u></b>	219-244-7025

1999  
Annual Aggregate Source Report

Limerock Quarries Inc.  
Markle Plant

INDOT Source #2799

**Prepared For INDOT**  
Materials & Tests Division  
120 S. Shortridge Rd.  
Indianapolis, IN 46219-0389

**Date Submitted**  
March 1, 1999

<b><u>Source Location:</u></b>	Section 11; T 27 N, R 10 E; 2.3 miles South of Markle, IN. One mile West of SR 3. Wells County; Longitude W 85°21'40", Latitude N 40°48'00"
<b><u>Regional Manager:</u></b>	Clay Mudstone
<b><u>Superintendent:</u></b>	Ferris Ore
<b><u>Quality Control:</u></b>	Richard Quality & Crystal Stone

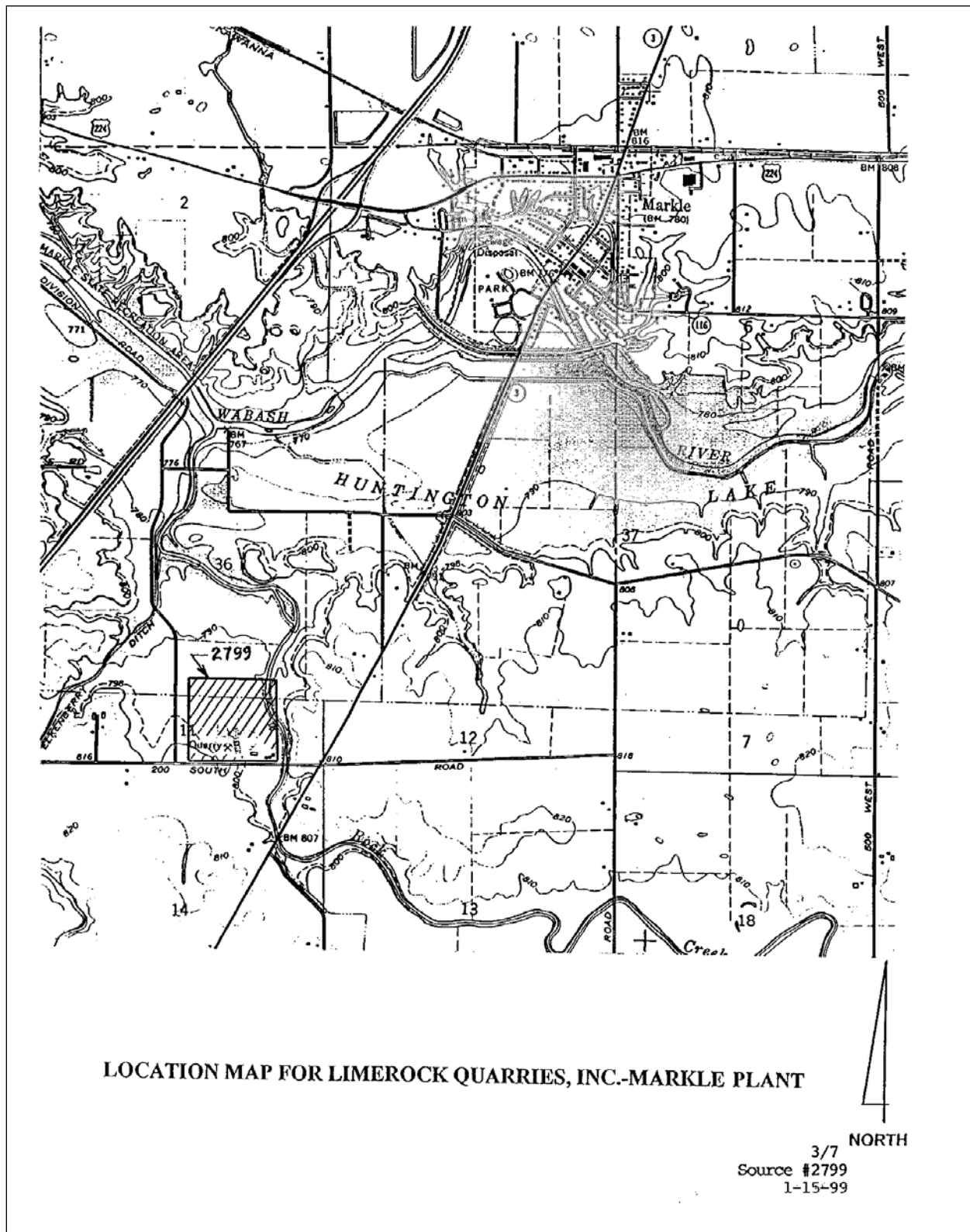
1/7  
Source #2799  
1-15-99

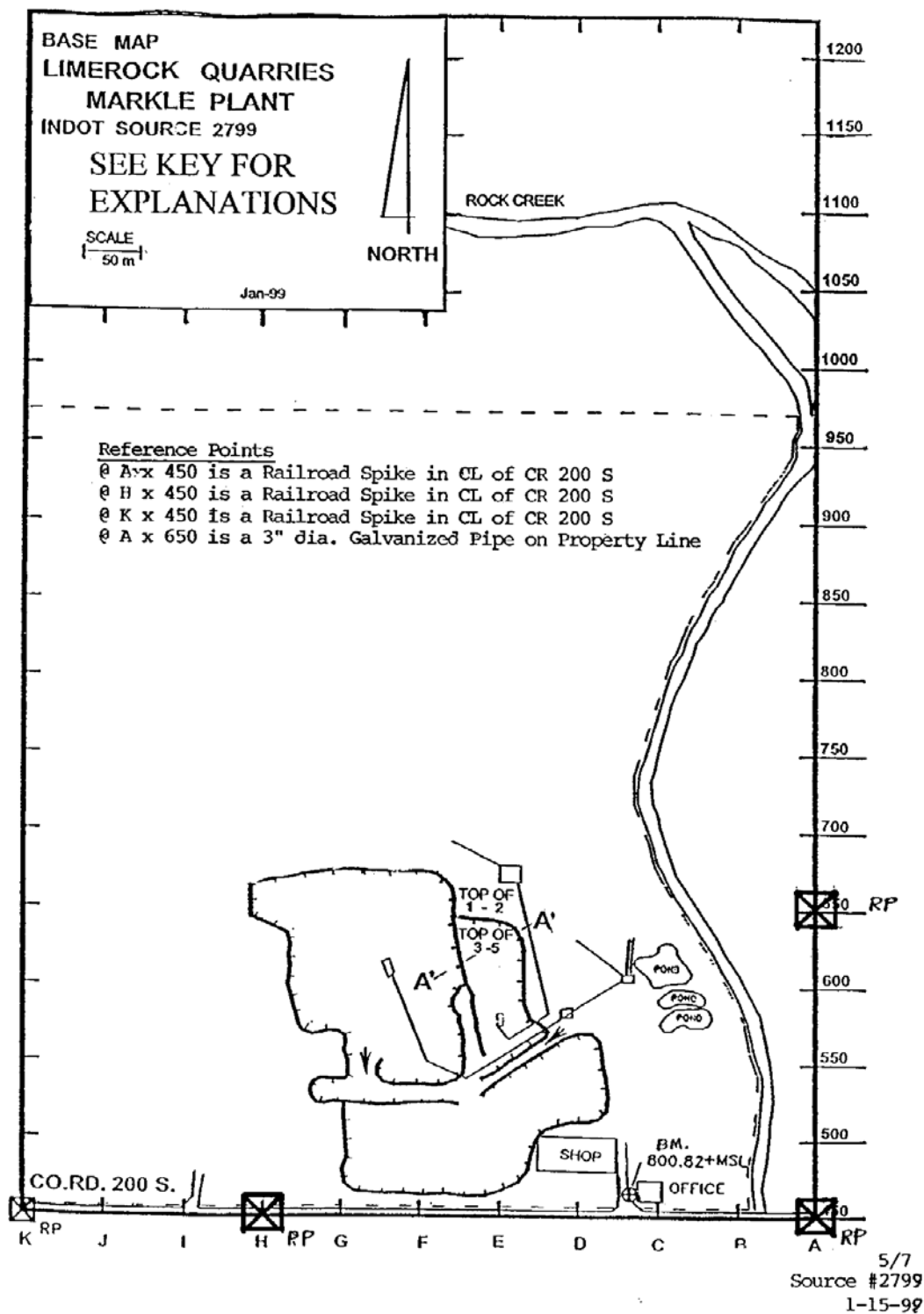
It is the intention of this report to satisfy the needs of Indiana Test Method (ITM) 203-99P, Section 8.0.

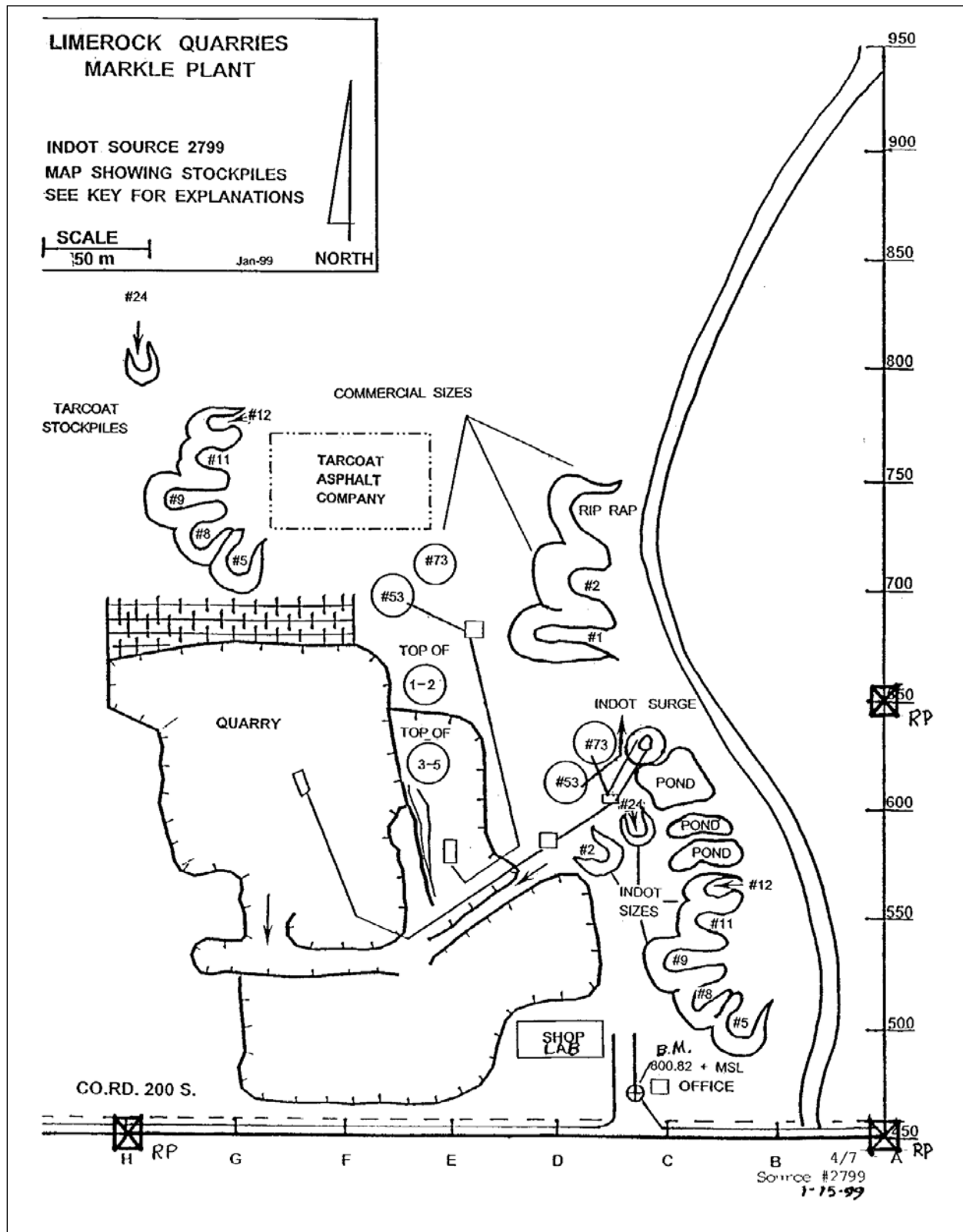
This source currently operates two open-pit benches. The upper bench is mined for commercial purposes only and consists of Ledges 1 and 2. This bench is approximately 60 feet thick. Ledge 3 is Waldron Shale and is wasted. the lower bench (Ledges 4 and 5) is approximately 73 feet thick, is classified as Category IA, and is INDOT Class AP approved up to March 15, 2001. Ledge 5 has a Magnesium content of 10.8 and is Dolomite Approved by INDOT.

Elevations for all benches may be found on the following geologic cross-section. A benchmark with an elevation of 800.82 is located next to the office, which is southeast of the quarry. The proposed 1999 operating areas may be found on the following Source Map. Also included is the location map and quarry map indicating the stockpile areas. The symbol Legend and a Stratigraphic Description for this source are shown on the last two pages.

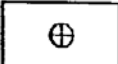

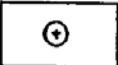


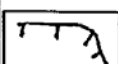
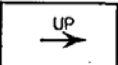

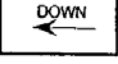
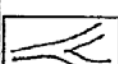
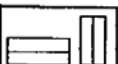
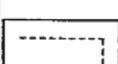

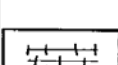
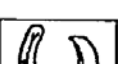
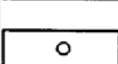

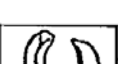
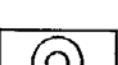
2/7  
Source #2799  
1-15-99







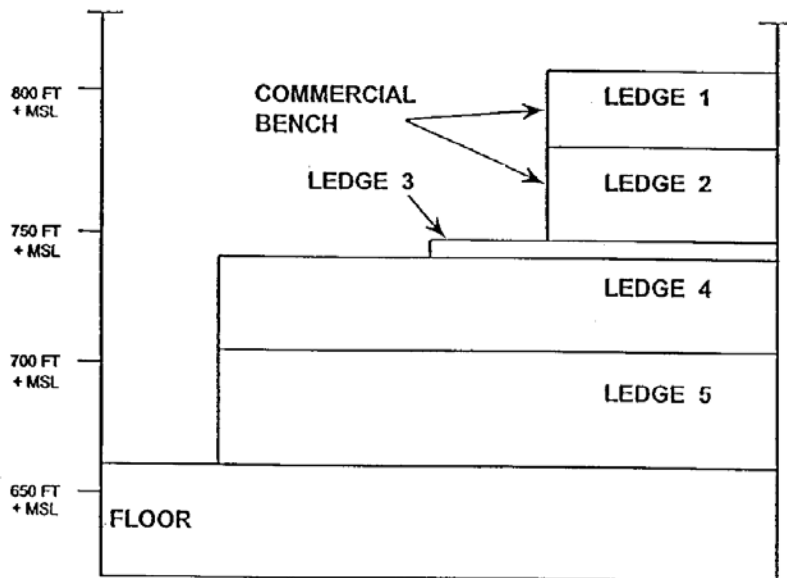
# KEY TO QUARRY MAP FOR INDOT LIMEROCK QUARRIES, INC. MARKLE PLANT # 2799

SYMBOL	EXPLANATION	SYMBOL	EXPLANATION
	PERMANENT BENCHMARK		TAILINGS / FILL
	TEMPORARY BENCHMARK		WATER BODIES
	REFERENCE POINT		QUARRY WALLS HANCHURES ON LOWER SIDE
	RAMP ARROW - UP		PROPOSED QUARRY AREA FOR CURRENT YEAR
	RAMP ARROW - DOWN		ROADS
	BUILDINGS		PROPERTY LINES
	PLANT STRUCTURES		PROPOSED OPERATING AREA FOR CURRENT YEAR
	INDOT STOCKPILES		CORE HOLE
	COMMERCIAL STOCKPILES		
	MISC. STOCKPILES		
	SURGEPILES		

6/7  
Source 2799  
1-15-99

# STRATIGRAPHIC DESCRIPTION FOR LIMEROCK QUARRIES, INC. MARKLE PLANT #2799

**CORE LOCATIONS**  
**F - 10 x 600 & E x 650**



LEDGE	THICKNESS	GEOLOGIC FORMATION
1	20'	LISTON CREEK
2	40'	MISSISSINEWA
3	5'	WALDRON
4	35'	LOUISVILLE
5	38'	LIMBERLOST

## **MARKLE PLANT - # 2799**

\* LEDGES 4 & 5 ARE AP APPROVED ; EXPIRES 12/12/99

\* THIS SOURCE HAS AN INDOT IA CLASSIFICATION

\* LEDGE #5 IS APPROVED DOLOMITE ( 10.8 Mg.)

7/7  
Source #2799  
1-15-99



**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**ACCEPTANCE PROCEDURES FOR DOLOMITE AGGREGATES  
ITM No. 205-17T**

**1.0 SCOPE.**

- 1.1** This method sets forth the acceptance procedures to be used when Aggregate Producers request that dolomite aggregates be evaluated for use in HMA surface mixtures. Dolomite aggregates are specified for use under certain ESAL loading conditions to obtain skid-resistant HMA surface courses.

These procedures cover the rapid instrumental chemical analysis and the referee chemical analysis of dolomite for elemental magnesium content.

- 1.2** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of this test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- M 92 Wire-Cloth Sieves for Testing Purposes
- M 231 Weighing Devices Used in the Testing of Materials
- T 248 Reducing Samples of Aggregate to Testing Size

**2.2 ASTM Standards.**

- C 25 Chemical Analysis of Limestone, Quicklime, and Hydrated Lime
- C 1271 X-Ray Spectrometric Analysis of Lime and Limestone
- C 1301 Major and Trace Elements in Limestone and Lime by Inductively Coupled Plasma-Atomic Emission Spectroscopy (ICP) and Atomic Absorption (AA)
- D 2698 Determination of Pigment Content of Solvent-Reducible Paints by High Speed Centrifuging

**2.3 ITM Standards.**

- 207 Sampling Stockpiled Aggregates
- 571 Quantitative Extraction of Asphalt/Binder and Gradation of Extracted Aggregate from HMA Mixtures

- 3.0 TERMINOLOGY.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specifications, Section 101.
- 4.0 SIGNIFICANCE AND USE.** This ITM will be used to evaluate dolomite aggregates for use in HMA surface mixtures.
- 5.0 APPARATUS.**
- 5.1** Instrumentation in accordance with ASTM C 25, C 1271, or C 1301
  - 5.2** Balance, Class A, in accordance with AASHTO M 231
  - 5.3** High-speed centrifuge in accordance with ASTM D 2698
  - 5.4** Sieves, in accordance with AASHTO M 92
- 6.0 GENERAL REQUIREMENTS.**
- 6.1** Each Aggregate Producer requesting to have a coarse aggregate tested in accordance with this procedure shall contact the appropriate District Testing Engineer to initiate the approval process.
  - 6.2** Testing will be conducted by the Office of Materials Management.
  - 6.3** Approval of the source as a dolomitic material will be based on results from either the rapid instrumental analysis or the referee analysis tests.
- 7.0 SAMPLING.**
- 7.1** Sampling of aggregates will be done in accordance with ITM 207.
  - 7.2** Each sample will consist of 10 to 15 lbm of material.
  - 7.3 Source Sampling - Coarse Aggregate.**
    - 7.3.1** Coarse Aggregate will be sampled by the District for initial evaluation. Three samples representing materials produced from each proposed dolomitic production ledge or area will be obtained.
    - 7.3.2** Each sample will be taken from a separate stockpile of at least 1000 t (1000 Mg) of any gradation material being produced.

**7.4 Sampling.** All dolomite aggregates will be sampled for acceptance at the HMA plant, either before or preferably during mix production.

**7.5 Sample Submittal.**

**7.5.1** Samples will be submitted to the Office of Materials Management for testing.

**7.5.2** The submittal report will indicate the source and the ledges or area represented.

**8.0 PREPARATION OF TEST SAMPLE.**

**8.1 Aggregate Samples.**

**8.1.1** Split at least 1000 g of material in accordance with AASHTO T 248.

**8.1.2** Crush the sample so that all of the material passes the No. 4 sieve.

**8.1.3** Split at least 50 g of the minus No. 4 material in accordance with AASHTO T 248.

**8.1.4** Pulverize the minus No. 4 material so that the sample will pass the No. 60 sieve.

**8.2 HMA Samples.**

**8.2.1** Extract the binder from the sample in accordance with a procedure in ITM 571.

**8.2.2** Prepare the sample in accordance with 8.1.

**8.2.3** To extract the binder from the sample passing the No. 60 sieve, weigh 10 g into a 50 mL centrifuge tube and add 25 mL Toluene. Stir the solution with a stirring rod, wash the residue from the stirring rod into the centrifuge tube, and centrifuge the material at 10000 rpm in a highspeed centrifuge for 20 minutes. Decant the liquid portion and repeat this procedure three more times using acetone for the last extraction. Dry the sample to a constant weight at 220°F.

## **9.0 PROCEDURE.**

**9.1 Referee Analysis.** Determine the elemental magnesium content by EDTA titration in accordance with ASTM C 25, C 1271, or C 1301.

**9.2 Rapid Instrumental Analysis.** Determine the elemental magnesium content using instrumentation in accordance with the performance requirements for alternative test methods of ASTM C 25.

**10.0 CALCULATIONS.** Calculations will be made to convert values reported in accordance with ASTM C 25 to elemental calcium and elemental magnesium contents.

## **11.0 ACCEPTANCE CRITERIA.**

**11.1** If the elemental magnesium content of the aggregate is 10.3 percent or greater, the material will be approved as a dolomitic aggregate.

**11.2** All samples that do not have a value of at least 10.3 percent elemental magnesium content determined by the rapid instrumental analysis will be verified by determining the elemental magnesium content using the referee analysis method.

**11.3** The Department will maintain a list of Approved Dolomite Sources.

## **12.0 REPORT.**

**12.1** All elemental calcium and magnesium content values will be reported to the nearest 0.1 percent.

**12.2** When the rapid instrumental analysis is used, the elemental magnesium content will be reported.

**12.3** When the referee analysis is used, the elemental calcium and elemental magnesium contents will be reported.

### **13.0 PRECISION.**

- 13.1** The within-laboratory standard deviation for elemental calcium is 0.21 percent, and for elemental magnesium content is 0.13 percent.
- 13.2** The between-laboratory standard deviation for elemental calcium is 0.22 percent, and for elemental magnesium content is 0.17 percent.

**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**SCRATCH HARDNESS OF COARSE AGGREGATE PARTICLES  
ITM No. 206-15T**

**1.0 SCOPE.**

- 1.1** This test method covers determining the quantity of soft particles in gravel coarse aggregates on the basis of scratch hardness.
- 1.2** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- T 27 Sieve Analysis of Fine and Coarse Aggregates  
M 92 Wire-Cloth Sieves for Testing Purposes  
M 231 Weighing Devices Used in the Testing of Materials

**2.2 ITM Standards.**

- 207 Sampling Stockpiled Aggregates

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specifications, Section 101.

**4.0 SIGNIFICANCE AND USE.**

- 4.1** This ITM is used to identify materials that are soft, including those which are so poorly bonded that the separate particles in the piece are easily detached from the mass. The test method is not intended to identify other types of deleterious materials in aggregates.

- 4.2** In case of questions, the scratch hardness test should be made on a freshly broken surface of the aggregate particle. If the particle contains more than one type of rock and is partly hard and partly soft, the particle should be classified as soft only if the soft portion is one third or more of the volume of the particle. Scratch hardness tests may be made on the exposed surface of a particle provided consideration is given to softening of the surface due to weathering. A particle with a thin, soft and weathered surface and a hard core should normally be classed as soft.

## **5.0 APPARATUS.**

- 5.1** Sieves, in accordance with AASHTO M 92
- 5.2** Balance, Class G2, in accordance with AASHTO M 231
- 5.3** Pans, as needed
- 5.4** Brass Rod, 1/16 in. (1.6 mm) in diameter, with a rounded point, mounted in a device so that a load of  $2 \pm 0.1$  lbf ( $8.9 \pm 0.4$  N) is applied to the specimen tested. The brass rod shall be of suitable hardness so that when filed to a sharp point, the brass rod will scratch a U.S. Lincoln design copper penny but fail to scratch a U.S. Jefferson design nickel.

- 6.0 SAMPLES.** Sampling shall be done in accordance with ITM 207.

- 7.0 PREPARATION OF TEST SPECIMEN.** Aggregate for the test shall consist of the material retained on the 3/8 in. and larger sieves following completion of AASHTO T 27.

- 8.0 PROCEDURE.** Subject each particle of aggregate under test to a scratching motion of the brass rod, using a pressure of 2 lbf. Particles are considered to be soft if, during the scratching process, a groove is made in them without deposition of metal from the brass rod, or if separate particles are detached from the rock mass. For some sandstones, brass fragments may be deposited on hard individual grains, while at the same time separate particles are detached from the mass due to a weak binding medium. Such particles are to be considered as soft.

- 9.0 CALCULATIONS.** Particles determined to be soft are combined with structurally weak particles to determine the amount of non-durable material. The weight (mass) of non-durable material is calculated by:

$$\text{Non - Durable, \%} = \frac{\text{Weight (Mass) of Non - Durable above 3/8 in. (9.5 mm) Sieve}}{\text{Weight (Mass) of Sample above 3/8 in. (9.5 mm) Sieve}} \times 100$$

- 10.0 REPORT.** Non-durable particles are reported to the nearest 0.1 percent. A brief lithologic description of the soft particles shall also be included.



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**SAMPLING STOCKPILED AGGREGATES  
ITM No. 207-23**

**1.0 SCOPE.**

- 1.1** This test method covers sampling fine and coarse aggregate stockpiles.
- 1.2** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 SIGNIFICANCE AND USE.** This ITM provides guidance on how to obtain aggregate samples from aggregate stockpiles for control of production at the source or control of the materials at the point of use.

**3.0 References**

**3.1 AASHTO Standards**

**R90 Standard Practice for Aggregate Products**

**4.0 TERMINOLOGY.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specifications, Section 101.

**5.0 APPARATUS.**

- 5.1** Square bit shovel
- 5.2** Fire shovel
- 5.3** Sampling tube, 3 in. minimum in diameter and 3 ft minimum in length

**6.0 SAMPLING.**

**6.1 Coarse Aggregate Sampling.**

- 6.1.1** Using a front-end loader, dig into the stockpile and set aside a small pile of 10 to 15 t of material. This procedure shall be done in the same manner as if a truck is being loaded for shipment (Figures 1 and 2). When forming the small pile, the loader bucket shall be as low as possible and roll the material from the bucket rather than dumping the material. Reducing the distance



the material is allowed to free-fall will reduce the amount of segregation that may occur in the small pile (Figure 3). Each additional bucket load of material shall be obtained and dumped in the same manner as set out above and shall be placed uniformly over the preceding one (Figure 4).

- 6.1.2** Thoroughly mix the small pile. Using the loader bucket, proceed to the end of the oblong pile and roll the material over. Keeping the loader bucket as low as possible, push the bucket into the material until the front of the bucket is past the midpoint of the original pile. The loader bucket shall then be slowly raised and rolled forward thus producing a smooth mixing of the material (Figures 5, 6, and 7). Proceed to the opposite end of the pile and repeat this mixing procedure. If the pile does not appear to be uniform, additional mixing shall be done.
- 6.1.3** The pile is now ready for sampling. Do not strike off the top of the stockpile (Figure 8). The sample shall be taken at the center of the volume which is approximately one-third of the height of the pile. The sample shall consist of not less than six full shovels of material taken at equal increments around the pile (Figures 9, 10, and 11). A square bit shovel shall be used. The size of the shovel shall be such that the sample meets the minimum weight (mass) requirements of the test conducted on the sample. The shovel shall be inserted full-depth horizontally into the material and raised vertically. Care shall be taken to retain as much of the material as possible on the blade of the shovel (Figure 12).

## **6.2 Fine Aggregate Sampling.**

- 6.2.1** Fine aggregate samples are normally obtained as set out above for coarse aggregate, except a fire shovel or sampling tube shall be used.
- 6.2.2** When fine aggregate stockpiles are constructed so as to not exceed height of the sampler, and when segregation is not apparent, the samples may be taken directly from the face of the large stockpile. The surface crust of the stockpile is required to be removed from the sampling area.

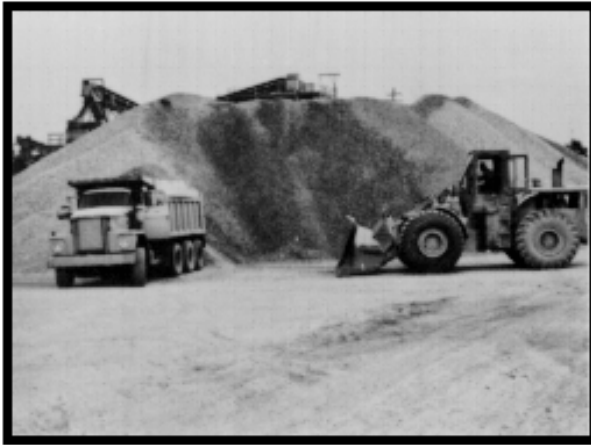
## **6.3 Coarse and Fine Aggregate Strike-off Acceptance.**

- 6.3.1** Follow 6.1.1 and 6.1.2 Stockpile Rolling Procedures to create the sample pile.
  - 6.3.1.1** The front-end loader shall reach across the small portion of the pile with the front-end loader bucket to drag the material across to create a flat surface not less than 1 foot thick per AASHTO R90. The loader operator shall not drive up onto the sample pile at any time.
  - 6.3.1.2** Sample in an X formation on the entire flat surface. The sample shall still have a minimum of six shovelfuls of material. A square bit shovel or fire shovel shall be used.

**6.3.1.3** Identify in the Source Quality Control Plan which specific products the Strike-off method is to be used.

## **7.0 SAFETY.**

- 7.1** Samples shall not be obtained by climbing onto stockpiles due to the hazard of burial and suffocation from unstable stockpiles of unconsolidated materials. Also, over-steepened stockpiles that may sluff and engulf personnel in the immediate area should be avoided.
- 7.2** Personnel requiring additional information concerning specific sampling situations are directed to contact the appropriate District Testing Engineer.



**Figure 1**



**Figure 2**



**Figure 3**



**Figure 4**



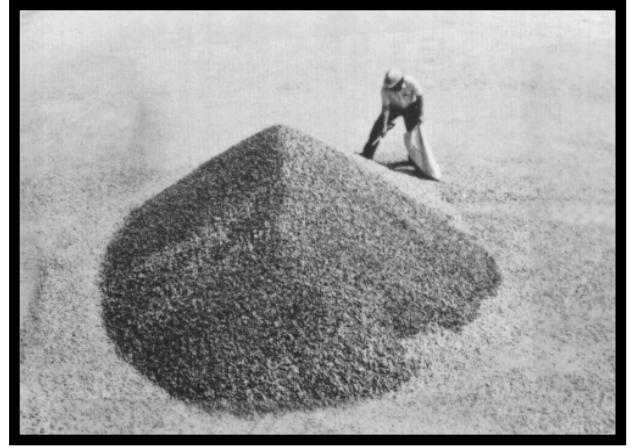
**Figure 5**



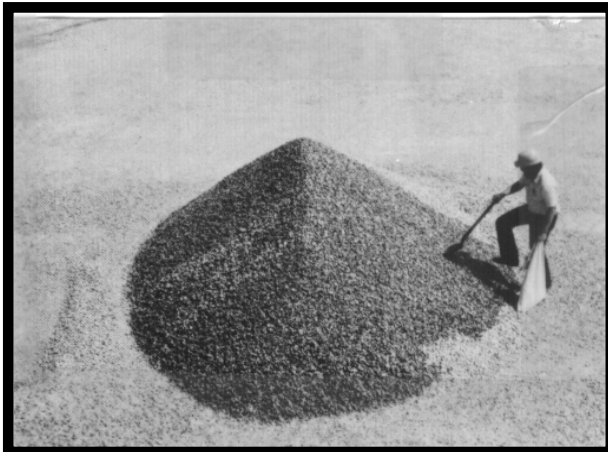
**Figure 6**



**Figure 7**



**Figure 8**



**Figure 9**



**Figure 10**



**Figure 11**



**Figure 12**

**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**SOUNDNESS OF AGGREGATES BY FREEZING AND  
THAWING IN A BRINE SOLUTION  
ITM No. 209-15T**

**1.0 SCOPE.**

- 1.1** This method of test covers the determination of the resistance of fine or coarse aggregates to disintegration by rapidly repeated cycles of freezing and thawing in the presence of a solution of water and sodium chloride. This test method provides information used in determining the soundness of fine or coarse aggregates subjected to weathering, particularly when adequate information is not available from service records of the behavior of the aggregates.
- 1.2** Coarse aggregate ledge, production, and point-of-use samples and all fine aggregate production and point-of-use samples are tested using this procedure.
- 1.3** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- M 92 Wire-Cloth Sieves for Testing Purposes
- M 231 Weighing Devices Used in the Testing of Materials

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM is used to verify the minimum specification requirements for brine freeze-and-thaw soundness for aggregates.

**5.0 APPARATUS.**

- 5.1** Freezing-and-thawing apparatus, consisting of a suitable environmental chamber, refrigeration and heating equipment, and controls. The apparatus shall produce continuous, automatic, and reproducible cycles at a low temperature freezing range not higher than -15°F at any point in the environmental freezing chamber and a high temperature thawing range between 70°F and 75°F at any point in the environmental thaw water zone of the environmental chamber. In the event that the equipment does not operate automatically, provisions shall be made for either continual manual operation on a 24 h a day basis or for the storage of all specimens in a frozen condition when the equipment is not in operation. In the event that a capacity load of test specimens is not available, provisions shall be made to occupy empty spaces.
- 5.2** Sample containers, plastic, rubber, or other materials suitable for the procedure to be followed to insure water tightness
- 5.3** Sieves, in accordance with AASHTO M 92
- 5.4** Balance, in accordance with AASHTO M 231 for the class of general purpose balance required for the sample weight being tested
- 5.5** Oven, appropriate size capable of providing a free circulation of air through the oven, and maintaining a temperature of  $230 \pm 9^{\circ}\text{F}$

**6.0 REAGENTS.** Sodium chloride solution with three percent of sodium chloride by weight

- 7.0 FREEZING-AND-THAWING CYCLE.** The nominal freezing-and-thawing cycle shall consist of alternately lowering the sample temperature from between 70°F and 75°F to below -15 °F and raising the temperature from below -15 °F to between 70°F and 75°F. At the end of the cooling period, the sample temperature shall be below -15°F and shall have been below this temperature for at least 15 minutes. At the end of the heating period the sample temperature shall be between 70°F and 75°F and shall have been maintained in this range at least 15 minutes.

## 8.0 SAMPLES.

- 8.1** Fine aggregate shall consist of material passing a 3/8 in. sieve. The sample shall yield not less than 100 g of each of the following sizes, which shall be available in amounts of 5 percent or more, expressed in terms of the following sieves:

Passing Sieve	Retained on Sieve
3/8 in.	No 4
No. 4	No. 8
No. 8	No. 16
No. 16	No. 30
No 30	No 50

**Table 1**

- 8.2** Coarse aggregate shall consist of material retained on the No. 4 sieve and all sieves above the No. 4 sieve. The sample shall yield not less than the following amounts of the different sieve sizes, which shall be available in amounts of 5 percent or more.

Size	
No. 4 - 3/8 in.	300 g
3/8 - 3/4 in. consisting of:	1000 g
3/8 - 1/2 in. material	33 percent
1/2 - 3/4 in. material	67 percent
3/4 - 1 1/2 in. consisting of:	1500 g
3/4 - 1 in. material	33 percent
1 - 1/2 in. material	67 percent
1 1/2 - 2 1/2 in. consisting of:	3000 g
1 1/2 - 2 in. material	50 percent
2 - 2 1/2 in. material	50 percent
Larger sizes by 1 in. spread in sieve size each fraction	3000 g

**Table 2**

- 8.3** If the sample contains less than 5 percent of the sizes specified in 8.1 or 8.2, that size shall not be tested. For the purpose of calculating the test results, the sample shall be considered to have the same loss in testing as the average of the next smaller and the next larger size. If one of these sizes is absent, the sample shall be considered to have the same loss as the next larger or next smaller size, whichever is present. When the 3/8 to 3/4 in., 3/4 to 1 1/2 in. or 1 1/2 to 2 1/2 in. test samples specified in 8.2 cannot be prepared due to absence of one of the two sizes of aggregate shown for each, the size available shall be used to prepare the sample tested.
- 8.4** When an aggregate to be tested contains appreciable amounts of both fine and coarse material having a grading with more than 10 percent coarser than the 3/8 in. sieve and more than 10 percent finer than the No. 4 sieve, separate samples of the minus No. 4 fraction and the plus No. 4 fraction shall be tested in accordance with the procedures for fine aggregate and coarse aggregate, respectively. The results shall be reported separately for the fine aggregate fraction and the coarse aggregate fraction, with the percentages of the coarse and fine size fractions in the initial grading stated.

## **9.0 PREPARATION OF TEST SPECIMEN.**

- 9.1** The sample of fine aggregate shall be washed on a No. 100 sieve, dried to constant weight at a temperature of  $230 \pm 9^{\circ}\text{F}$ , and separated into the different sizes by sieving, as follows.
- 9.2** A rough separation of the graded sample shall be made by means of a nest of the sieves specified in 8.1. From the fractions obtained in this manner, samples of sufficient size shall be selected to yield 100 g after sieving (In general, one 100 g sample will be sufficient). Fine aggregate sticking in the meshes of the sieves shall not be used in preparing the samples. Samples consisting of 100 g shall be weighed from each of the separated fractions after final sieving and placed in separate containers for the test.
- 9.3** The sample of coarse aggregate shall be thoroughly washed and dried to constant weight at a temperature of  $230 \pm 9^{\circ}\text{F}$  and shall be separated into the different sizes shown in 8.2 by sieving. The proper weight of sample for each fraction shall be determined and placed in separate containers for the test. For aggregate coarser than the 3/4 in. sieve, the number of particles shall be counted.
- 9.4** **Ledge Rock.** For testing ledge rock, the sample shall be prepared by breaking the material into fragments reasonably uniform in size and shape and having a weight of approximately 100 g each. The test sample shall have a weight of  $5000 \text{ g} \pm 2$  percent. The sample shall be thoroughly washed and dried prior to the test as described in 9.3.



## 10.0 PROCEDURE.

**10.1** Each of the fractions specified in 8.1 and 8.2 shall be completely immersed in separate leak proof sample containers containing a 3 percent solution of sodium chloride (NaCl) for a period of 24 h prior to the start of the freezing cycle and shall be frozen and thawed in this completely immersed condition.

**10.2** Alternate freezing and thawing shall be repeated until 25 cycles are obtained.

**10.3** After the completion of the final cycle, each sample shall be dried to constant weight at  $230 \pm 9^{\circ}\text{F}$ , weighed and, except in the case of ledge rock, sieved over the sieve indicated for the appropriate size of aggregate in Table 3. The material retained on the sieve shall be weighed and the weight recorded.

**10.3.1** Samples of fine aggregate shall be sieved over the original retaining sieve to determine the loss. The material retained on each sieve shall be weighed and the weight recorded.

**10.3.2** For ledge rock, the loss in weight shall be determined by subtracting from the original weight of the sample the final weight of all fragments which have not broken into three or more pieces.

Size of Aggregate	Sieve Used to Determine Loss
2 1/2 to 1 1/2 in.	1 1/4 in.
1 1/2 to 3/4 in.	5/8 in.
3/4 to 3/8 in.	5/16 in.

**Table 3**

## 11.0 REPORT.

**11.1** The report shall include the following data:

**11.1.1** Weight of each fraction of each sample before testing.

**11.1.2** Except in the case of ledge rock, the actual loss of each fraction of the sample expressed as a percentage of the original weight of the fraction.

**11.1.3** Weighted average calculated from the percentage of loss for each fraction, based on the average grading of the material from that portion of the material of which the sample is representative. In these calculations, sizes finer than the No. 50 sieve shall be assumed to have 0 percent loss.

- 11.1.4** For an aggregate containing appreciable amounts of both fine and coarse material tested as two separate samples as required in 8.4, the weighted average losses shall be calculated separately for the minus No. 4 and plus No. 4 fractions based on recomputed gradings considering the fine fraction as 100 percent and the coarse fraction as 100 percent. The results shall be reported separately with the percentage of the minus No. 4 and plus No. 4 material in the initial grading stated.
- 11.1.5** For ledge rock, the loss as determined in 10.2.2 shall be reported as a percentage of the original weight.
- 11.1.6** If considered beneficial or when requested, the type of failure of the discrete particles in the sample shall be reported.



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**CLASS AP COARSE AGGREGATE  
ITM No. 210-23**

**1.0 SCOPE**

- 1.1** This method sets forth the control procedures used for classification of an aggregate as Class AP.
- 1.2** The coarse aggregate is incorporated into the casting of concrete beams, and the beams are cured and tested in a freeze and thaw cycling procedure. The length of the concrete beams is measured before and after the test to determine the average maximum change in length.
- 1.3** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 GENERAL REQUIREMENTS.**

- 2.1** No testing of the aggregate will be made until the material is rated Class A aggregate; however, the material may be tested for class A and AP concurrently if so directed by the Department. Blending or combining of a ledge that does not meet quality or deleterious requirements will be permitted only by the approval of the Department.
- 2.2** The coarse aggregate Producer shall provide a written description of the production control in the Source Quality Control Plan in accordance with ITM 211. This plan shall specify the ledges to be incorporated into the production for crushed stone, the relative production zone within the pit for gravel, general handling and crushing procedures used in the production, the final production gradation obtained, and any other pertinent information relative to the coarse aggregate production, such as stockpile signage. Any unauthorized change in the approved Quality Control Plan will be cause for the suspension of shipment of this material.
- 2.3** Tests will be conducted by the Division of Materials and Tests, or a Department approved AASHTO Accredited Laboratory until Department tests are available. Department tests results will control the re-sampling schedule.

### **3.0 REFERENCES.**

#### **3.1 AASHTO Standards.**

M 85	Portland Cement
M 92	Wire-Cloth Sieves for Testing Purposes
M 154	Air-Entraining Admixtures for Concrete
M 210	Use of Apparatus for the Determination of Length Change of Hardened Cement Paste, Mortar, and Concrete
M 231	Weighing Devices Used in the Testing of Materials
T 27	Sieve Analysis of Fine and Coarse Aggregates
T 119	Slump of Hydraulic Cement Concrete
T 121	Mass Per Cubic Meter (Cubic Foot), Yield, and Air Content (Gravimetric) of Concrete
T 152	Air Content of Freshly Mixed Concrete by the Pressure Method
T 161	Resistance of Concrete to Rapid Freezing and Thawing
T 196	Air Content of Freshly Mixed Concrete by the Volumetric Method

#### **3.2 ASTM Standards.**

C192	Standard Practice for Making and Curing Concrete Test Specimens in the Laboratory
C666	Standard Test Method for Resistance of Concrete to Rapid Freezing and Thawing

#### **3.3 ITM Standards.**

203	Control Procedures for Classification of Aggregates
207	Sampling Stockpiled Aggregates
211	Certified Aggregate Producer Program

**4.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

**5.0 SIGNIFICANCE AND USE.** This ITM shall be used to classify aggregates as Class AP for use as designated in the Standard Specifications.

### **6.0 APPARATUS.**

- 6.1** Balance, G2, in accordance with AASHTO M 231
- 6.2** Beam molds, in accordance with AASHTO T 126, except for the dimensional requirements
- 6.3** Freezing and Thawing Apparatus, in accordance with AASHTO T 161

- 6.4 Length Comparator, in accordance with AASHTO M 210
  - 6.5 Mechanical Sieve Shaker, in accordance with AASHTO T 27
  - 6.6 Oven, appropriate size capable of maintaining a uniform temperature of  $230 \pm 9^{\circ}\text{F}$
  - 6.7 Sieves, in accordance with AASHTO M 92
  - 6.8 Miscellaneous equipment, such as tamping rods, scoops, trowels, straightedge, mixing bowl, sieve brush, etc.
- 7.0 **SAMPLING.** An approximate 300 lb coarse aggregate sample of the material to be tested will be obtained in accordance with ITM 207.
- 8.0 **PREPARATION OF TEST SPECIMEN.**

- 8.1 **Coarse Aggregate.** The sample shall be separated into the required sieve sizes in accordance with AASHTO T 27. Depending on the maximum particle size of the material, the quantity from each sieve size shall be recombined to obtain the following gradation:

Maximum particle size of 1-1/2 inch.

Sieve Size	Percent Passing
1-1/2 in.	100
1 in.	95
3/4 in.	70
1/2 in.	45
3/8 in.	20
No. 4	0

Maximum particle size of 1 inch.

Sieve Size	Percent Passing
1 in.	100
3/4 in.	95
1/2 in.	55
3/8 in.	35
No. 4	0

The sample shall be submerged in water for at least 24 h prior to mixing for beam casting and shall be SSD at the time of mixing.

- 8.2 **Fine Aggregate.** The fine aggregate sample will be No. 23 natural sand from Source No. 2310. The sample will be separated into the required sieve sizes in

accordance with AASHTO T 27. The quantity from each sieve size will be recombined to obtain the following gradation:

Sieve Size	Percent Passing
No. 4	100
No. 8	87
No. 16	67
No. 30	42
No. 50	9
No. 100	0

**8.3 Cement.** The cement will be Type **IL** cement in accordance with AASHTO M240 from Source No. **W028369**.

**8.4 Air Entraining Admixtures.** The air entraining admixture shall be **a Vinsol resin-based product and** in accordance with AASHTO M 154.

**8.5 Mix Design Parameters.** The concrete shall have the following properties:

Portland Cement Content	564 lb/yd <sup>3</sup>
Water/Cement Ratio (Weight Basis)	0.43
Air Content	6.5 ± 1.5%
Slump	1 in. to 3 in.
Absolute Volume of Coarse Aggregate (Saturated Surface Dry)	0.40

**8.5.1** The air content shall be determined in accordance with AASHTO T 152 or AASHTO T 196.

**8.5.2** The unit weight shall be determined in accordance with AASHTO T 121.

**8.5.3** The slump shall be determined in accordance with AASHTO T 119.

**8.6 Casting of Test Beams.** The beams shall be cast in accordance with ASTM C192 and the beam dimensions shall be 3 in. by 4 in. by 15 in. (depth x width x length). Three or five beams shall be cast depending on the source of aggregates as determined by the Department. Gage studs shall be cast into both ends of the beams. The gage studs shall be stainless steel hex bolts 1/4 in. - 20 UNC 1 in., with the threaded end finished in accordance with AASHTO M 210.

**8.7 Curing of Cast Beams.** As soon as the concrete surface is capable of supporting a curing material, the beams will be covered with two layers of wet burlap (or similar material) and one layer of at least 4 mil thick plastic sheeting. After 24 h, the beams will be de-molded and placed in submerged curing conditions in accordance with ASTM C192. At an age of 14 days the freeze-and-thaw exposure will begin.

## 9.0 PROCEDURE.

- 9.1 Immediately after the curing period, bring the beams to a temperature of  $40 \pm 3^{\circ}\text{F}$  in water.
- 9.2 Measure the length of the beams from stud tip to stud tip using a length comparator in accordance with AASHTO M 210.
- 9.3 Apply freeze-and-thaw cycling in accordance with ASTM C666, Procedure B. The freeze-and-thaw unit will be adjusted to achieve 8 cycles per day (approximately 3 h per cycle).
- 9.4 Measure the length of the beams at least every 50 cycles throughout the testing at  $40 \pm 3^{\circ}\text{F}$ . Check the gage studs to assure that no movement of the studs has occurred.
- 9.5 Continue the test until at least two beams break or until at least 350 cycles are achieved. When five beams have been cast, continue the test until at least three beams break or until at least 350 cycles are achieved.

## 10.0 CALCULATIONS.

- 10.1 Calculate the length change in percent as follows:

$$L_c = \frac{l_2 - l_1}{L_g} \times 100$$

where:

$L_c$  = length change of the test beam after C cycles of freezing and thawing,  
%

$l_1$  = length comparator reading of beam at 0 cycles, in.

$l_2$  = length compactor reading of beam at C cycles, in.

$L_g$  = effective gage length between the innermost ends of the gage studs

## 11.0 REPORT.

- 11.1 Test Report Data will include the following items:

- 11.1.1 Concrete batch weights

- 11.1.2 Coarse aggregate source identification

- 11.1.3 Type of material

- 11.1.4 Gradation of production material

- 11.1.5 Ledges of aggregate, if applicable
- 11.1.6 Date sampled
- 11.1.7 Individual(s) obtaining sample
- 11.1.8 Number of beams cast
- 11.1.9 Plastic concrete parameters (air content, unit weight, and slump)
- 11.1.10 Duration of freeze-and-thaw cycle used
- 11.1.11 Date of test completion
- 11.1.12 Initial length of each beam, length of each beam after C cycles, length change of each beam, and average length change for the beams. Length change values for each beam and the average of all beams will be reported to the nearest 0.001 percent.
- 11.1.13 Plot of percent length change versus number of cycles for each specimen
- 11.1.14 Brief description of any specimen which failed during the test including any distress observed from the beams during the testing
- 11.1.15 Retest date

## **12.0 AGGREGATE ACCEPTANCE OR REJECTION CRITERIA.**

### **12.1 Aggregate Acceptance Criteria.**

- 12.1.1 Department Acceptance Criteria. The average of all of the beams tested shall be less than .060 percent expansion. When three beams are cast, two of the three beams shall be less than .060 percent expansion. When five beams are cast, three of the five beams shall be less than .060 percent expansion. Broken beams shall be excluded to a maximum of one broken beam per test beam set when three beams are cast and a maximum of two broken beams per test beam set when five beams are cast.
- 12.1.2 Approved Laboratory Test Results. Acceptance of test results from an approved laboratory will be considered if the test results are less than two years old at the time of submission.

### **12.2 Retesting of Class AP Aggregate.**



- 12.2.1 AP aggregate sources with an average beam expansion of 0.010 percent or less will be sampled and tested at least every three to five years.
  - 12.2.2 AP aggregate sources with an average beam expansion greater than 0.010 percent but less than 0.045 percent will be sampled and tested at least every two years.
  - 12.2.3 AP Aggregate sources with an average beam expansion of 0.045 percent but less than 0.060 percent will be sampled and tested at least annually.
  - 12.2.4 The Department may approve the retesting of aggregates for AP use by an approved laboratory if prior written request is made and approval is given. In all cases, the Department will obtain the aggregate samples and properly identify them.
- 12.3 **Re-Sampling of Rejected Aggregate.** After one year or when enhancing process changes have been made, additional sampling and testing may be considered. Requests for additional sampling and testing shall be submitted in writing to the State Materials Engineer, Division of Materials and Tests with a copy to the appropriate District Testing Engineer. If less than one year has elapsed, an explanation of the enhancing process changes shall be included.

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## **INDIANA DEPARTMENT OF TRANSPORTATION**

### **Division of Materials and Tests**

#### **Directive 201**

### **Certified Aggregate Producer Program Procedures**

The Procedures for administering the Certified Aggregate Producer Program (CAPP) will be as follows:

#### **PLANT**

A Plant is defined as any location which mineral aggregate is processed into a final material. Different processes for separate materials and stockpile yards at one location are considered part of the Plant. A Producer Yard is defined as a location of stockpiled aggregate materials under the control of the Producer at a point removed from the Plant. The Producer Yard is considered part of the Plant for CAPP purposes.

For a Producer Yard, an addenda is added to the Quality Control Plan (QCP) of the Plant and shall include the specifics concerning the location, materials stockpiled at the Producer Yard, and the required sampling and testing that will be conducted. If there is a new Certified Material produced at the Producer Yard, a QCP Annex also is submitted. If the Producer Yard is for temporary purposes, such as for a specific contract, the addenda is removed from the QCP upon completion of supplying the material to the contract and a QCP Annex is submitted to remove the product as a Certified Material. Examples of a Producer Yard are as follows:

1. A Plant elects to stockpile materials at a location that would be geographically closer to their customers and subsequently shipped for multiple applications. This may include materials that are supplied for an INDOT contract, for a HMA or concrete plant, or for other customers. Start of production and normal production samples are obtained and tested at the originating source of the aggregates and load-out samples are obtained and tested for material shipped from the Producer Yard stockpiles.
2. A Contractor recycles concrete at a location close to a contract to provide an aggregate for the same contract. The Contractor requests that a Certified Aggregate Producer add the recycled concrete location as a Producer Yard. Start of production and normal production samples are obtained and tested at the Producer Yard in accordance with the Coordinated Testing Phase requirements. Upon acceptance of the aggregate material as a Certified Material, the aggregate may be shipped to the contract. Normal production samples are obtained and tested during additional production of the material and load-out samples are obtained and tested for material shipped to the contract.

#### **REDISTRIBUTION TERMINAL**

A Redistribution Terminal is defined as a supplier of aggregate material(s) other than a Plant. Aggregates may be materials from either a non-CAPP or CAPP source, and all aggregates are

required to have acceptable production quality test results and meet all of the applicable CAPP requirements. Aggregates originating from a non-CAPP source are tested at the start of production, normal production, and load-out frequencies, and aggregates originating from a CAPP source are tested at the load-out frequency. The Redistribution Terminal is required to maintain the production quality testing documentation for each non-CAPP source. A CAPP Q number is assigned to the Redistribution Terminal and this location may supply Certified Materials to any customer.

The QCP is required to include the originating source and size of aggregate and the applicable requirements of the CAPP. Examples of a Redistribution Terminal are as follows:

1. A CAPP source purchases aggregates from non-CAPP aggregate sources that are not interested in becoming Certified Producers or purchases aggregates from other CAPP sources. The Plant is designated as both a Plant and Redistribution Terminal.
2. A Certified HMA Producer obtains aggregates from non-CAPP sources and/or distributes aggregates from CAPP sources. The HMA plant may use the aggregates in the HMA plant or sell the aggregates as CAPP Certified Materials. Aggregates from CAPP sources that are used in the HMA plant do not require any testing for the CAPP unless the aggregates in the same stockpile are sold as Certified Materials. The HMA plant is designated a Redistribution Terminal.
3. A trucking firm purchases aggregates from CAPP and/or non-CAPP sources and stockpiles the aggregates at a designated location. The location where the aggregates are stockpiled is designated a Redistribution Terminal.
4. An aggregate source has limited access by truck and ships all aggregates by barge to other locations for stockpiling and subsequent distribution to customers. Each location that the aggregate is unloaded and stockpiled for distribution is designated a Redistribution Terminal. The originating source of the aggregates is designated a non-CAPP source.

## **COORDINATED TESTING PHASE**

1. Any Producer making a request to enter the CAPP shall do so in writing to the State Materials Engineer. This request shall include a list of all of the materials to be produced at the source regardless of whether the materials are for Department or other uses.
2. The Geologist Supervisor will inform the District by memorandum of the notification from the Producer and request the District to check the Producer's laboratory and readiness to proceed with the Coordinated Testing Phase. A copy of the memorandum will be sent to the Producer.
3. The District will forward their findings to the Geologist Supervisor by memorandum.
4. During this phase, the Producer shall develop the details of the QCP and demonstrate the ability to produce to the 95 % compliance standard for a coarse aggregate and the Standard Specifications gradation requirements for a dense

graded and fine aggregate, if applicable. The Coordinated Testing procedures should be limited to coarse aggregate sizes 5 or 8 or a QA coarse aggregate, dense - graded sizes 43, 53, or 73, and fine aggregate sizes 23 or 24 or a QA fine aggregate. Frequency of sampling and testing shall be as follows:

Start of Production -- a minimum of once every 1000 t for the first 5000 t but not to exceed 2 per calendar day per material

Normal Production -- a minimum of once every 2000 t but not to exceed 2 per calendar day per material

The District will conduct side-by-side testing on as many of the Producers samples as possible. In any event, District testing will be conducted on not less than every other test done by the Producer or until the District is satisfied that the Producer is sampling and testing correctly and the test results are within the CAPP guidelines. The District will notify the Geologist Supervisor in writing when the side-by-side testing is completed and indicate the products that were tested.

5. The Producer shall submit test results, control charts, and diaries on the first of each month to the District Testing Engineer (DTE), who will maintain this data. A minimum of three months of data or a period as determined by the Department will be required in this phase.
6. The DTE will prepare a Coordinated Phase checklist to verify that the test data, control charts, and diary meet the requirements of the CAPP. The diary may be maintained either electronically or by hard copy; however, if maintained electronically, the Producer shall submit a hard copy. This phase will not be completed until these checklists have been satisfied. A copy of each checklist will be sent to the Geologist Supervisor.
7. Two copies of the QCP shall be submitted by the Producer. One copy shall be submitted to the District and the other copy to the Geologist Supervisor. After reviewing the QCP, the District will forward the QCP checklist, any additional comments, and the District recommendations to the Geologist Supervisor.
8. A QCP checklist will be completed by the Geologist Supervisor. If problems or deficiencies are discovered in the QCP, a letter will be sent to the Producer stating the recommended changes. A copy of the letter will be sent to the DTE.

## **TRIAL PHASE**

1. When the QCP is approved and the side-by-side testing is successfully completed, a letter from the Division of Materials and Tests is sent to the Producer informing him of the QCP approval and entrance into Step Two of the CAPP, the Trial Phase. Monthly submissions of the test results, control charts and diaries are continued during this phase. A copy of the letter is sent to the DTE and Geologist Supervisor.
2. During the Trial Phase, the Producer shall demonstrate the ability to meet the control limits for each active material. Changes in the QCP by addenda may be

made at the audit close-out meeting.

If the target mean and control limits for an aggregate are not changed from the value shown in the approved QCP, the Producer shall have a minimum of one month of data to complete this phase. If the target mean and control limits are changed by an addendum to the QCP, the Producer shall have a minimum of 10 test values to verify the proposed revision.

If the aggregate source is confident that the materials are being maintained within the control limits, the source may request an audit. Target means, standard deviations and control limits for materials not included in the approved QCP may be added by addenda at a later date. However, if the aggregate source does not have the confidence in their ability to control all of their products, the Producer may wait for the completion of the Trial Phase on all of the products before an audit is conducted. The Department retains the right to delay certification until the Producer demonstrates the ability to perform in accordance with their approved QCP.

3. Toward the apparent end of the Trial Phase, the District will schedule the audit of the Producer. Representatives from the Aggregates Section of Materials and Tests may accompany the District audit team on this inspection. An audit checklist will be used to verify that the Producer's operations are in accordance with the QCP. Any deficiencies found will be documented on the Corrective Action sheets. The DTE will arrange a close-out meeting with the Producer within 10 working days after the audit to discuss the resolution of all outstanding matters or set deadlines for solutions.
4. The DTE will forward the CAPP Audit checklist, any additional District comments, and recommendations of the audit, including the necessary addenda to the QCP, to the Geologist Supervisor.

## **CERTIFICATION**

1. After approval for certification has been made, a certificate will be prepared and given to the Producer.
2. A letter welcoming the Producer to certified status that assigns the Q number, lists the Certified Materials, and establishes the beginning date of Certification will be prepared by the Geologist Supervisor, signed by the State Materials Engineer and forwarded to the Certified Producer. A copy will be sent to the DTE. A current list of Certified Producers will be maintained by the Geologist Supervisor.

## **ADDENDA**

1. A Producer may add a Certified Material to the QCP by an addendum. A QCP Annex form is completed by the Producer and submitted to the DTE. The form is reviewed and if satisfactory is forwarded to the Geologist Supervisor for assignment of a D number.

2. All addenda are submitted to the DTE for approval. Addenda may only be submitted at an audit close-out meeting or between January 1<sup>st</sup> and April 1<sup>st</sup> of each calendar year. Each submittal is required to have an Authentication Page that is dated and signed by the Management Representative. The DTE will review the addenda, sign and date the Authentication Page if satisfactory, and submit the document to the Geologist Supervisor. A copy of the document is retained by the District and another copy is returned to the Producer.

## **INACTIVE STATUS**

1. A Producer may request to be placed on Inactive Status to temporarily suspend meeting the requirements of a Certified Producer by submitting a statement to the State Materials Engineer requesting Inactive Status.
2. The statement will include estimated tonnages of approved materials onsite, plans for the material and, if possible, locations of materials off site such as asphalt plants or concrete plants. Once the statement has been reviewed and approved, the Division of Materials and Tests will place the source on inactive status in SiteManager and remove the Source from the Certified Materials Listings.

## **ANNUAL AUDITS OF CERTIFIED PRODUCERS**

1. The District will audit each Certified Producer on a random basis at least once per year.
2. An advance notification of at least 24 hours will be given to the Certified Producer of a scheduled audit, which will take place during a normal working day. The DTE will notify the Geologist Supervisor a minimum of one week in advance of the date that an audit will take place.
3. The audit will be conducted by the District Geologist, the Area Supervisor for the region overseeing the Producer, and an Aggregate Technician. The DTE should participate in selected audits.
4. After the audit, completed copies of the CAPP audit checklist, including at least one Side-by-Side Test Result, at least one Compliance Rate Worksheet, Corrective Action sheets, and necessary addenda to the QCP, will be sent to the Geologist Supervisor.
5. The Aggregates Section of Materials and Tests will participate in a joint audit with each District at least once per year.

## **PARTIAL AUDITS OF CERTIFIED PRODUCERS**

1. Certified Producers with special concerns such as the source is known to have a deleterious problem, or if there is recurring problems with a Producer, then partial audits may be randomly scheduled.

2. The partial audit will be conducted by the District Geologist; the Area Supervisor for the region overseeing the Producer; and an Aggregate Technician. The DTE should participate in selected partial audits.
3. An advance notification of at least one day will be given to the Certified Producer of a scheduled partial audit, which will take place during a normal working day.
4. Copies of the Partial Audit Checklist including Corrective Action sheets will be provided to the DTE.



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**CERTIFIED AGGREGATE PRODUCER PROGRAM  
ITM No. 211-26**

**1.0 SCOPE.**

- 1.1** This procedure covers the requirements for an aggregate supplier to become a Certified Aggregate Producer.
- 1.2** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

- 2.1** Documents required by the Program shall be maintained either electronically or hard copies. All required documents shall be readily available and easily accessible to all necessary personnel.

**2.2 AASHTO Standards.**

- T 11 Materials Finer Than 75  $\mu\text{m}$  (No. 200) Sieve in Mineral Aggregates by Washing
- T 27 Sieve Analysis of Fine and Coarse Aggregates
- T 112 Clay Lumps and Friable Particles in Aggregates

**2.3 ASTM Standards**

- D5821 Determining the Percentage of Fractured Particles in Coarse Aggregate
- E29 Practice for Using Significant Digits in Test Data to Determine Conformance with Specifications

**2.4 ITM Standards**

- 203 Control Procedures for Classification of Aggregates
- 207 Sampling Stockpiled Aggregates
- 206 Scratch Hardness of Coarse Aggregate Particles
- 210 Class AP Coarse Aggregate
- 212 Acceptance Procedures of Air Cooled Blast Furnace Slag for Leachate Determination
- 219 Acceptance Procedures of Steel Furnace Slag for Deleterious Materials
- 588 Percent Within Limits (PWL)
- 902 Verifying Sieves
- 906 Verifying Mechanical Shakers
- 910 Verifying Balances



**2.5** Each Plant/Redistribution Terminal shall have the following current documents on file at the location indicated in the Quality Control Plan (QCP):

**2.5.1** Indiana Department of Transportation Certified Aggregate Producer Program (ITM 211).

**2.5.2** Indiana Department of Transportation Standard Specifications. (Includes applicable Supplemental Specifications)

**2.5.3** Indiana Quality Assurance Certified Aggregate Technician Training Manual for Producer Technicians.

**2.5.4** The QCP for the Certified Plant/Redistribution Terminal.

### **3.0 TERMINOLOGY.**

**3.1 Terms and Abbreviations.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specifications, Section 101 and the following:

**3.2** Addenda. Any addition or deletion to the QCP.

**3.3** Addenda Summary Sheet. A page of the QCP, located in the Appendix that is used to record a brief description of addenda, other than items on the QCP Annex, until such time that they are incorporated into the QCP.

**3.4** Adherent Fines. Fine particles smaller than the No. 200 sieve created from handling, or silt or clay **particles** that adhere to the coarse aggregate.

**3.5** Certified Aggregate Producer. A Plant/Redistribution Terminal that meets the requirements of the Program, continues to be under the same ownership, and is approved by the Department.

**3.6** Certified Material. An aggregate product produced under the Certified Aggregate Producer Program (CAPP) for Department use.

**3.7** Coarse Aggregate. Aggregate that has a minimum of 20 percent retained on the No. 4 sieve.

**3.8** District. The Department's District Office responsible for administering the materials and test functions in a local area of the state.

**3.9** Fine Aggregate. Aggregate that is 100 percent passing the 3/8 in. sieve and a minimum of 80 percent passing the No. 4 sieve.

- 3.10** Non-Certified Aggregate Producer. Any Plant/Redistribution Terminal not approved under the Program. These shall include Plants/Redistribution Terminals never entering the Program, those dropping out, and those that have been removed from Certified status by the Department for failure to comply with the Program.
- 3.11** Producer. A company or owner who shall assume responsibility for each of their Certified Plants in compliance with the CAPP.
- 3.12** Program. The Department CAPP.
- 3.13** Qualified Products List (QPL). INDOT's lists of preapproved manufacturers, materials, products, sources, or suppliers.
- 3.14** Quality Control Plan (QCP). A document written by the Producer that is site-specific and includes the production, policies, and procedures used by the Producer.
- 3.15** QCP Annex. A page of the QCP, located in the Appendix, that is used to record revisions for Certified Material additions, Certified Material deletions, target mean and control limit values, or Certified Aggregate Technicians until such time that they are incorporated into the QCP.
- 3.16** SC Aggregate. Aggregate specification required for select seal coat (chip seal) projects.
- 4.0** **SIGNIFICANCE AND USE.** The Indiana Certified Aggregate Producer Program is a program whereby a qualified aggregate Producer desiring to supply material to the Indiana Department of Transportation assumes all of the Plant site controls and the Department monitors the Producers production, sampling and testing procedures.
- 5.0** **PARTICIPANTS.**
- 5.1** **Plant.** Any location at which aggregate is processed into a final material shall be considered a Plant. Different processes for separate materials and stockpile yards at one location shall be considered part of the Plant.
- A Producer Yard shall be the location of stockpiled aggregate materials under the control of the Producer at a point removed from the Plant. This will be considered part of the Producer's total operation.
- 5.2** **Redistribution Terminal.** Any supplier of aggregate material(s) other than at a Plant shall be considered a Redistribution Terminal. Prior source documentation will be required by the Department.
- 6.0** **PRODUCER PERSONNEL.** The Producer's personnel shall include a Management Representative, a Certified Aggregate Technician, and a Qualified Technician.

- 6.1 Management Representative.** The Management Representative shall be responsible for all aspects of production, handling, and control required by the CAPP at each Certified Aggregate Producer's Plant/Redistribution Terminal.
- 6.2 Certified Aggregate Technician.** A Certified Aggregate Technician is a Producer or Consultant employee who has successfully completed the Certified Aggregate Technician Training Program and has been certified by the Department.

The Certified Aggregate Technician may be responsible for more than one Plant/Redistribution Terminal. The technician shall be at the Plant(s)/Redistribution Terminal(s) to perform the pertinent duties during critical activities and to meet the requirements of the QCP. The technician shall supervise the sampling and testing of material, the maintenance of control charts, and the maintenance of the diary. All sampling and testing required by the Program shall be conducted by a Qualified Technician.

- 6.3 Qualified Technician.** An individual who has successfully completed the written and proficiency testing requirements of the Department's Independent Assurance and Qualified Acceptance Personnel Programs.

## **7.0 MATERIALS.**

- 7.1** Material shall be produced in one of three categories: Standard Specification, Quality Assurance, or Alternate. The intended end use of the material and the control limits shall determine the category in which the material is classified.
- 7.2 Standard Specification.** Standard Specification materials shall include all Certified Materials controlled by aggregate gradations as defined in the Department Standard Specifications and the construction contract documents.
- 7.3 Quality Assurance.** Quality Assurance (or QA) materials shall include all Certified Materials controlled by aggregate gradations established by the Producer.
- 7.4 Alternate.** Alternate materials shall include all materials produced for non-state (commercial) use which are not intended to comply with either Standard Specification material or Quality Assurance material.

## **8.0 LABORATORY**

- 8.1** The Producer shall have a suitable laboratory to accomplish the requirements of the CAPP. Laboratories will be inspected by a Department representative before the Producer enters the Coordinated Testing Phase. Laboratories will also be inspected during annual audits and as needed to maintain the integrity of the Program.

- 8.2** The laboratory testing equipment shall meet the requirements of the test methods identified for the required sampling and testing, and as stated herein except that an electronic balance shall be provided. The electronic balance shall be readable to 0.1 g and accurate to 0.2 g or 0.1 percent of the test load, whichever is greater, at any point within the range of use.
- 8.3** The Producer shall maintain laboratory service for each Certified Plant/Redistribution Terminal. One approved laboratory may be used for more than one Plant/Redistribution Terminal provided the requirements of the Program are being maintained. Additional Laboratories can be listed in the QCP if backup laboratories are needed for the source.
- 8.4** The Department shall be allowed access to inspect any laboratory used for the Program, and witness production, handling, and control activities during production of Certified Materials.

## **9.0 TEST EQUIPMENT CALIBRATION**

- 9.1** The test equipment furnished by the Producer shall be properly calibrated or verified, and maintained within the limits described in the applicable test method. The equipment shall be calibrated or verified prior to beginning testing in the Coordinated Testing Phase.
- 9.2** The Producer shall verify the equipment at the frequency indicated:

<b>Equipment</b>	<b>Requirement</b>	<b>Minimum Frequency</b>	<b>Procedure</b>
Balances	Verification	12 mo.	ITM 910
Mechanical Shakers	Check Sieving Thoroughness	12 mo.	ITM 906
Sieves	Check Physical Condition	12 mo.	ITM 902

- 9.3** The equipment verification documentation shall include:
- 9.3.1** A description of the equipment verified including Model and Serial Number.
- 9.3.2** Name of the person and company performing the verification
- 9.3.3** Identification of the verification equipment used, if any (namely, standard weights, thermometers etc.).
- 9.3.4** Last date verification was performed, and next due date.
- 9.3.5** A reference to the procedure used.
- 9.3.6** Detailed records showing the results of the verification performed.

## **10.0 DIARY.**

- 10.1** Each Certified Aggregate Producer shall maintain a diary. The diary, either electronic and/or hard copy, shall have at least one page devoted to each day that there is a material related operation.
- 10.2** The Producer shall retain the diary on file for a minimum period of three years.
- 10.3** Entries in the diary shall as a minimum include:
- 10.3.1** General weather conditions;
  - 10.3.2** Area of mining operation (location and ledges, or pit area);
  - 10.3.3** Estimated quantity of materials produced;
  - 10.3.4** Time test samples were obtained and tests completed;
  - 10.3.5** Nonconforming gradation tests, and the resulting appropriate action taken;
  - 10.3.6** Changes in key personnel, if any;
  - 10.3.7** Significant changes in equipment, plant, screens, etc., which may affect the current statistical results of aggregate materials;
  - 10.3.8** Any significant event or problem;
  - 10.3.9** Any nonconforming trends in the five-point moving average, as well as the action taken to correct the trends, if needed;
  - 10.3.10** Current compliance rate, updated weekly, for critical sieve products being produced; and
  - 10.3.11** Current load-out PWL from the Department's provided spreadsheet, updated weekly, for applicable products being shipped;
- 10.4** The entry in the diary shall be signed by the Certified Aggregate Technician. On occasion the diary may be signed by another person; however, the diary is required to be counter-signed by the Certified Aggregate Technician.

## **11.0 MATERIALS SAMPLING AND TESTING.**

- 11.1** Sampling shall be in accordance with ITM 207 and testing of all materials that require control for aggregate gradation, decantation, deleterious, crushed particles, and flakiness shall be in accordance with the applicable test method. Sampling shall be performed on uniform tonnage increments in an unbiased manner, and testing of the samples shall be accomplished in such time as to assure that process

control is maintained. Testing shall be performed in accordance with the test methods as designated herein, and the applicable exceptions listed in the Standard Specifications. Test values shall be reported to the nearest 0.1 percent, except for the crushed particle content which shall be reported to the nearest 1 percent. Results are to be rounded using the standard "5" up procedures in accordance with 109.01(a).

**11.2** The Producer shall retain the test results on file for a minimum period of three years.

**11.3 Gradation.** The gradation of the material shall be determined in accordance with AASHTO T 27, except as required in 11.4. The frequency of sampling and testing shall be as follows:

**11.3.1 Start of Production Frequency.** Start of production material is the first 5000 t when producing a new material. Sampling and testing shall be performed a minimum of once every 1000 t for the first 5000 t, but not required to exceed two per calendar day per material.

**11.3.2 Normal Production Frequency.** Normal production material is material produced after the start of production. Sampling and testing shall be performed a minimum of once every 2000 t, but not required to exceed two per calendar-day per material.

**11.3.3 Load-Out Frequency.** Load-out material is the Certified Material that is shipped from the Plant/Redistribution Terminal. For all products shipping  $\geq 100$  tons per year, sampling and testing shall be in accordance with the QCP and shall be established at a consistent frequency per year per product and at least as frequently as the following Load-Out Frequency Table.

Load-Out Frequency	
Shipping Tons/Year	Frequency
< 100 t/year	No samples required
≥ 100 t/year	1 / ≤ 8000 t
Notes on Frequency:	
<p>1. Begin every calendar year with at least one load-out sample within the first 1000 tons. The remaining samples shall be at the QCP established frequency tonnage interval, calculated beginning at zero tons for the calendar year.</p> <p>2. No daily, monthly, or annual limit on the number of tests.</p> <p>3. Sample within ±1000 t shipped or ±1 business day at the completion of each full interval.</p> <p>4. Report and plot all QCP scheduled frequency test results, even when corrective action is later taken based on the test results.</p> <p>5. Track tonnage continuously throughout the year for determination of the next sample. “Reset” the sampling tonnage to zero on January 1 of each calendar year.</p> <p>For example, if the QCP for a product sets the frequency at 8000 tons, then: Obtain the first sample of the calendar year between 0 and 1000 tons. Obtain the second sample of the calendar year at 8000+/-1000 tons, the third sample of the calendar year at 16,000+/-1000 tons, and so on.</p>	

- 11.4 Decantation.** The decantation of the material shall be determined in accordance with AASHTO T 11. All load-out samples shall be washed and decanted. Unless specific problems are encountered, start of production and normal production samples need not be washed and decanted.
- 11.5 Crushed Particles.** The percentage of crushed coarse aggregate particles for gravel shall be determined in accordance with ASTM D5821. The frequency of sampling and testing shall not be less than once per week for each size of material during the start of production and normal production or as designated in the QCP. No test is required if the week's production is less than 100 t.
- 11.6 Deleterious Materials.** The percentage of deleterious materials shall be determined for coarse aggregates in accordance with AASHTO T 112, ITM 206

and the Standard Specifications. The frequency of sampling and testing shall not be less than once per week for each size of material during the start of production and normal production or as designated in the QCP. No test is required if the week's production is less than 100 t.

- 11.7 Flakiness Index.** The flakiness index shall be determined for SC aggregates in accordance with ITM 224 and the Standard Specifications. The frequency of sampling and testing shall be not less than once per week for each size of material during the start of production and normal production or as designated in the QCP. No test is required if the week's production is less than 100 t.

## 12.0 GRADATION CONTROL

- 12.1** Gradation control shall be performed for each Plant/Redistribution Terminal in accordance with the QCP and requirements herein.
- 12.2 Sieve Control.** Standard Specification materials shall meet the critical sieve requirements for the materials and sieves shown below:

Material	Critical Sieve
No. 5	1/2 in.
No. 8	1/2 in.
No. 9	3/8 in.
No. 11, SC 11	No. 4
No. 12, SC 12	No. 4
SC 16	No. 4

All other Standard Specification materials shall meet the specification or construction contract gradation limit requirements for all sieves, unless controlled by a critical sieve as identified in the QCP.

Quality Assurance coarse aggregate materials, with a designated maximum particle size of 1½ in. or smaller, shall have a critical sieve identified in the QCP. All other Quality Assurance materials shall meet the gradation limit requirements for all sieves, unless controlled by a critical sieve as identified in the QCP.

- 12.3 Target Mean.** All data that exists on a single process shall be used to establish the target mean. For a totally new process, at least 10 consecutive normal production test results shall be used. If production within a year does not result in sufficient data to establish a target mean, then a target mean shall not be determined.

The target mean on the critical sieve for Standard Specification materials, shall be no closer to either specification limit than 1.65 multiplied by the standard deviation designated in the QCP. The standard deviation shall be equal to or less than 5.0.



The target mean on the critical sieve for Quality Assurance materials shall be established by the Producer for any sampling point.

- 12.4 Control Limits.** Control limits are applicable to all critical sieve test results. They are established as plus or minus two standard deviations, but no greater than plus or minus 10 percent, from the target mean. When a target mean cannot be determined because of insufficient production, the Standard Specification gradation limits or gradation for Quality Assurance material shall be used.

When identified in the Producer's QCP, the Department may agree to designated target mean values and control limits on the critical sieve for the load-out tests that may be different from the production target mean values and control limits.

- 12.5 Test Compliance.** For material produced under either the Standard Specification or Quality Assurance categories, 95 percent of all gradation test results on the critical sieve shall statistically be between 10 percent below and 10 percent above the target mean at any one point of sampling. All other sieves shall be maintained within the Standard Specification, Quality Assurance, or construction contract gradation requirements.

- 12.5.1 Production.** All normal production data on the critical sieves identified for sieve control representing a process shall be included in the calculations for statistical compliance. When the control limits for load-out tests are different than the production control limits, all of the load-out tests may be used. All retest and other extraneous data shall be used for information.

The Producer shall review the 30 most recent production tests for each critical sieve product to ensure the compliance rate is met in accordance with Section 917.

**12.5.1.1** If the compliance rate is less than 95% and the standard deviation is less than 5.0%, the Producer shall establish a new target mean and recalculate the compliance rate.

**12.5.1.2** If the compliance rate is less than 95% and the standard deviation is 5.0% or greater, the Producer shall designate the stockpile of that product as non-certified material.

**12.5.1.3** The Producer shall notify the District Geologist and note in the diary when either of the situations in 12.5.1.1 or 12.5.1.2 occurs.

- 12.5.2 Load-Out.** The Producer shall monitor the PWL for each size 8 and 11 product and their Quality Assurance material equivalents on the Department's provided spreadsheet. For these products, the most recent 30

normal frequency load-out tests since the last documented significant process change on the critical sieves identified shall be included in the calculation for statistical compliance. After at least 5 tests are reported, when the PWL drops below 90 for a given product, the Producer shall notify the District Geologist. A plan shall be developed to bring the PWL of the product back above 90.

**12.5.3 Load-Out Reporting/Uploading.** The Producer shall upload load-out results for size 8s and 11s (and QA equivalents) as directed by the Department in the annual Management Representative letter.

**12.6 Alternate Materials.** Controls and limits as detailed in the Program herein do not apply for Alternate materials unless so stated in the QCP.

**12.7 Nonconforming Tests.** A nonconforming test shall be any test which falls outside the control limits identified in the QCP for Standard Specification or Quality Assurance materials controlled by a critical sieve. For sieves other than the critical sieve, a nonconforming test shall be any test which falls outside the control limits identified in the QCP for Quality Assurance materials, or outside the specification limits for materials controlled by Standard Specification requirements. Any nonconforming normal production or load-out test shall be followed immediately by a corrective action. Corrective actions shall include, but are not limited to, investigation for assignable cause, correction of known assignable cause, and retesting. Plants/Redistribution Terminals that continue to have repeated nonconforming normal production or load-out test results, due to lack of appropriate action, shall be subject to suspension from the Certified status by the Department.

A second consecutive nonconforming normal production test result shall require the material to be isolated from the approved stockpile until action has been taken to eliminate the cause of the nonconformity. When a second consecutive nonconforming load-out test occurs, shipping from that stockpile shall cease until corrective action and testing has occurred that verifies the stockpile is acceptable for shipment.

**12.8 Statistical Compliance.** The Producer shall continuously monitor and maintain statistical compliance in accordance with 917 and this ITM.

## **13.0 CONTROL CHARTS.**

**13.1** Control charts are a visual representation of the process control exercised by a Producer. Unless otherwise provided in the QCP, the control charts shall be either posted on a wall at the laboratory or maintained electronically. At a minimum, the control charts shall be maintained until 30 production data points have been plotted. After that time at least 30 production data points shall be continuously

displayed. If load-out points are plotted separately, then the load-out charts shall be maintained for a minimum of 30 load-out data points.

**13.2** The Producer shall retain the control charts on file for a minimum period of three years.

**13.3 Application.** As a minimum, control charts will be required for gradation control using all start of production and normal production test results for all Standard Specification and Quality Assurance materials or for gradation controlled Alternate materials. For materials which have a critical sieve, only the critical sieve is required to be charted. For all other charted materials, all applicable sieves shall be shown on the chart. Load-out test results shall also be plotted and may be displayed on the same chart as start of production and normal production test results when the target mean remains unchanged. When the load-out target mean is designated in the QCP to be different from the production target mean, the load-out samples shall be charted separately. Other properties may also be charted as part of the Producer's overall QCP. A separate chart shall be maintained for each size of material being produced.

The Producer shall report and plot all normal frequency production and normal frequency load-out test results, even when corrective action is taken based on the test results. Resamples, retests, information tests, and diversions of entire production runs from under the cone, shall have test results listed in the diary but are not normally plotted.

**13.4 Chart Construction.**

**13.4.1** The target mean shall be represented by a heavy long dash followed by a short dash line.

**13.4.2** Control limits shall be represented by heavy solid lines.

**13.4.3** Upper and lower specification limits shall be indicated by short, dashed lines. Specification limits are not required to be included on charts for critical sieves that have established control limits.

**13.4.4** The horizontal lines on the chart indicating the specification limits, control limits, and target mean value, if applicable, shall be numerically identified in the left margin.

**13.4.5** The plot point for the production test results shall be surrounded by a small circle, and each consecutive point shall be connected by a solid straight line.

**13.4.6** The moving average of the most current five production test values shall be plotted for the critical sieve. The plot points shall be indicated by a small triangle symbol and connected by solid straight lines.

**13.4.7** When load-out test points appear on the same chart as production points, they shall be represented by a small square.

**13.4.8** When load-out test points are plotted on a separate chart, they shall be represented by a small square and connected by a straight line.

**13.4.9** Test results shall be plotted left to right in chronological order, and dates corresponding to each test shall be shown along the horizontal axis.

**13.4.10** Any proposed deviation from these procedures shall be identified in the QCP.

**13.5 Conformance.** The Producer shall apply statistical techniques to interpret all control charts and take corrective action when so indicated. Corrective action shall include, but is not limited to, investigation for assignable cause, correction of known assignable cause, and retesting, if necessary.

## **14.0 QUALITY CONTROL PLAN.**

**14.1** The QCP is a fundamental element in the Program, and shall be one of the first considerations in approval by the Department. Each Plant/Redistribution Terminal providing aggregate under the Program shall have a written QCP, which shall be site and plant specific, and be the basis of control. The QCP shall describe the methods of controlling all properties and quality aspects, which shall involve greater detail than the basic requirements of the Department specifications and policies. The QCP shall encompass the total process from preliminary material quality approval through the point where the aggregate leaves the Producer's control.

**14.2** The QCP shall include the following information for each Plant/Redistribution Terminal, if applicable:

**14.2.1** The location of the site, including latitude and longitude, physical address, and mailing address if different than the physical address. Reference to the nearest identifiable points such as highways and towns shall also be included.

**14.2.2** The Parent Company, Management Representative, Certified Aggregate Technician(s) and Qualified Technician(s) at each location and the procedure for contacting these individuals by phone, US Mail, and email address. Also, the CAPP duties and responsibilities of each of the people listed shall be specified.

- 14.2.3** A list and description of all portions of the mineral deposit(s) indicating the different quality classes as established in the current editions of ITM 203 and ITM 210, and as indicated on the Source Category Classification Approval letter. The manner in which each quality class will be processed, handled, and stockpiled shall be specified.
- 14.2.4** A statement regarding AP Aggregate shall be made to include the ledges for stone, production zone for gravel, the general handling and crushing procedures, and stockpile signage. An AP Production Control Plan may be included in the Appendix instead of this statement.
- 14.2.5** A statement regarding ACBF shall be made to include the procedures for sampling and testing for determination of leachate in accordance with ITM 212.
- 14.2.6** A statement regarding SF shall be made to include the procedures for sampling and testing for determination of bulk specific gravity when SF is used in SMA mixtures. Also, a statement regarding SF coarse aggregate shall be made to include the procedures for sampling and testing for determination of deleterious materials in accordance with ITM 219.
- 14.2.7** A statement regarding lightweight aggregate shall be made to include the procedures for sampling and testing for determination of the specific gravity factor and the absorption in accordance with ITM 222.
- 14.2.8** A statement regarding natural sand fine aggregate composite stockpiling shall be made to include the procedures for sampling and testing when composite stockpiling of multiple sources into one stockpile.
- 14.2.9** An explanation for each material having marginal quality characteristics, and the plan or controls to be used for such materials.
- 14.2.10** An identification of the category in which each material produced is classified.
- 14.2.11** A generic production flow diagram, which shall be a step-by-step chart using standard symbols, showing all the steps involved with mining and processing from the natural deposit to the finished material and the points of sampling. Detailed items such as equipment manufacturer's names, screen sizes, dimensions, etc., are not required to be listed. A copy of the symbol legend shall be included.
- 14.2.12** A sampling plan that includes locations, devices, techniques, frequencies, and test methods, if applicable.

**14.2.13** A testing plan that includes the types of tests, and test methods. The procedure for isolating nonconforming material shall be specified.

**14.2.14** A list of the target mean values, standard deviations, and control limits on the critical sieves for each material identified as being controlled by critical sieve requirements. Changes in the target mean are **allowed** by addenda to the QCP.

Materials for which no control limits are appropriate shall be identified.

The gradation limits for all Quality Assurance materials shall be included.

**Gradation limits for products that are intended to meet multiple gradation requirements shall use the more stringent controls for each product and these limits shall be included.**

**14.2.15** A description of any other process control techniques that will be used beyond the minimums established by Department specifications and policies. These controls may include, but are not limited to, the following:

- a) Different types or greater frequencies of material testing.
- b) Other midstream sampling and testing prior to material completion.
- c) Visual checks and monitoring.

**14.2.16** A plan for downstream controls after material completion. These controls shall address such items as the identification of material stockpiles by signing, **including ledges if applicable**, or other acceptable methods, techniques for construction of proper stockpiles, material retrieval techniques and safeguards to ensure the loading and shipping of uncontaminated material.

**14.2.17** **A list of test equipment that is calibrated or verified, the test methods and frequency of calibration or verification of the equipment, and a statement of accessibility of the laboratory and documentation to Department personnel.**

**If the laboratory is not located at the Plant, the location of the laboratory shall be designated, and the procedure for transporting samples to the laboratory shall be included.**

**14.2.18** A documentation plan with details on control charting, test data, and the diary. Copies of the forms may be included.

- 14.2.19** The method by which the frequency of production and load-out testing of the Certified Materials is verified.
- 14.2.20** The location of the reference documents, control charts, diary, test data, material shipment records, and any other pertinent information.
- 14.2.21** The method of control for each Producer Yard.
- 14.2.22** A statement of the procedure for handling addenda.
- 14.2.23** Annual Aggregate Source Report (stone sources only). The report shall be in accordance with ITM 203, 8.1 and shall be included in the Appendix of the QCP. Redistribution Terminals are required to include this report for materials received from a source that is not a Certified Aggregate Producer.
- 14.2.24** An Appendix. As a minimum the appendix shall contain an Addenda Summary Sheet.
- 14.3 Authentication.** The last page of the QCP shall contain two signature blocks. The right-hand block shall be signed and dated at the time of submittal by the Producer's Management Representative, and shall include the title of the person making the signature. The left hand block shall be signed and dated at the time of approval by the State Materials Engineer, Division of Materials and Tests. The Producer shall submit the QCP to the District Geologist for review. The District Geologist will forward it to the INDOT Statewide Geologist for final review and approval. The authentication page will be returned to the Producer after approval.
- 14.4 Addenda.** The Producer shall transmit all applicable process control revisions to the District Testing Engineer in the format of addenda to the QCP. Each page of the QCP that is revised shall include the source number, date of revision, and means of identifying the revision. The addenda shall include a signed and dated authentication page that is signed by the Management Representative and subsequently signed by the District Testing Engineer upon approval.
- Revisions for Certified Material additions, Certified Material deletions, target mean and control limit values, or Certified Aggregate Technicians shall be submitted in the format of a QCP Annex as they occur, and upon approval by the District Testing Engineer shall be included in the Appendix of the QCP. Revisions, other than items on the QCP Annex, shall be maintained on the Addenda Summary Sheet in the QCP Appendix.
- Addenda may be submitted at the annual audit close-out meeting or between January 1<sup>st</sup> and April 1<sup>st</sup> of each calendar year. The addenda shall include items on the QCP Annex, items on the Addenda Summary Sheet, and any other

necessary revisions at the time of submittal. Upon incorporation into the QCP as addenda, the QCP Annex and items on the Addenda Summary Sheet shall be removed from the QCP Appendix. The Annual Aggregate Source Report shall be submitted by April 1<sup>st</sup> of each calendar year.

**14.5 Certified Material Additions.** A Producer without a Certified Material that is controlled by a critical sieve is required to meet the applicable requirements of 12.0 prior to adding the Certified Material. A Producer without a Certified Material that is not controlled by a critical sieve is required to verify that the material meets the requirements of the Standard Specifications or Quality Assurance gradation limits for all sieves prior to adding the Certified Material. A Producer may add a Certified Material using the following procedure:

**14.5.1** Prior to shipment of the material, the Producer shall submit a QCP Annex to the District Testing Engineer.

**14.5.2** Shipment of the material may begin as soon as approved by the District Testing Engineer.

**14.5.3** The control limits will be the Standard Specification gradation limits for the appropriate size of material. If the material is a Quality Assurance material then the Producer shall designate the control limits. If the material is controlled by a critical sieve, then the control limits for that sieve shall be used when the target mean and standard deviation are in accordance with 12.3.

**14.5.4** Materials that do not have a critical sieve requirement shall be sampled and tested in accordance with 11.3.

Materials that have a critical sieve requirement shall be sampled and tested a minimum of once every 1000 t, but not required to exceed two per calendar day. Load-out frequency shall be in accordance with 11.3. When the target mean and standard deviation are in accordance with 12.3, the frequency of sampling and testing for normal production shall be in accordance with 11.3.

**14.5.5** Split samples shall be obtained by the Producer for each sample and provided to the Department. The sample splitting procedure and test results agreement shall be the same as the guidelines used during the Coordinated Testing Phase. Split samples shall be obtained until the Department is satisfied with the performance and testing results from the Producer. If the material is similar in size to an existing Certified Material, the District Testing Engineer may waive the requirement for split samples.

## **15.0 COORDINATED TESTING PHASE.**



- 15.1** The Coordinated Testing Phase is the initial phase for Certification. The purpose of this phase is to build mutual confidence in production capability, capacity, uniformity, and quality. Sampling and testing procedures, and test results will be reviewed in a coordinated and shared manner between the Department and the Producer. While operating in this phase, the Producer shall develop the details of the QCP and demonstrate the ability to produce to the required test compliance rate. Mean test values and standard deviations are developed during this process for the critical sieves. Each Plant/Redistribution Terminal shall be under the Coordinated Testing Phase for at least three months of production, or a period as determined by the Department.
- 15.2** Each Plant/Redistribution Terminal requesting to enter the Coordinated Testing Phase shall do so in writing to the State Materials Engineer, Division of Materials and Tests. The request shall include all of the materials to be supplied at the source regardless of whether the materials are for Department or other uses.
- 15.3 Aggregate Sizes.** While operating in the Coordinated Testing Phase, Producers are encouraged to limit the Coordinated Testing procedures to aggregate sizes 5 or 8; AASHTO 57, 43, 53, or 73; or 23 or 24. Quality Assurance materials may also be used for the Coordinated Testing procedures.
- 15.4 Control Charts.** Test results shall be charted in accordance with requirements for Certified Material, except that the corrective action need not apply. The Producer will be expected to use the charts as basic indicators of variation, and to become aware of limitations needed on any process. During this phase charts shall be maintained for all sieves.
- 15.5 Sampling and Testing.** The frequency and types of tests for the Producer's sampling and testing shall be the same as the minimum requirements of the start of production and normal production for Certified Material, except that decantation will be required.

The use of a random sampling method is encouraged; however, if a random sampling method is not used, and if more than one sample per day is required, the samples shall be spread throughout the day's expected production. Department aggregate technicians will conduct coordinated/side-by-side testing on as many of these samples as possible. In any event, Department testing will be conducted on not less than every other test conducted by the Producer, or until the Department is satisfied with the performance and testing results from the Producer.

The coordinated tests shall utilize a split sample for all tests except non-durable, total chert, which shall use the same sample. The procedure for splitting samples shall be in accordance with AASHTO R 76. Both split halves on the final split shall weigh within 10 percent of each other after splitting. If not, both halves shall be recombined and split until this requirement is met.

**15.6 Producer Records.** During the Coordinated Testing Phase the Plant/Redistribution Terminal shall maintain the same references, charts, reports, diary, and Source Category Classification Approval letter as required for the Certified Aggregate Producer

**15.7 Test Results Agreement.** The Coordinated Testing Phase guidelines for test agreement are:

Sieve Analysis	
Sieve Size	Maximum % Difference
1-1/2 in. thru 3/8 in.	5
No. 4 thru No. 8	3*
Minus No. 200 (decant less than 5.0)	0.5
Minus No. 200 (decant equal to or greater than 5.0)	1.0

\* The maximum % difference for sizes 43, 53, and 73 is 5 %.

**Crushed Particles.** The difference should not exceed 5 percent for both one and two face fractured particles.

**Non-Durable, Total Chert.** The difference should not exceed 40 percent of the lowest results or 1 percent, whichever is greater.

Test result differences will be resolved on-site between the Producer's technicians and the Department's technician, if feasible. If such differences are not readily resolved, the Area Supervisor and/or the District Testing Engineer will resolve the difference. The resolution will be recorded by both technicians.

**15.8 Data Reporting.** Within the first week of each month copies of the test results, control charts, and the diary shall be forwarded to the District Testing Engineer.

**16.0 TRIAL PHASE.** The Trial Phase is the second phase for obtaining Certification. This phase starts when the Producer has successfully completed the Coordinated Testing Phase and the QCP has been approved. During this phase the Producer shall demonstrate the ability to follow the QCP. Monthly submissions of test results, control charts, and the diary shall be continued. Revisions to the QCP by addenda may be made at the Trial Phase Audit close-out meeting.

## **17.0 CERTIFICATION.**

**17.1** Each Plant/Redistribution Terminal meeting the requirements of the CAPP shall be eligible for Certification. Each Certified Producer must comply with the Program as detailed herein. After approval, monthly submissions of test results, control charts, and the diary are not required.

- 17.2 Material Shipment Record.** Certified Producers shall have records in sufficient detail so as to enable the Department to verify the frequency of load-out testing.

The record shall include:

**17.2.1** Date of shipment.

**17.2.2** Originating source name of material.

**17.2.3** Total amount shipped each day.

**17.2.4** The type and size of aggregate.

**17.2.5** Ledges for stone materials.

- 17.3 Weigh Tickets.** The Certified Producers approval number, originating source name, source number, aggregate size, class of aggregate from the INDOT QPL, and ledges for stone materials, shall be entered on each weigh ticket representing material for Department use.

- 17.4 Change of Ownership.** Once the Department has Certified a Plant/Redistribution Terminal, there is no automatic expiration date for the Certification; however, in the event of a change in ownership of the Plant/Redistribution Terminal, the certification shall expire on the date of such change. The new ownership may avoid such expiration by immediately submitting a statement to the State Materials Engineer, Division of Materials and Tests indicating recognition of the details of the CAPP, the existing QCP, and a clear pronouncement of intent to operate in accordance with the requirements of both documents.

- 17.5 Inactive Status.** A Producer may request to be placed on Inactive Status to temporarily suspend meeting the requirements as a Certified Aggregate Producer. The procedures for temporary suspension of the Program shall be as follows:

**17.5.1** A Producer may request to be placed on Inactive Status to temporarily suspend meeting the requirements of a Certified Producer by submitting a statement to the State Materials Engineer, Division of Materials and Tests requesting Inactive Status. If for a duration of three years, a Producer has not produced or shipped any material which would require production or load-out testing under the CAPP, the Division of Materials and Tests may notify and place the source in Inactive Status.

**17.5.2** While in the Inactive Status, the Producer may ship all Certified Materials in existing stockpiles at the time of suspension if the requirements of the Program have been met for these stockpiles.

**17.5.3** Once made “Inactive”, the Producer may obtain Certified Aggregate Producer status again by submitting a statement to the State Materials Engineer, Division of Materials and Tests requesting re-entry into the Program. Any revisions to the Quality Control Plan that the Division of Materials and Tests has on file shall be submitted with the re-entry request. Quality approval will be granted in accordance with ITM 203, including updated Source Production Qualities as described in Table II. The district will work with the source to complete Start of Production Frequency gradation testing in accordance with ITM 211 11.3.1. Passing physical quality and gradation results are required prior to shipping any state approved material.

## **18.0 DEPARTMENT RESPONSIBILITIES**

**18.1 QPL.** The Department will maintain an updated QPL of all Plants/Redistribution Terminals that are currently certified.

**18.2 Certified Technician List.** The Department will maintain an updated, published list of all currently certified technicians.

**18.3 Auditing.** The Department will conduct an annual audit of the Program on a random basis at each Plant/Redistribution Terminal to verify that the Producer's production, load-out, sampling, and testing procedures are in accordance with the Program. The audit will include random samples taken by the Producer for informational purposes as directed by the Department. These samples shall be provided to the Department. The sample splitting procedure, and test result agreement shall be in accordance with 15.5.

**18.4 Mineral Quality.** The Department will be responsible for the pre-approval of the mineral quality at each Plant location in accordance with ITM 203 and ITM 210.

**18.5 Training.** The Department will administer a Certified Aggregate Technician Training Program for those aggregate technicians that perform the required duties of the Program. Certification of the technicians will be provided by the Department upon successful completion of the training. Certification for all Certified Technicians expires annually on July 1. Recertification is accomplished by attending a Regional Spring Meeting or by watching a video of the meeting and passing a quiz. Recertification will extend the certification to July 1 of the following year.

The Department will administer the Independent Assurance and Qualified Acceptance Personnel Program for those aggregate technicians that perform

acceptance sampling and testing duties of the Program. Qualification of the technicians will be provided by the Department upon successful completion of a written examination and proficiency test. Certified Technicians cannot perform acceptance sampling and testing duties unless the Certified Technician is currently qualified by Independent Assurance. Qualified Technicians are only allowed to perform acceptance sampling and testing duties but are not allowed to sign the daily diary and CAPP documents. Only Certified Technicians can sign these documents.

- 18.6 Certification Removal.** Removal from the approved status of a Certified Producer will be the responsibility of the State Materials Engineer, Division of Materials and Tests.

The Producer shall have a right to dispute the removal from Certified Producer status to the Director, Division of Materials and Tests.

- 18.7 District Jurisdiction.** The District Testing Engineer will have the authority to suspend shipment of a specific material or stockpile if the Producer has failed to comply with the Program such that material quality and uniformity is not being met. Such action will be promptly reported to the State Materials Engineer, Division of Materials and Tests.

The Producer shall have the right to dispute the suspension of shipment by the District Testing Engineer to the State Materials Engineer, Division of Materials and Tests.

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**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**ACCEPTANCE PROCEDURES OF AIR COOLED BLAST  
FURNACE SLAG FOR LEACHATE DETERMINATION  
ITM No. 212-19**

**1.0 SCOPE.**

- 1.1** This method sets forth the procedure for sampling and testing ACBF slag for determination of leachate from the aggregate.
- 1.2** Unaged ACBF slag may contain an excessive quantity of calcium sulfide which may leach when inundated with a large volume of water. The leachate may emit hydrogen sulfide gas, and have a greenish-yellow color. When exposed to air, the rate of leaching will diminish with time as the ACBF slag ages. The aging process will allow the calcium sulfide to oxidize to sulfates and carbonates.
- 1.3** This procedure will apply to all uses of ACBF slag except for use in HMA or PCC.
- 1.4** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- E 11** Wire-Cloth Sieves for Testing Purposes
- M 231** Weighing Devices Used in the Testing of Materials
- R 76** Reducing Samples of Aggregate to Testing Size

**2.2 ASTM Standards.**

- D 1193** Reagent Water
- E 70** pH of Aqueous Solutions with the Glass Electrode
- E 832** Laboratory Filter Paper
- E 960** Laboratory Glass Beakers

**2.3 ITM Standards.**

- 207** Sampling Stockpiled Aggregates

- 3.0 TERMINOLOGY.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specification, Section 101.
- 4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate ACBF slag for determination of leachate from the aggregate. The ACBF slag is required to meet the requirements of this test method before use, except when used in HMA or PCC.
- 5.0 APPARATUS.**
- 5.1** Balance, Class G20, in accordance with AASHTO M 231
  - 5.2** No. 4 sieve, in accordance with AASHTO M 92
  - 5.3** Filter paper, medium grade, in accordance with ASTM E 832
  - 5.4** Five-gallon bucket, plastic, with lid
  - 5.5** Funnel
  - 5.6** Glass beaker, 150 mL, in accordance with ASTM E 960
  - 5.7** pH meter
  - 5.8** Tamping rod, round, steel, approximately 5/8 in. in diameter, and approximately 24 in. in length
- 6.0 REAGENTS.** Deionized or distilled water, conforming to the requirements of ASTM D 1193
- 7.0 GENERAL REQUIREMENTS.**
- 7.1** Each Aggregate Producer requesting to have ACBF slag tested in accordance with this procedure shall contact the appropriate District Testing Engineer to initiate the approval process.
  - 7.2** Sampling and testing shall be conducted by the Aggregate Producer.
  - 7.3** ACBF slag shall be sampled as the stockpiles are being constructed. Existing stockpiles shall be sampled randomly from the interior of the stockpile.

7.4 Acceptance for use of ACBF slag will be given on each stockpile of approximately 2000 tons. Stockpiles that do not meet the acceptance criteria of this test method may be tested again after 30 days from the test date.

7.5 Stockpile location, stockpile identification, and test results shall be maintained at the ACBF slag source and shall be available for inspection.

## 8.0 SAMPLING.

8.1 Sampling of aggregates shall be from a small pile in accordance with ITM 207, Section 5.1. Samples shall not be obtained from the surface of the stockpile.

8.2 Each sample shall consist of 80 to 100 lbm of material.

8.3 The test sample shall be obtained by reducing the original sample in accordance with AASHTO R 76 to a sample size of 20 to 25 lbm

## 9.0 PROCEDURE.

9.1 Place the test sample in a five-gallon bucket, fill with distilled or deionized water until the sample is covered with at least 1/2 in. and not more than 1 in. of water, and place the lid on the bucket. No additional water shall be added after the test is started. Allow the sample to soak for one day.

9.2 After the one day soaking period, thoroughly stir the sample with the tamping rod and collect a water sample of approximately 100 mL.

9.3 Using a funnel, filter the water sample through the filter paper into a glass beaker.

9.4 Observe the clarity and color of the water.

9.5 Calibrate a pH meter in accordance with the manufacturer's instructions and ASTM E 70, and then determine the pH of the water sample to the nearest 0.1 pH unit.

9.6 If the water is not clear and colorless, or the pH is not within 6.0 to 10.5, the material is not acceptable and the test is completed. Upon the completion of the color observation and pH measurement, the 100 mL water sample is discarded.

9.7 If the water is clear and colorless and the pH is within 6.0 to 10.5, then allow the sample to soak for another fourteen days. The bucket shall be covered with the lid and steps 9.2 to 9.6 repeated after three days, seven days, and fifteen days from the initial soaking.



## **10.0 ACCEPTANCE CRITERIA.**

- 10.1** If after one day, three days, seven days, or fifteen days of soaking, the water is not clear, the material will not be acceptable for use.
- 10.2** If after one day, three days, seven days, or fifteen days of soaking, the pH is not within 6.0 to 10.5, the material will not be acceptable for use.
- 10.3** If after fifteen days of soaking, the water is clear and the pH is within 6.0 to 10.5, the material will be acceptable for use.

## **11.0 REPORT.**

- 11.1** All pH values shall be reported to the nearest 0.1 unit.
- 11.2** The clarity and color of the water.



## INDIANA DEPARTMENT OF TRANSPORTATION DIVISION OF MATERIALS AND TESTS

### ACCEPTANCE PROCEDURES FOR POLISH RESISTANT COARSE AGGREGATES ITM No. 214-23

#### 1.0 SCOPE.

- 1.1 This method sets forth the acceptance procedures to be used when Aggregate Producers request that polish resistant coarse aggregates be evaluated for use in HMA surface mixtures.
- 1.2 Polish resistant coarse aggregates are specified for use under certain traffic ESAL loading conditions to obtain skid-resistant HMA surface courses.
- 1.3 Coarse aggregates tested in accordance with this procedure shall be dolomite containing less than 10.3 percent elemental magnesium, crushed limestone, or gravel.
- 1.4 The acceptance procedure is a two-part process. Part One requires a comparison of the coarse aggregate to a Department-approved dolomite (listed in §6.2 below) or polish resistant coarse aggregate using the British Wheel in accordance with ASTM D3319 and the British Pendulum Tester in accordance with ASTM E303. If the results of the comparison indicate that the coarse aggregate has a Residual Polishing Value (RPV-10) of at least one less than the RPV-10 value obtained from the Department-approved dolomite or polish resistant coarse aggregate, then Part Two may be initiated.

Part Two requires that a test section of HMA using the coarse aggregate and a control test section of HMA using a Department-approved dolomite (listed in §6.2 below) or polish resistant coarse aggregate be placed on a contract. The coarse aggregate and the Department-approved dolomite or polish resistant coarse aggregate may be blended with air-cooled blast furnace slag, steel furnace slag, or sandstone coarse aggregate for the two test sections. Acceptance of the coarse aggregate is made on the basis of an evaluation of friction test data obtained after two years of exposure to traffic; however an aggregate may be considered for acceptance after one year of exposure to traffic at the discretion of the Department.

The Department may utilize data obtained from Part 1 of ITM 221 for acceptance of a coarse aggregate as a polish resistant coarse aggregate at the discretion of the Department.

- 1.5 The Aggregate Producer will be required to maintain a warranty bond on the HMA surface course of the test section using the proposed polish resistant coarse

aggregate. The bond amount shall be sufficient to replace the test section with material satisfactory to the Department. Upon opening the test section to unrestricted traffic, the warranty bond will be in effect for a total of two years. The warranty bond is required to be properly executed by a surety company satisfactory to the Department and be payable to the State of Indiana. Appendix A shall be used for the warranty bond.

- 1.6** If, within two years of exposure to traffic, the average friction number of the proposed polish resistant coarse aggregate is less than the average friction number of the Department-approved dolomite or polish resistant coarse aggregate, the Department will evaluate the test section to determine if a problem exists. If remedial work is required, the Aggregate Producer shall conduct the work at no cost to the Department. If the Aggregate Producer cannot conduct the remedial work within a timely manner, the Department has the option to execute the warranty bond and have the remedial work conducted by other forces.
- 1.7** This procedure may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

## **2.0 REFERENCES.**

### **2.1 AASHTO Standards.**

- T 11 Materials Finer than 75  $\mu\text{m}$  (No. 200) Sieve in Mineral Aggregates by Washing  
T 27 Sieve Analysis of Fine and Coarse Aggregates

### **2.2 ASTM Standards.**

- D3319 Accelerated Polishing of Aggregates Using the British Wheel  
E274 Skid Resistance of Paved Surfaces Using a Full Scale Tire  
E303 Measuring Surface Frictional Properties Using the British Pendulum Tester  
E524 Smooth Tread Standard Tire for Special-Purpose Pavement Skid Resistance Tests

### **2.3 ITM Standards.**

- 207 Sampling Stockpiled Aggregates

## **3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate the polish resistance of coarse aggregates for use in HMA surface mixtures.

**5.0 APPARATUS.**

**5.1** British Wheel, in accordance with ASTM D3319

**5.2** British Pendulum Tester, in accordance with ASTM E303

**5.3** Friction vehicle and instrumentation in accordance with ASTM E274

**5.4** Smooth Tread Standard Tire in accordance with ASTM E524

**6.0 GENERAL REQUIREMENTS.**

**6.1** Each Aggregate Producer requesting to have a coarse aggregate tested in accordance with this procedure shall do so in writing to the State Materials Engineer, Division of Materials and Tests with a copy sent to the appropriate District Testing Engineer. Information concerning the type of material, and ledge numbers, if applicable, shall be included.

**6.2** The Department-approved dolomite or polish resistant coarse aggregate used in the control test section shall be obtained from one of the following sources.

2232 – Heidelberg Materials Midwest Agg Inc.,  
Ft. Wayne, IN (Ledges 1-7)  
Approved Dolomite Source

2267 – U.S. Aggregates, Inc. – Pleasant Mills  
Decatur IN (Ledges 1001-4)  
Approved Dolomite Source

2535 – U.S. Aggregates – Columbus  
Columbus, IN (Ledges 11-12)

2542 – IMI/Sellersburg Plant  
Sellersburg, IN (Ledges 11-13)

2651 – Knox County Sand & Gravel  
Vincennes, IN

An alternate dolomite or polish resistant coarse aggregate source for the control section will be approved by the Department if the dolomite or polish resistant coarse aggregate meets the requirements of Part One of §1.4 when compared to the dolomite or polish resistant coarse aggregate from one of the approved sources.

- 6.3** Testing shall be conducted by the Department or by a Department-approved Laboratory. The cost of shipping and testing of the coarse aggregate shall be the responsibility of the Aggregate Producer.
- 6.4** Friction testing of the test sections will be conducted by the Department at no expense to the Aggregate Producer.
- 6.5** Approval of the source as a Polish Resistant Aggregate Source will be based on results from both Part One and Part Two of this ITM.

## **7.0 SAMPLING.**

- 7.1** Sampling of the coarse aggregate and Department-approved dolomite or polish resistant coarse aggregates shall be in accordance with ITM 207 in the presence of the Department.
- 7.2** The samples shall be sufficient in quantity to yield a minimum of 50 lb of material that is passing the 3/8 in. sieve and retained on the No. 4 sieve.
- 7.3** The samples shall be washed and decanted in accordance with AASHTO T 11.
- 7.4** The samples shall be sieved in accordance with AASHTO T 27 to obtain the appropriate quantity of material passing the 3/8 in. sieve and retained on the No. 4 sieve required for the test of Part One in §1.4.

## **8.0 PROCEDURE.**

### **8.1 Calibration and Testing Using the British Wheel and British Pendulum Tester (PART ONE).**

#### **8.1.1 Control Specimens.**

- a)** Control specimens shall be fabricated using a 4 to 1 mixture by weight of 20-30 grade Ottawa sand and polyester resin in accordance with ASTM D3319. The back portion of the specimen shall be finished with the polyester resin to facilitate preparation of the bearing surface of the specimens.
- b)** The specimens may be heated in the molds in an oven at a temperature of 212°F for 2 to 4 hours if warping of the specimen occurs due to shrinkage of the polyester resin. Specimens shall be shaped to an 8 in. radius of curvature by mechanical clamps.

- c) If the specimens do not properly fit on the road wheel, the bottom of the specimens shall be finished by hand sanding to ensure a proper fit.

**8.1.2** Pendulum Calibration. Two sets of four control specimens each shall be prepared and used to calibrate the British Pendulum Tester. The two sets of specimens shall have initial friction values (PV-i) of  $29 \pm 1$  and  $38 \pm 1$  respectively in accordance with ASTM E303. These values shall be designated as the benchmark values for the control specimens. At the beginning of each polish resistant test, the control specimens shall be tested. The British Pendulum Tester shall be adjusted until the average PV-i values for each of the two calibration groups are within plus or minus one unit from the established averages.

**8.1.3** Specimen Preparation and Polish Resistant Calibration.

- a) Specimens shall be prepared in accordance with ASTM D3319. A minimum of five specimens each shall be prepared for the coarse aggregate and the Department-approved dolomite.
- b) Control specimens shall be used to develop consistency in specimen preparation and polishing. Four specimens shall be prepared and included with the test specimens on the British Wheel. The average of the four control specimens shall have PV-I values of 37 to 39 measured in accordance with ASTM E303. If these criteria are not met, all specimens prepared for the polish resistant test shall be discarded and new specimens prepared.
- c) After polishing for 10 hours, the RPV-10 values for the four control specimens shall be measured in accordance with ASTM E303. The average RPV-10 value shall be 28 to 30. If this criteria is not met, the results from the polish resistant test shall not be used.
- d) A control chart shall be prepared and maintained for control specimen RPV-10 values and the corresponding date of test. The average of the four values for each test shall be plotted.

**8.1.4** Polishing Machine Tire.

- a) The Accelerated Polishing Machine tire shall be a smooth-tread, solid (non-pneumatic), tire approved by the Department.
- b) The tire shall be replaced when the RPV-10 values of the control specimens have decreased by more than four points from the RPV-10 values obtained from a new tire.

- c) A control chart shall be prepared and maintained for each tire indicating the tire usage hours and the corresponding RPV-10 values of the control specimens.

#### **8.1.5 Pendulum Testing and Reporting.**

- a) Specimens shall be tested in accordance with ASTM E303. Broken specimens and specimens with aggregate missing in the slider or contact area shall be discarded. Tests shall be made until four consecutive measurements give the same RPV-10 value.
- b) The test value for any aggregate shall be the average from a minimum of four specimens.
- c) If the coarse aggregate RPV-10 value is  $\pm 1$  of the Department-approved dolomite or polish resistant coarse aggregate RPV-10 value, the Aggregate Producer may request to proceed to Part Two of this procedure.
- d) Copies of the test information shall be sent to the Aggregate Producer and the Department and shall include the following:
  - 1. Coarse aggregate source identification
  - 2. Type of material
  - 3. Ledges of the aggregate, if applicable
  - 4. Date sampled
  - 5. Individual(s) obtaining sample of coarse aggregate
  - 6. PV-i and RPV-10 values for the control specimens
  - 7. Control chart for the RPV-10 values of the control specimens
  - 8. Control chart for the tire usage hours
  - 9. RPV-10 values of the coarse aggregate
  - 10. RPV-10 values of the Department-approved dolomite or polish resistant coarse aggregate

## **8.2 TEST SECTIONS (PART TWO).**

### **8.2.1 Test Section Selection.**

- a) Upon evaluation and approval of the polish resistant data, a contract will be selected by the Department for placement of the coarse aggregate and Department-approved dolomite or polish resistant coarse aggregate test sections. The contract will have traffic ESAL's equal to or greater than 3,000,000 and have continuous uninterrupted traffic over the test sections.

- b) A 1 mi test section of HMA using the coarse aggregate shall be placed adjacent to a 1 mi test section of HMA using the Department-approved dolomite or polish resistant coarse aggregate. Both test sections shall be placed in the same Driving Lane. The two test sections shall be located between any major intersections on the contract.

#### **8.2.2 Friction Testing.**

- a) Each test section will be tested by the Department in accordance with ASTM E274. A smooth tire in accordance with ASTM E524 and a 40 mph test speed will be used.
- b) Friction testing will be performed after six months, one year, eighteen months, and two years of exposure to traffic.

### **9.0 ACCEPTANCE CRITERIA.**

- 9.1 After two years exposure to traffic, if the coarse aggregate HMA friction values are equal to or greater than the Department-approved dolomite or polish resistant coarse aggregate HMA friction values, the aggregate will be approved as a Polish Resistant Coarse Aggregate.

-or-

After three years exposure to traffic, if the coarse aggregate HMA friction values are equal to or greater than an average of 35.0, with no individual location value less than 30.0, the aggregate will be approved as a Polish Resistant Coarse Aggregate.

- 9.2 The Department will maintain a Qualified Product List of Polish Resistant Aggregate Sources including those aggregates meeting the requirements outlined herein. The list will include two categories as follows:

- 9.2.1 Coarse aggregates that are approved for use in HMA surface mixtures for contracts with traffic ESAL's equal to or greater than 3,000,000 and less than 10,000,000.

- 9.2.2 Coarse aggregates that are approved for use when blended with air-cooled blast furnace slag, steel furnace slag, or sandstone in HMA surface mixtures for contracts with traffic ESAL's equal to or greater than 10,000,000.

The aggregate source and ledge number(s), if applicable, will be placed on the Qualified Products List in the ESAL category that the contract used to approve the aggregate was within.



- 9.3** The aggregate will remain on the Department's Qualified Products List unless the material is not performing satisfactorily, as determined by the Department.

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**POLISH RESISTANT COARSE AGGREGATE PRODUCER  
ITM 214  
WARRANTY BOND**

Know all persons by these presents that we, \_\_\_\_\_ as principal and \_\_\_\_\_ as surety, are held and firmly bound unto the State of Indiana (hereinafter referred to as obligee) in the full and just sum of \$ \_\_\_\_\_, lawful money of the United States of America, for the payment of which, well and truly to be made, we bind ourselves, our heirs, administrators, executors, successors, and assigns, jointly and severally, firmly by these presents.

The condition of the above obligation is that for two (2) years after the date the test section of HMA pavement located on \_\_\_\_\_, reference point \_\_\_\_\_ to reference point \_\_\_\_\_ is  
(Beginning Point) (Ending Point)  
completed and opened to unrestricted traffic; such warranty is to be in accordance with the Indiana Test Method 214 which is made a part of this bond for warranted test section of HMA pavement. If the principal satisfactorily fulfills the above condition, then this obligation shall be null and void; otherwise such obligation is to remain in full force and effect.

It is agreed that no modifications, omissions, or additions in or to the terms of the ITM 214 or the contract or in or to the plans or specifications shall affect the obligation of the surety on its bond.

In witness whereof, we hereunto set our hands and seal.

Name: \_\_\_\_\_

Address: \_\_\_\_\_

By: \_\_\_\_\_

Signature Surety

Title

\_\_\_\_\_  
(Print or Typed) Surety

State of Indiana, County of \_\_\_\_\_ SS:

Personally appeared before me,

\_\_\_\_\_  
as surety and acknowledge the executions of the  
above bond

this \_\_\_\_\_ day of \_\_\_\_\_, 20 \_\_\_\_\_

By \_\_\_\_\_

Signature

Notary Public

\_\_\_\_\_  
(Print of Typed) Notary

My Commission Expires \_\_\_\_\_, 20 \_\_\_\_\_

\_\_\_\_\_  
(County of Residence)

Name: \_\_\_\_\_

Address: \_\_\_\_\_

By: \_\_\_\_\_

Signature Principle

Title

\_\_\_\_\_  
(Print of Typed) Principal

State of Indiana, County of \_\_\_\_\_ SS:

Personally appeared before me,

\_\_\_\_\_  
as surety and acknowledge the executions of the  
above bond

this \_\_\_\_\_ day of \_\_\_\_\_, 20 \_\_\_\_\_

By \_\_\_\_\_

Signature

Notary Public

\_\_\_\_\_  
(Print of Typed) Notary

My Commission Expires \_\_\_\_\_, 20 \_\_\_\_\_

\_\_\_\_\_  
(County of Residence)



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**ACCEPTANCE PROCEDURES OF STEEL FURNACE SLAG  
FOR DELETERIOUS MATERIALS  
ITM No. 219-22**

**1.0 SCOPE.**

- 1.1** This method sets forth the procedure for sampling and testing SF slag for determination of deleterious materials in the aggregate.
- 1.2** Unaged SF slag may contain an excessive amount of unstable compounds such as calcium oxide and magnesium oxide which when hydrated will expand.
- 1.3** This procedure will apply to the use of coarse aggregate SF slag in HMA Base and Intermediate mixtures.
- 1.4** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- M 92 Wire-Cloth Sieves for Testing Purposes
- M 231 Weighing Devices Used in the Testing of Materials
- T 11 Materials Finer Than 75  $\mu\text{m}$  (No. 200) Sieve in Mineral Aggregates by Washing
- T 27 Sieve Analysis of Fine and Coarse Aggregates
- T 107 Autoclave Expansion of Hydraulic Cement
- T 245 Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus
- T 248 Reducing Samples of Aggregate to Testing Size

**2.2 ITM Standards.**

- 207 Sampling Stockpiled Aggregates

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations will be in accordance with the Department's Standard Specification, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate SF slag for determination of the potential for expansion of deleterious materials in the aggregate. Expansion of steel slag in encapsulated uses such as HMA Base and Intermediate mixtures may cause heaving of the HMA and eventual failure of the pavement by raveling. This test procedure is conducted by the Certified Aggregate Producer and the amount of deleterious material of the stockpiled steel slag is required to be determined prior to use.

**5.0 APPARATUS.**

**5.1** Balance, Class G2, in accordance with AASHTO M 231

**5.2** Sieves, in accordance with AASHTO M 92

**5.3** Autoclave, in accordance with AASHTO T 107

**5.3** Rupture disk, in accordance with AASHTO T 107

**5.4** Mold assembly, 4 in., in accordance with AASHTO T 245, with the collar, compaction mold, and base plate welded together

**6.0 SAFETY PRECAUTIONS.**

**6.1** The pressure gage shall have a capacity of 600 psi. A gage with too small or too large a capacity may be a hazard. For pressure above the specified maximum working pressure, with a smaller capacity gage, the pressure may not be measured by the scale. With a larger capacity gage, the arc of movement may be too small to be noticed. The operator shall assure that the gage hand has not passed the maximum graduation on the scale.

**6.2** The pressure gage should be tested for proper operation. A thermometer is used together with the pressure gage to provide a means of detecting any failure of the pressure gage to operate properly and to indicate any unusual condition.

**6.3** The automatic control should be maintained in proper working order at all times.

**6.4** The safety valve is set to relieve the pressure at approximately 6 to 10 % above the maximum of 305 psi specified in this test method, which is approximately 330 psi. Unless the manufacturer has given specific maintenance instructions for the safety valve, the valve should be tested twice each year. A gage testing device is used for the testing, or the automatic controls are adjusted to allow the autoclave to reach a pressure of approximately 330 psi, at which pressure the safety valve will either open or be adjusted to open. The safety valve discharge should be

directed away from the operator. (Note 1)

Note 1 - Unexpected combinations of conditions may occur. For example, in one case the automatic control had failed, the safety valve had become stuck, and the gage hand, which at first glance appeared to be at about zero, had passed the maximum graduation and had come to stop on the wrong side of the pin. This condition of the gage was finally detected and the pressure, then of an unknown magnitude, was released before failure could occur in the apparatus.

- 6.5** Heavy leather work gloves should be worn to prevent burning of the hands when removing the top of the autoclave at the completion of the test. The vent valve is directed away from the operator. When removing the autoclave lid, the lid is tilted so that any steam escaping from beneath the lid will be discharged away from the operator. Care shall be taken to avoid scalding by any liquid that may have been used in the autoclave well.
- 6.6** The operator shall be made aware that for many autoclave pressure gages the return of the gage hand to the initial rest or starting point does not necessarily indicate zero pressure within the autoclave. An appreciable dangerous pressure may still be present.

## **7.0 GENERAL REQUIREMENTS.**

- 7.1** Each Aggregate Producer requesting to have SF slag approved in accordance with this procedure shall contact the appropriate District Testing Engineer to initiate the testing process.
- 7.2** Sampling and testing shall be conducted by the Aggregate Producer.
- 7.3** SF slag shall be sampled as the stockpiles are being constructed. Existing stockpiles shall be sampled randomly from the exterior and interior of the stockpile.
- 7.4** Acceptance for use of SF slag will be given on each stockpile of approximately 2000 t. Stockpiles that do not meet the acceptance criteria of this test method may be tested again after 30 days from the test date.
- 7.5** Stockpile location, stockpile identification, and test results shall be maintained at the SF slag source and shall be available for inspection.

## **8.0 SAMPLING.**

- 8.1** Sampling of aggregates shall be done in accordance with ITM 207.
- 8.2** Each sample shall consist of 25 to 110 lbm of material, depending on the size of the coarse aggregate tested.

**9.0 SAMPLE PREPARATION.**

- 9.1** Reduce the sample in accordance with AASHTO R 76-16 (Standard Practice for Reducing Samples of Aggregate to Testing Size) to a sample size of  $1500 \pm 50\text{g}$  and decant the sample in accordance with AASHTO T 11.
- 9.2** Sieve the sample in accordance with AASHTO T 27 and retain the + No. 4 sieve material. The sample shall be 100 % passing the 1 1/2 in. sieve and a maximum of 2 % passing the No. 4 sieve
- 9.3** Record the initial weight of the + No. 4 sieve material (A) and the gradation
- 9.4** Place the sample in the mold assembly in three approximately equal layers. Each layer should be leveled by raising the opposite sides of the mold alternately and dropping the mold on a firm base.

**10.0 PROCEDURE.**

- 10.1** Place the mold in the autoclave
- 10.2** Following the manufacturer's instructions, bring the autoclave to  $295 \pm 10$  psi
- 10.3** Maintain the autoclave at  $295 \pm 10$  psi for three hours
- 10.4** Remove the mold after the autoclave has cooled
- 10.5** Allow the mold to cool to room temperature
- 10.6** Remove the material from the mold and sieve the material in accordance with AASHTO T 27
- 10.7** Weigh the material passing the No. 4 sieve (B)

**11.0 CALCULATIONS.** The deleterious content is calculated as follows:

$$\% \text{ Deleterious} = \frac{B}{A} \times 100$$

where:

A = initial weight of the sample retained on the No. 4 sieve, g

B = final weight of the sample passing the No. 4 sieve, g

**12.0 REPORT.**

**12.1** The gradation of the sample is reported to the nearest 0.1 %.

**12.2** The deleterious content is reported to the nearest 0.1 %.



**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**CLASS AS AGGREGATE  
FOR USE IN SMA MIXTURE  
ITM No. 220-15T**

**1.0 SCOPE.**

- 1.1** This method covers the procedures to evaluate Class AS coarse aggregates for use in SMA mixture. The procedure includes determining the Micro-Deval abrasion value of the aggregate or aggregate blend and the aggregate degradation of the SMA mixture.
- 1.2** Coarse aggregates applicable to this ITM shall include steel furnace slag, sandstone, dolomite, or polish resistance aggregates from Certified Aggregate Producers in accordance with ITM 211.
- 1.3** This procedure may involve hazardous materials, operations and equipment and may not address all of the safety problems associated with the use of the test method. The user of this ITM is responsible for establishing the appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- T 30 Mechanical Analysis of Extracted Aggregate
- T 312 Preparing and Determining the Density of HMA Specimens by Means of the Superpave Gyratory Compactor
- T 327 Resistance of Coarse Aggregate to Degradation by Abrasion in the Micro-Deval Apparatus
- R 46 Designing Stone Matrix Asphalt

**2.2 ITM Standards.**

- 207 Sampling Stockpiled Aggregates
- 211 Certified Aggregate Producer Program
- 571 Quantitative Extraction of Asphalt and Gradation of Extracted Aggregate from HMA Mixtures
- 586 Asphalt Content by Ignition

- 3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.
- 4.0 SIGNIFICANCE AND USE.** This ITM shall be used to determine the Micro-Deval abrasion and the aggregate degradation of an aggregate or aggregate blend for use as Class AS aggregates in SMA mixtures.
- 5.0 APPARATUS.** The apparatus shall be as required to compact gyratory specimens in accordance with AASHTO T 312, extract the asphalt in accordance with ITM 571 or ITM 586, and determine the gradation of the extracted aggregates in accordance with AASHTO T 30.
- 6.0 SAMPLING.** Sampling of aggregates shall be in accordance with ITM 207.
- 7.0 PROCEDURES.**

**7.1 Micro-Deval Total Abrasion Loss.**

- 7.1.1** Obtain the proposed SMA aggregate blend sheet or an aggregate blend sheet from a mix design prepared in accordance with AASHTO R 46
- 7.1.2** Determine the abrasion loss value for each coarse aggregate of the aggregate blend in accordance with AASHTO T 327. The coarse aggregate or blend of coarse aggregates shall have the total abrasion loss value determined by proportioning the individual coarse aggregate abrasion loss value with the blend percentage for each coarse aggregate.
- 7.1.3** The total abrasion loss value of the coarse aggregate or blend of coarse aggregates shall be determined as follows:

$$LV_{TOTAL} = \frac{(CA_1)(LV_1) + (CA_2)(LV_2) + \dots + (CA_N)(LV_N)}{(CA)_{TOTAL}}$$

where:

- $LV_{TOTAL}$  = total percent abrasion loss value for the coarse aggregate or blend of coarse aggregates
- $CA_1, CA_2 \dots CA_N$  = the blend percentage for coarse aggregates 1, 2...N
- $LV_1, LV_2 \dots LV_N$  = the percent abrasion loss value for coarse aggregates 1, 2...N
- $CA_{TOTAL}$  = total percentage by weight of all coarse aggregates in the blend

**7.2 Aggregate Degradation Loss.**

**7.2.1** Prepare a mix design in accordance with AASHTO R 46

**7.2.2** Compact two gyratory specimens at the optimum design binder content to Ndes gyrations in accordance with AASHTO T 312. Mixture conditioning is not required.

**7.2.3** Prepare an uncompacted mixture sample at the optimum design binder content in accordance with AASHTO T 312. Mixture conditioning is not required.

**7.2.4** Extract the uncompacted mixture and the two gyratory specimens separately in accordance with ITM 571 or ITM 586. Determine the aggregate gradation of each in accordance with AASHTO T 30.

**7.2.5** The aggregate degradation loss value shall be determined as follows:

$$\text{Aggregate Degradation Loss, \%} = A - B$$

where:

A = average % passing the No. 8 sieve from the gyratory specimens

B = % passing the No. 8 sieve from the uncompacted mixture sample

**8.0 ACCEPTANCE CRITERIA.**

**8.1** The total Micro-Deval Abrasion loss value for an acceptable coarse aggregate or blend of coarse aggregates shall be 18.0% or less.

**8.2** The Aggregate Degradation loss value for an acceptable coarse aggregate or blend of coarse aggregates shall be 3.0% or less.

**9.0 REPORT.** The total abrasion loss value as determined in 7.1.3 and the aggregate degradation loss value as determined in 7.2.5 shall be reported on the submitted DMF form.

**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**ACCEPTANCE PROCEDURES FOR HMA SURFACE MIXTURE  
COARSE AGGREGATES FOR ESAL  $\geq 10,000,000$   
ITM No. 221-18**

**1.0 SCOPE.**

- 1.1** This method sets forth the acceptance procedures to be used when Aggregate Producers request that coarse aggregates be evaluated for use in HMA surface mixtures with ESAL  $\geq 10,000,000$ .
- 1.2** HMA surface mixture aggregates are specified for use under certain traffic ESAL loading conditions to obtain skid-resistant HMA surface courses.
- 1.3** Coarse aggregates tested in accordance with this procedure shall be Polish Resistant Aggregates in accordance with ITM 214 or otherwise approved by the Department.
- 1.4** This method is a two part process. Part One requires a comparison of a HMA surface mixture with the proposed coarse aggregate to a HMA surface mixture with an approved steel furnace slag coarse aggregate or other coarse aggregate approved by the Department using the Circular Track Meter (CTM) in accordance with ASTM E 2157 and the Dynamic Friction Tester (DFT) in accordance with ASTM E 1911. The CTM and DFT values are used to determine the International Friction Index (IFI) in accordance with ASTM E 1960. If the results of the comparison indicate that the HMA surface mixture with the proposed coarse aggregate has an IFI value equal to or greater than the IFI value of the HMA surface mixture with steel slag then Part Two may be initiated.

Part Two requires that a test section of HMA surface mixture using the proposed coarse aggregate and a control test section of HMA surface mixture using steel slag or an approved coarse aggregate be placed on a contract. Steel slag may be blended with an approved dolomite or polish resistant aggregate for the control test section. Acceptance of the coarse aggregate is made on the basis of an evaluation of friction test data obtained after two years of exposure to traffic; however, an aggregate may be accepted after one year of exposure to traffic at the discretion of the Department.

- 1.5** The Aggregate Producer will be required to maintain a warranty bond on the HMA surface course of the test section using the proposed coarse aggregate. The bond amount shall be sufficient to replace the test section with material satisfactory to the Department. Upon opening the test section to unrestricted traffic, the warranty bond will be in effect for a total of two years. The warranty

bond is required to be properly executed by a surety company satisfactory to the Department and be payable to the State of Indiana. Appendix A shall be used for the warranty bond.

- 1.6** If within two years of exposure to traffic, the average friction number of the proposed aggregate is less than the average friction number of the approved steel slag, the Department will evaluate the test section to determine if a problem exists. If remedial work is required, the Aggregate Producer shall conduct the work at no cost to the Department. If the Aggregate Producer cannot conduct the remedial work within a timely manner, the Department has the option to execute the warranty bond and have the remedial work conducted by other forces.
- 1.7** This procedure may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

## **2.0 REFERENCES.**

### **2.1 AASHTO Standards.**

- T 11 Materials Finer than 75  $\mu\text{m}$  (No. 200) Sieve in Mineral Aggregates by Washing  
T 27 Sieve Analysis of Fine and Coarse Aggregates

### **2.2 ASTM Standards.**

- E 274 Skid Resistance of Paved Surfaces Using a Full Scale Tire  
E 524 Smooth Tread Standard Tire for Special-Purpose Pavement Skid Resistance Tests  
E 1911 Measuring Paved Surface Frictional Properties Using the Dynamic Friction Tester  
E 1960 Calculating International Friction Index of a Pavement Surface  
E 2157 Measuring Pavement Macrottexture Properties Using the Circular Track Meter

### **2.3 ITM Standards.**

- 207 Sampling Stockpiled Aggregates  
214 Acceptance Procedures for Polish Resistant Aggregates

## **3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate coarse aggregates for use in HMA surface mixtures for ESAL  $\geq 10,000,000$  applications.

**5.0 APPARATUS.**

**5.1** Dynamic Friction Tester in accordance with ASTM E 1911

**5.2** Circular Track Meter in accordance with ASTM E 2157

**5.3** Circular Track Polishing Machine. This device consists of three rubber tires attached to a rotating plate that travels at approximately 47 revolutions per minute resulting in approximately 141 wheel passes per minute. Water is sprayed on the mixture **test ring** surface to remove debris generated during polishing. A total load of 217 lbm is applied through the tires to the surface.

**5.4** Friction vehicle and instrumentation in accordance with ASTM E 274

**5.5** Smooth Tread Standard Tire in accordance with ASTM E 524

**6.0 GENERAL REQUIREMENTS.**

**6.1** Each Aggregate Producer requesting to have a coarse aggregate tested in accordance with this procedure shall do so in writing to the Manager, Office of Materials Management. Information concerning the type of material, and ledge numbers, if applicable, shall be included.

**6.2** The steel slag or other approved **coarse** aggregate material used in the control test section shall be obtained from a Certified Aggregate Producer.

**6.3** Testing shall be conducted by the North Central Superpave Center. The cost of shipping and testing of the coarse aggregate shall be the responsibility of the Aggregate Producer.

**6.4** Friction testing of the test sections will be conducted by the Department at no expense to the Aggregate Producer.

**6.5** Approval of the coarse aggregate for use in HMA surface mixtures for ESAL  $\geq 10,000,000$  will be based on results from both Part One and Part Two of this procedure.

**7.0 SAMPLING.**

**7.1** Sampling of the coarse aggregate and steel slag (or other approved **coarse** aggregate) shall be in accordance with ITM 207 in the presence of the Department.

7.2 Each sample shall be sufficient in quantity to yield a minimum of 60 lbm of dry material that is passing the 1/2 in. sieve.

7.3 The samples shall be washed and decanted in accordance with AASHTO T 11.

## 8.0 PROCEDURE (PART ONE)

### 8.1 Mix Design

8.1.1 Mixtures shall be produced in the laboratory using a mix design designated by the Department. The mix design shall be in accordance with 401.04 for a 9.5mm HMA mixture. PG 64-22 asphalt and a mix design for ESAL Category 4 shall be used.

8.1.2 All other aggregates in the mix design shall be the same type and source for both the mixture using the proposed aggregate and the mixture using steel slag as the coarse aggregate. The design binder content and proportion of the aggregates may vary to account for the difference in absorption and specific gravities of the various aggregates.

### 8.2 Mixture Test Rings

8.2.1 Laboratory produced mixtures shall be reheated to  $300 \pm 9^{\circ}\text{F}$ .

8.2.2 Based on the volume of the gyratory mold and the specific gravity of the mixture, the approximate weight of the mixture that would yield 7 to 8 % air voids ( $V_a$ ) shall be determined.

8.2.3 A minimum of seven gyratory pills shall be compacted at the target air void content.

8.2.4 Once compacted, the gyratory pills shall be thoroughly cooled.

8.2.5 The vertical sides of the gyratory pills shall be cut at a slight angle to allow them to fit together to create a level test ring with a centerline diameter of 11.2 in. (i.e., to fit the footpath of the polishing machine, DFT and CTM). See Figure 1

8.2.6 The seven gyratory pills shall be secured in the test ring with a metal ring clamp, placed in the center of a 20 x 20 x 1.5 in. wooden mold. See Figure 1

8.2.7 Self-leveling cement mix and water shall be thoroughly mixed according to the manufacturer's recommendations and carefully poured into the mold

around the test ring and allowed to cure for a minimum of two days before testing.

- 8.2.8** The mixture **test ring** shall be polished a total of 300,000 wheel passes with the Circular Track Polishing Machine.



Figure 1: Completed Test Ring

### 8.3 Testing and Reporting

- 8.3.1** The mixture **test ring** shall be tested for the surface texture using the CTM in accordance with ASTM E 2157 and the friction using the DFT in accordance with ASTM E 1911 initially before polishing.
- 8.3.2** Polishing shall be stopped after 1500, 9000, 30000, 75000, 165000 and 300000 passes so that the surface texture and friction of the mixture **test ring** may be measured.
- 8.3.3** The CTM and DFT values are used to determine the International Friction Index (IFI) in accordance with ASTM E 1960. If the IFI value of the mixture **test ring** using the proposed aggregate is equal to or greater than the IFI value for the mixture **test ring** using the steel slag, the Aggregate Producer may request to proceed to Part Two of this procedure.



**8.3.4** Copies of the test information shall be sent to the Aggregate Producer and the Department and shall include the following:

- Coarse aggregate source identification
- Type of material
- Ledges of the aggregate, if applicable
- Date sampled
- Individual(s) obtaining the sample of coarse aggregate
- The International Friction Index in accordance with ASTM E 1960

## **9.0 TEST SECTIONS (PART TWO).**

### **9.1 Test Section Selection**

**9.1.1** Upon evaluation and approval of the IFI data, a contract will be selected by the Department for placement of the proposed coarse aggregate test section and a control test section using steel slag from an approved source. The contract will have traffic ESAL's equal to or greater than 10,000,000 and have continuous uninterrupted traffic over the test sections.

**9.1.2** A 1 mi test section of HMA using the proposed coarse aggregate material shall be placed adjacent to a 1 mi test control section of HMA using steel slag. A mixture blend of Polish Resistant Aggregates or dolomite coarse aggregates with steel slag may be used. Both test sections shall be placed in the same driving lane. The two test sections shall be located between any major intersections on the contract.

### **9.2 Friction Testing**

**9.2.1** Each test section will be tested by the Department in accordance with ASTM E 274. A smooth tire in accordance with ASTM E 524 and a 40 mph test speed will be used.

**9.2.2** Friction testing will be performed after approximately six months, one year, eighteen months, and two years of exposure to traffic.

## **10.0 ACCEPTANCE CRITERIA.**

- 10.1** If the proposed coarse aggregate HMA friction values are equal to or greater than the control section HMA friction values after two years of exposure to traffic, the proposed coarse aggregate will be approved for HMA surface mixtures for ESAL  $\geq 10,000,000$ .
- 10.2** The Department will maintain a list of Approved Aggregates including aggregates meeting the requirements outlined herein. The list will include coarse aggregates that are approved for use when air-cooled blast furnace slag, steel furnace slag, or sandstone are required in HMA surface mixtures for contracts with traffic ESAL's equal to or greater than 10,000,000.

The aggregate source and ledge number(s), if applicable, will be placed on the Approved List in the ESAL  $\geq 10,000,000$  category.

- 10.3** The aggregate will remain on the Department Approved List unless the material is not performing satisfactorily, as determined by the Department.

**AGGREGATE PRODUCER  
ITM 221-12P  
WARRANTY BOND**

Know all persons by these presents that we, \_\_\_\_\_ as principal and \_\_\_\_\_ as surety, are held and firmly bound unto the State of Indiana (hereinafter referred to as obligee) in the full and just sum of \$ \_\_\_\_\_, lawful money of the United States of America, for the payment of which, well and truly to be made, we bind ourselves, our heirs, administrators, executors, successors, and assigns, jointly and severally, firmly by these presents.

The condition of the above obligation is that for two (2) years after the date the test section of HMA pavement located on \_\_\_\_\_, reference point \_\_\_\_\_ to reference point \_\_\_\_\_ is  
(Beginning Point) (Ending Point)  
completed and opened to unrestricted traffic; such warranty is to be in accordance with the Indiana Test Method 221 which is made a part of this bond for warranted test section of HMA pavement. If the principal satisfactorily fulfills the above condition, then this obligation shall be null and void; otherwise such obligation is to remain in full force and effect.

It is agreed that no modifications, omissions, or additions in or to the terms of the ITM 221 or the contract or in or to the plans or specifications shall affect the obligation of the surety on its bond.

In witness whereof, we hereunto set our hands and seal.

Name:

Name:

Address:

Address:

By:

Signature Surety

Title

By:

Signature Principle

Title

\_\_\_\_\_  
(Print or Typed) Surety

\_\_\_\_\_  
(Print of Typed) Principal

State of Indiana, County of \_\_\_\_\_ SS:

State of Indiana, County of \_\_\_\_\_ SS:

Personally appeared before me,

Personally appeared before me,

\_\_\_\_\_  
as surety and acknowledge the executions of the  
above bond

\_\_\_\_\_  
as surety and acknowledge the executions of the  
above bond

this \_\_\_\_\_ day of \_\_\_\_\_, 20 \_\_\_\_\_

this \_\_\_\_\_ day of \_\_\_\_\_, 20 \_\_\_\_\_

By

Signature

Notary Public

By

Signature

Notary Public

\_\_\_\_\_  
(Print of Typed) Notary

\_\_\_\_\_  
(Print of Typed) Notary

My Commission Expires \_\_\_\_\_, 20 \_\_\_\_\_

My Commission Expires \_\_\_\_\_, 20 \_\_\_\_\_

\_\_\_\_\_  
(County of Residence)

\_\_\_\_\_  
(County of Residence)



**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**FLAKINESS INDEX OF AGGREGATES  
ITM No. 224-21**

**1.0 SCOPE.**

- 1.1** This test method describes a procedure to determine the Flakiness Index (FI) of aggregates. The FI is used as a design input to determine the rate of application of aggregate and asphalt material for seal coats.
- 1.2** The flakiness index is a measure of the cubical shape of the aggregate which is needed for effective use of aggregates in seal coat applications.
- 1.3** This ITM may involve hazardous materials, operations, and equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and determining the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

- M 231 Weighing Devices Used in the Testing of Materials
- R 76 Reducing Samples of Aggregate to Testing Size
- T 11 Materials Finer Than 75  $\mu\text{m}$  (No. 200) Sieve in Mineral Aggregates by Washing
- T 27 Sieve Analysis of Fine and Coarse Aggregates

**2.2 ASTM Standards.**

**E11 Woven Wire Test Sieve Cloth and Test Sieves**

**2.3 ITM Standards.**

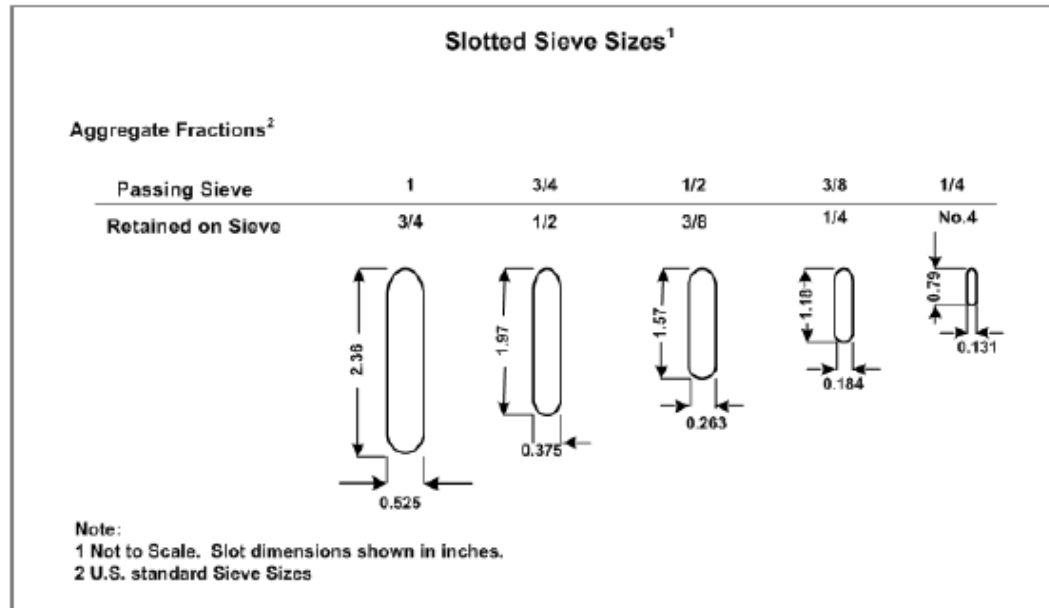
207 Sampling Stockpiled Aggregates

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specification, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate seal coat (SC) aggregates to meet the requirements of SC aggregates in 904.

## 5.0 APPARATUS.

- 5.1 Balance, Class G2, in accordance with AASHTO M 231
- 5.2 Sieves, in accordance with **ASTM E11**
- 5.3 Metal plate, approximately  $0.1175 \pm 0.0100$  in. thick with slotted openings conforming to the dimensions indicated in Figure 1 and the table below. Tolerances for the openings are indicated in the table below.



Opening Widths and Tolerances					
Passing Sieve	Retained on Sieve	Width (in.)	Width Tolerance (in.)	Length (in.)	Length Tolerance (in.)
1	3/4	0.525	0.0160	2.36	0.0630
3/4	1/2	0.375	0.0120	1.97	0.0590
1/2	3/8	0.263	0.0080	1.57	0.0430
3/8	1/4	0.184	0.0060	1.18	0.0310
1/4	No. 4	0.131	0.0040	0.79	0.0240

## 6.0 SAMPLE PREPARATION.

- 6.1 Obtain the sample in accordance with ITM 207
- 6.2 **Prepare two laboratory specimens**

- 6.3 Reduce **both test specimens** in accordance with AASHTO R 76 to the following minimum weight:

Size of Sample		
Aggregate Size	Minimum Weight, g	Maximum Weight, g
8	6000	8000
9	4000	6000
11	2000	---
12	1000	---
SC 11	2000	---
SC 12	1000	---
SC 16	2000	---

- 6.4 Determine the percentage of materials finer than the No. 200 sieve **for both test specimens #1 and #2** in accordance with AASHTO T 11
- 6.5 Determine the gradation of test specimen **#1** in accordance **with** AASHTO T 27. **The 1/4 in. (6.3 mm) sieve must be used.**

## 7.0 PROCEDURE.

- 7.1 For each size aggregate, determine the weight of the aggregate retained on the No. 4, 1/4 in., 3/8 in., 1/2 in., and 3/4 in. sieves, if applicable.
- 7.2 Determine the percentage of aggregate retained on each sieve to the nearest 0.1% by dividing the weight of the aggregate on each sieve by the total weight of the sample and multiplying by 100. Each size fraction present in the amount of 4 % or more will be tested. Size fractions containing less than 4% are not tested and shall be discarded.
- 7.3 Reduce **test specimen #2** in accordance with AASHTO R 76 to the following minimum size sample:

Size of <b>Test Specimen #2</b>	
Aggregate Size	Minimum Weight, g
8*	<b>3500</b>
9*	<b>1750</b>
11	<b>650</b>
12	<b>350</b>
SC 11	<b>650</b>
SC 12	<b>350</b>
SC 16	<b>650</b>

\* For No. 8 and No. 9 aggregate size, the test sample may be separated on the 3/8 in. sieve. The portion passing the 3/8 in. sieve may then be further reduced to a minimum of 200 g. This will reduce the number of particles to be evaluated. The flakiness index shall be determined on each portion and a weighted average percentage shall be calculated.

- 7.4** Determine the gradation of **test specimen #2** in accordance with AASHTO T 27. For aggregates retained on each applicable size sieve, each particle shall be tested by attempting to pass the particle through the appropriate slot for the size of material being tested.
- 7.5** The weight of the particles not passing the slot opening and the weight of the particles passing the same slot opening shall be determined to the nearest 0.1 g.

## **8.0 CALCULATIONS**

- 8.1** The FI shall be determined to the nearest whole percentage as follows:

$$\text{Flakiness Index (FI), \%} = \frac{\text{Total Weight Passing Slots}}{\text{Total Weight Retained on No. 4 Sieve}} \times 100$$

## **9.0 REPORT.**

- 9.1** The Flakiness Index shall be reported to the nearest whole number using the Flakiness Index Worksheet shown in Appendix A.



## FLAKINESS INDEX WORKSHEET

Sample ID #: \_\_\_\_\_ Source #: \_\_\_\_\_

Test Specimen #1 Gradation			Test Specimen #2 Gradation			
Sieve Size	Weight Retained, g	% of Total Sample	Slot Size	(A) Weight Not Passing Slot, g	(B) Weight Passing Slot, g	(C) Total Weight, g (A+B)
3/4 in.			1 to 3/4			
1/2 in.			3/4 to 1/2			
3/8 in.			1/2 to 3/8			
1/4 in.			3/8 to 1/4			
No. 4			1/4 to No. 4			
Total						

$$\text{Flakiness Index (FI), \%} = \frac{\text{Total Weight Passing Slots (Column B)}}{\text{Total Weight Retained on No. 4 Sieve (Column C)}} \times 100$$

Remarks: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Tested by: \_\_\_\_\_ Date: \_\_\_\_\_

**INDIANA DEPARTMENT OF TRANSPORTATION  
OFFICE OF MATERIALS MANAGEMENT**

**ACCEPTANCE PROCEDURES FOR AGGREGATE DRAINAGE LAYERS  
ITM No. 225-18**

**1.0 SCOPE.**

- 1.1** This method sets forth the acceptance procedures to be used when Aggregate Producers request that coarse aggregates be evaluated for use as Aggregate Drainage Layers under HMA pavement.
- 1.2** Aggregate Drainage Layer (ADL) aggregates are required to be both stable to allow adequate conditions for HMA placement and permeable to allow for adequate drainage of the pavement.
- 1.3** This method requires a Certified Aggregate Producer submit a candidate coarse aggregate to Office of Materials Management. The material will be tested for stability in accordance with AASHTO T 307, and for permeability in accordance with AASHTO T 215. If the results meet the requirements of this ITM, the material will be approved for use as an ADL material.
- 1.4** The gradation of the material submitted for testing will be established as the target gradation for the approved ADL material. The ADL material shall be controlled as a Quality Assurance material in accordance with ITM 211.
- 1.5** This procedure may involve hazardous materials, operations, and special equipment. All of the safety problems associated with the use of the test method may not be addressed herein. The user of the ITM is responsible for the establishment and implementation of appropriate safety and health practices and the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 AASHTO Standards.**

T 11	Materials Finer than 75 $\mu$ m (No. 200) Sieve in Mineral Aggregates by Washing
T 27	Sieve Analysis of Fine and Coarse Aggregates
T 89	Determining the Liquid Limit of Soils
T 90	Determining the Plastic Limit and Plasticity Index of Soils
T 215	Permeability of Granular Soils (Constant Head)
T 307	Determining the Resilient Modulus of Soils and Aggregate Materials

## 2.2 ITM Standards.

- 207 Sampling Stockpiled Aggregates
- 211 Certified Aggregate Producer Program

**3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

**4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate coarse aggregates for use in Aggregate Drainage Layers under HMA pavements.

## 5.0 APPARATUS.

**5.1** All apparatus will be in accordance with respective test method.

## 6.0 GENERAL REQUIREMENTS.

**6.1** Requests to have a coarse aggregate tested in accordance with this procedure shall be made by individual Aggregate Producers in writing to the Manager, Office of Materials Management. Information concerning the type of material and any applicable ledge numbers shall be included.

**6.2** The candidate coarse aggregate shall meet the following requirements:

**6.2.1** The aggregate shall be Class B or higher in accordance with 904.03(a). Aggregate classification will be based on materials of a similar origin supplied by the same Source.

**6.2.2** The gradation shall meet the following requirements:

Sieve Size	Percent Passing
1 1/2 in. (37.5 mm)	100
No. 4 (4.75 mm)	0-50
No. 200 (75 µm)	0-10

**6.2.3** Liquid limit shall be a maximum 25, in accordance with AASHTO T 89

**6.2.4** Plasticity Index shall be a maximum 5, in accordance with AASHTO T 90

**6.2.5** Permeability shall be a minimum 350 ft/day, in accordance with AASHTO T 215

**6.2.6** Permeability shall be a maximum 1000 ft/day, in accordance with AASHTO T 215

**6.2.7** Resilient Modulus shall be a minimum 15,000 psi, in accordance with AASHTO T 307

**6.3** Testing will be conducted by the Office of Materials Management and the Office of Geotechnical Services. In the event that these offices are unable to perform the testing, testing shall be conducted by an approved laboratory. In this case, the cost of shipping and testing of the coarse aggregate shall be the responsibility of the Aggregate Producer.

## **7.0 SAMPLING.**

**7.1** Sampling of the coarse aggregate shall be in accordance with ITM 207 in the presence of the Department.

**7.2** The samples shall be sufficient in quantity to yield a minimum of 200 lbs of material.

## **8.0 PROCEDURE**

### **8.1 Gradation Testing**

**8.1.1** Gradation testing will be performed in accordance with AASHTO T 27.

### **8.2 Liquid Limit and Plasticity Index**

**8.2.1** The Liquid Limit will be determined in accordance with AASHTO T 89.

**8.2.2** The Plasticity Index will be determined in accordance with AASHTO T 90.

### **8.3 Stability Testing**

**8.3.1** To determine the stability of the candidate aggregate, Resilient Modulus will be determined in accordance with AASHTO T 307.

### **8.4 Permeability Testing**

**8.4.1** Permeability testing will be performed in accordance with AASHTO T 215.

## **9.0 ACCEPTANCE CRITERIA.**

- 9.1** If the candidate coarse aggregate material meets the criteria in section 6.2, the aggregate will be approved for use as an Aggregate Drainage Layer aggregate.

## **10.0 ADDITIONAL REQUIREMENTS**

- 10.1** The gradation, as measured at the time of approval, shall be used as the established gradation for control as a Quality Assurance material in accordance with ITM 211, except the material shall be controlled on the following sieves:

**10.1.1** 1 ½" (37.5 mm)

**10.1.2** 1" (25.0 mm)

**10.1.3** ¾" (19.0 mm)

**10.1.4** ½" (12.5 mm)

**10.1.5** No. 4 (4.75 mm)

**10.1.6** No. 8 (2.36 mm)

**10.1.7** No. 30 (600 µm)

**10.1.8** No. 200 (75 µm)

- 10.2** The following tolerances shall be used for gradation control:

**10.2.1** Sieve size No. 8 (2.36 mm) and above =  $\pm 10\%$

**10.2.2** Sieve size No. 30 (600 µm) =  $\pm 6\%$

**10.2.3** Sieve size No. 200 (75 µm) =  $\pm 2\%$



## **INDIANA DEPARTMENT OF TRANSPORTATION DIVISION OF MATERIALS AND TESTS**

### **MATERIAL REQUIREMENTS FOR COARSE AGGREGATES USED IN CONCRETE ITM No. 226-25**

#### **1.0 SCOPE.**

- 1.1** This method sets forth the material requirements for coarse aggregates when used in concrete.
- 1.2** Coarse aggregates that have also been shown to be suitable to create optimized concrete mix designs in accordance with this ITM are called Concrete Coarse Aggregates (CCA).
- 1.3** This procedure may involve hazardous materials, operations, and special equipment. All the safety problems associated with the use of the test method may not be addressed herein. The user of the ITM is responsible for the establishment and implementation of appropriate safety and health practices and the applicability of regulatory limitations prior to use.

#### **2.0 REFERENCES.**

##### **2.1 ITM Standards.**

211 Certified Aggregate Producer Program

- 3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

- 4.0 SIGNIFICANCE AND USE.** This ITM shall be used to evaluate coarse aggregates for use in concrete mixtures, and coarse aggregate blends in concrete mixtures.

#### **5.0 GENERAL REQUIREMENTS.**

##### **5.1 Concrete Coarse Aggregate for Section 501**

- 5.1.1** Concrete Coarse Aggregate for Standard Specification Section 501 concrete may utilize any aggregate gradation that complies with the following:

Sieve Size	Percent Passing
1 1/2 in. (37.5 mm)	100
1 in. (25.0 mm)	95 - 100*
Decant (Stone and Slag)	0 - 2.5
Decant (All Others)	0 - 1.5

\*This requirement will not apply for approved Class AP aggregate with expansion values 0.030 or lower. Request to waive this requirement will be via the CMDS.

## 5.2 Concrete Coarse Aggregate for Section 309, 502, 506, and 702

**5.2.1** Concrete Coarse Aggregate for Standard Specification Sections 309, 502, 506, and 702 shall be one of the following gradations:

	INDOT #8	AASHTO #57
Sieve Size	Percent Passing	Percent Passing
1 1/2 in. (37.5 mm)	100	100
1 in. (25.0 mm)	100	95 - 100
3/4 in. (19.0 mm)	75 - 95	
1/2 in. (12.5 mm)	40 - 70	25 - 60
3/8 in. (9.5 mm)	20 - 50	
No. 4 (4.75 mm)	0 - 15	0 - 10
No. 8 (2.36 mm)	0 - 10	0 - 5
Decant (Stone and Slag)	0 - 2.5	0 - 2.5
Decant (All Others)	0 - 1.5	0 - 1.5

## 5.3 Intermediate-Sized Coarse Aggregate

**5.3.1** Intermediate-sized coarse aggregate shall be of the quality classification specified by the Division of Materials and Tests.

For an aggregate such as pea gravel, this determination will be made based on the quality classification of aggregates from the same pit.

## 5.4 Fine Aggregate

**5.4.1** Fine aggregate shall be in accordance with either 6.3 or 6.4 below.

## 6.0 PROCEDURE

### 6.1 Spreadsheet

**6.1.1** A spreadsheet has been developed for review of Concrete Coarse Aggregates by this ITM. The spreadsheet is posted on the Department Materials and Tests website.

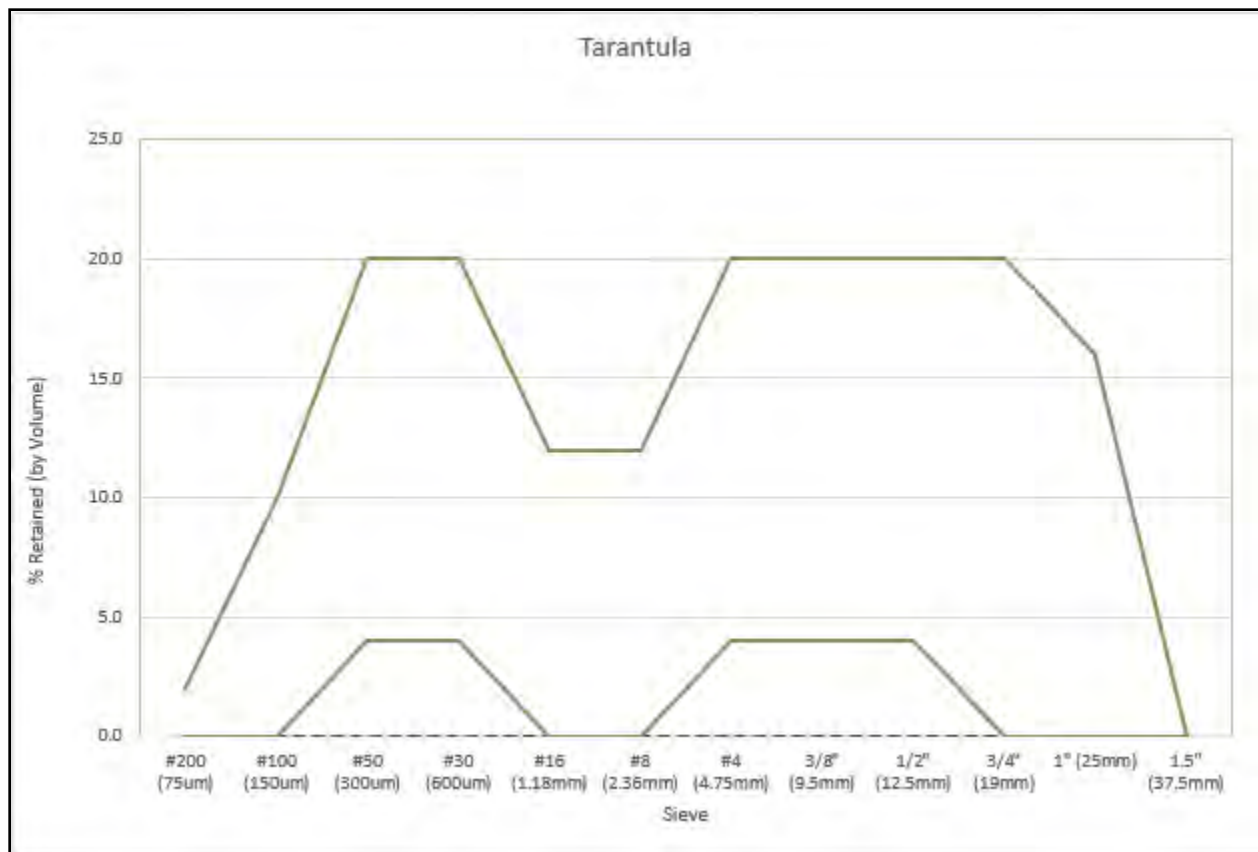
### 6.2 Tarantula Curve

**6.2.1** The tarantula curve is a tool used to evaluate aggregate proportioning for concrete mixtures. Evaluation is accomplished by evaluating individual percent retained on each sieve of the combined mixture gradation. The percent retained requirements are the tarantula curve are as follows:

	Individual Percent Retained by Volume (%)	
Sieve size	Min	Max
1-1/2 in. (37.5 mm)	0	0
1.00 in. (25 mm)	0	16
3/4 in. (19 mm)	0	20
1/2 in. (12.5 mm)	4	20
3/8 in. (9.5 mm)	4	20
No. 4 (4.75 mm)	4	20
No. 8 (2.36 mm)	0	12
No. 16 (1.18 mm)	0	12
No. 30 (600 µm)	4	20
No. 50 (300 µm)	4	20
No. 100 (150 µm)	0	10
No. 200 (75 µm)	0	2

**Figure 1**





**Figure 2**

### **6.3 Coarse and Fine Sand Limits**

**6.3.1** Coarse Sand is defined as the total of the material retained on the #8, #16, and #30 sieves.

**6.3.2** Fine Sand percentage is defined as the total of the material retained on the #30, #50, #100, and #200 sieves.

**6.3.3** Utilizing the same four trial aggregate blends developed in 6.2, coarse and fine sand limits will be evaluated by the following criteria:

- a) Coarse sand percentage shall be 20% or greater.
- b) Fine sand percentage shall be between 25% and 34%.

### **6.4 Fine Aggregate Limits for Mixes not using 6.2 above**

**6.4.1** For normal weight concrete the fine aggregate shall be no less than 40% nor more than 45% of the total weight of aggregates used, except the limit may be increased to 50% when slag coarse aggregate is used. The aggregates shall be proportioned to use the maximum amount of coarse aggregate

which produces a workable mix. For lightweight concrete, the fine aggregate proportioning limits will not apply.

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**INDIANA DEPARTMENT OF TRANSPORTATION  
DIVISION OF MATERIALS AND TESTS**

**VERIFYING SIEVES  
ITM No. 902-22**

**1.0 SCOPE**

- 1.1** This test method covers the procedure for verifying the physical condition of laboratory testing sieves ranging in size from 4 in. to No. 200.
- 1.2** Two procedures are included in this test method: verifying with calipers and verifying with a go-no go gauge. The Department will use the verifying with calipers method. Industry may utilize the procedure with a go-no go gauge upon approval of the Department.
- 1.3** This ITM may involve hazardous materials, operations, equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and to determine the applicability of regulatory limitations prior to use.

**2.0 REFERENCES.**

**2.1 ASTM Standards.**

E11 Woven Wire Test Sieve Cloth and Test Sieves

- 3.0 TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

- 4.0 SIGNIFICANCE AND USE.** This ITM is used by laboratory personnel to verify the physical condition of testing sieves.

**5.0 APPARATUS.**

- 5.1** Calipers, readable to 0.01 mm and having a jaw depth allowing the blades to stay perpendicular to the screen throughout the measurements.
- 5.2** [Optional] Go-No Go Gauge, conforming to the tolerances shown in Table 1.

**6.0 PROCEDURE.**

**6.1 No. 4 Sieves and Coarser using Calipers.**

- 6.1.1** Record the sieve identification, manufacturer, opening size and diameter.

- 6.1.2 Hold the sieve against a uniformly illuminated background. Check the general condition of the sieve for cracks in frame, broken solder joints, wire tightness, and irregular openings.
- 6.1.3 Select two perpendicular fields of five openings each for verification. (Appendix A - Figure 1)
- 6.1.4 Using the calipers, measure and record the openings at their vertical (Y) and horizontal (X) midpoints (Appendix A - Figure 2). Keep the X and Y components separate and calculate the average of all 10 X measurements and all 10 Y measurements.

## 6.2 No. 4 Sieves and Coarser With Go-No Go Gauge.

- 6.2.1 Specific procedures for checking sieves with a go-no go gauge shall be developed by the Industry end user and included as a part of their Quality Control Plan, subject to approval by the Department.

## 6.3 Sieves Finer than No. 4.

- 6.3.1 Record the sieve identification, manufacturer, opening size and diameter.
- 6.3.2 Hold the sieve against a uniformly illuminated background. Check and record the general condition of the sieve for cracks in frame, broken solder joints, weaving defects, creases, wrinkles, wire tightness, and irregular openings.

## 7.0 TOLERANCE.

- 7.1 **No. 4 Sieves and Coarser.** The maximum individual opening and average opening for each sieve shall not exceed the sieve tolerances of Table 1. If the tolerances of Table 1 are exceeded or there are general physical condition deficiencies as noted in 6.1.2, the sieve shall be replaced.
- 7.2 **Sieves Finer than No. 4.** If there are general physical condition deficiencies as noted in 6.3.2, the sieve shall be replaced.

**SIEVE TOLERANCES  
TABLE 1**

<b>Sieve Designation</b>	<b>Permissible Average Opening</b>	<b>Maximum Individual Opening</b>
100 mm (4 in.)	(97.35 - 102.65) mm	103.44 mm
90 mm (3-1/2 in.)	(87.61 - 92.39) mm	93.18 mm
75 mm (3 in.)	(73.00 - 77.00) mm	77.78 mm
63 mm (2-1/2 in.)	(61.31 - 64.69) mm	65.44 mm
50 mm (2 in.)	(48.66 - 51.34) mm	52.06 mm
37.5 mm (1-1/2 in.)	(36.49 - 38.51) mm	39.17 mm
25 mm (1.00 in.)	(24.32 - 25.68) mm	26.24 mm
19 mm (3/4 in.)	(18.48-19.52) mm	20.01 mm
12.5 mm (1/2 in.)	(12.15 - 12.85) mm	13.25 mm
9.5 mm (3/8 in.)	(9.24 - 9.76) mm	10.11 mm
6.3 mm (1/4 in.)	(6.12 – 6.48) mm	6.76 mm
4.75 mm (No. 4)	(4.62 - 4.88) mm	5.12 mm

Tolerances for sieves not in Table 1 may be found in ASTM E11

## SIEVE VERIFICATION ITM 902

Sieve Identification: \_\_\_\_\_ Manufacturer: \_\_\_\_\_ Opening Size: \_\_\_\_\_

Frame Diameter: \_\_\_\_\_ Calipers (if used): \_\_\_\_\_

General Physical Condition			
No. 4 Sieves or Coarser	√	Sieves Finer than No. 4	√
The frame is not cracked		The frame is not cracked	
The welds are not broken		The welds are not broken	
The wires are tight		No weaving defects, creases, or wrinkles	
No irregular openings		The screen is tight	
		No irregular openings	

Opening Verification No. 4 and Coarser				
	Field 1		Field 2	
	X	Y	X	Y
1				
2				
3				
4				
5				

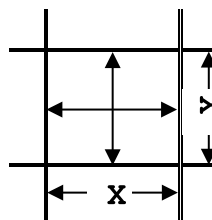
Average X = \_\_\_\_\_

Average Y = \_\_\_\_\_

**Figure 1**

		1					
			1			2	
				1	2		
				2	1		
			2			1	
		2					

**Figure 2**



**Does the X or Y component exceed the maximum individual opening given in Table 1? (Y or N)**

**Does the X or Y average exceed the permissible average opening given in Table 1? (Y or N)**

Remarks: \_\_\_\_\_

Verified by: \_\_\_\_\_

Date: \_\_\_\_\_

Next Due Date: \_\_\_\_\_



## INDIANA DEPARTMENT OF TRANSPORTATION DIVISION OF MATERIALS AND TESTS

### VERIFYING MECHANICAL SHAKERS ITM No. 906-24

#### 1.0 SCOPE.

- 1.1 This test method covers the procedure for verifying the sieving sufficiency of mechanical shakers and the accuracy of timers used in the sieve analysis of aggregates.
- 1.2 This ITM may involve hazardous materials, operations, equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and to determine the applicability of regulatory limitations prior to use.

#### 2.0 REFERENCES.

##### 2.1 ITM Standards.

902 Verifying Sieves

- 3.0 **TERMINOLOGY.** Definitions for terms and abbreviations shall be in accordance with the Department's Standard Specifications, Section 101.

- 4.0 **SIGNIFICANCE AND USE.** This ITM is used by laboratory personnel to verify the sieving sufficiency of mechanical shakers and the accuracy of timers used in the sieve analysis of aggregates.

#### 5.0 APPARATUS.

- 5.1 Balance, readable to 0.1 g
- 5.2 Timing Device consisting of either: NIST Traceable Timer, or Android/iPhone with a stopwatch function/app, readable to 1 s
- 5.3 Sieves, verified in accordance with ITM 902
- 5.4 Sieve pan and lid

## **6.0 PROCEDURE.**

### **6.1 Timer.**

- 6.1.1** Operate the mechanical shaker with the timer set at 5 min, and measure the time using a Timing Device in accordance with 5.2.
- 6.1.2** Repeat 6.1.1 with the timer set at 10 min and 15 min.
- 6.1.3** If the timer is not within the allowable tolerance of 8.1, the manufacturers markings shall not be used, and accurate settings on the shaker shall be established by trial and error determination.

### **6.2 Shakers using 8 in. and 12 in. diameter sieves.**

- 6.2.1** Determine and record an initial sample weight.
- 6.2.2** Insert sieves No. 4 through No. 200 for fine aggregates or 1 in. through No. 200 for coarse or dense graded aggregates into the shaker.
- 6.2.3** Shake sample mechanically for 15 min for sands or 10 min for blended aggregates.
- 6.2.4** Place the first sieve retaining material on a pan and cover the sieve with the lid.
- 6.2.5** Hand shake the first sieve for 1 min by holding the sieve in a slightly inclined position in one hand and striking the side of the sieve sharply and with an upward motion against the heel of the other hand at approximately 150 times per min. The sieve should be turned about 1/6 of a revolution at intervals of about 25 strokes. For sieves larger than the No. 4 sieve, the material on the sieve should be limited to a single layer of particles.
- 6.2.6** Weigh the material passing the sieve and retained in the pan.
- 6.2.7** Weigh the material retained on the sieve.
- 6.2.8** Add the weight retained on the sieve and weight passing the sieve, and verify the sieve was not overloaded in accordance with Table 1. If the sieve was overloaded, verification is void, and a new sample shall be obtained.



Screen Size	Standard 15 in. x 23 in.	Standard 14 in. x 14 in.	12 in. Diameter	8 in. Diameter
3 in.	40.5 kg	23.0 kg	12.6 kg	-----
2 in.	27.0 kg	15.3 kg	8.4 kg	3.6 kg
1 1/2 in.	20.2 kg	11.5 kg	6.3 kg	2.7 kg
1 in.	13.5 kg	7.7 kg	4.2 kg	1.8 kg
3/4 in.	10.2 kg	5.8 kg	3.2 kg	1.4 kg
1/2 in.	6.7 kg	3.8 kg	2.1 kg	890 g
3/8 in.	5.1 kg	2.9 kg	1.6 kg	670 g
No. 4	2.6 kg	1.5 kg	800 g	330 g
8 in. diameter sieves: No. 8 to No. 200 shall not exceed 200 g/sieve				
12 in. diameter sieves: No. 8 to No. 200 shall not exceed 450 g/sieve				

**APPROXIMATED SIEVE OVERLOAD  
TABLE 1**

**6.2.9** Repeat 6.2.5 through 6.2.8 on all remaining sieves.

**6.2.10** If a sieve does not meet the allowable tolerance of 8.2, the shaking time shall be increased to determine an adequate time.

**6.3 Shakers using 15 in. x 23 in., 14 in. x 14 in., or other size sieves.**

**6.3.1** Determine and record an initial sample weight of an aggregate having a nominal maximum aggregate size of 1 in.

**6.3.2** Insert sieves 1 in. through No. 8 into the shaker.

**6.3.3** Shake sample mechanically for 5 min.

**6.3.4** Remove the first sieve retaining material, determine the weight of material retained, and verify that the sieve was not overloaded in accordance with Table 1. If the sieve was overloaded, verification is void, and a new sample shall be obtained.

**6.3.5** Place the material on a 8 in. or 12 in. diameter sieve of equivalent opening size in increments that will not overload the sieve in accordance with Table 1. Place the sieve on a pan and cover the sieve with the lid.

**6.3.6** Handshake for one min as described in 6.2.5. Continue until all material has been introduced onto the 8 in. or 12 in. sieve.

**6.3.7** Weigh the accumulated material passing the sieve and retained in the pan.

**6.3.8** Repeat 6.3.4 through 6.3.7 for all remaining sieves.

**6.3.9** If a sieve does not meet the allowable tolerance of 8.2, the shaking time shall be increased to determine an adequate time.

**7.0 CALCULATIONS.** The percent passing a sieve by hand shaking after mechanical shaking is calculated by the following formula:

$$\% \text{ Passing} = \frac{W_1}{W_2} \times 100$$

where:

$W_1$  = weight (mass) of sample passing a sieve by hand shaking, g

$W_2$  = initial sample weight (mass), g

**8.0 TOLERANCE.**

**8.1** The timer of the mechanical shaker shall be within  $\pm 5$  s at 5 min,  $\pm 10$  s at 10 min, and  $\pm 15$  s at 15 min of the stopwatch reading.

**8.2** After mechanical shaking, no more than 0.5 percent by weight of the total sample shall pass any sieve after 1 min of hand sieving.

**9.0 REPORT.** The timing and sieving sufficiency verification shall be reported on the form in Appendix A.

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Appendix A

**MECHANICAL SHAKER AND TIMER  
VERIFICATION  
ITM 906**

**SHAKER IDENTIFICATION**

Manufacturer: \_\_\_\_\_

Model. No.: \_\_\_\_\_ Serial No.: \_\_\_\_\_

**VERIFICATION EQUIPMENT USED**

Balance: \_\_\_\_\_ Have sieves been verified using ITM 902? \_\_\_\_\_

Timer ID (if NIST): \_\_\_\_\_ or Android / iPhone used (circle one)

**TIMER VERIFICATION**

Setting on Shaker Timer	Timing Device Reading	Corrective Adjustment Made
5		
10		
15		

**SIEVING SUFFICIENCY VERIFICATION**

Frame Dimensions: \_\_\_\_\_ Mechanical Sieving Time: \_\_\_\_\_

Total Sample Weight: \_\_\_\_\_

Sieve Size	Weight Retained by Mechanical Sieving	Weight Passing After Hand Sieving	% Passing After Hand Sieving
1 in.			
3/4 in.			
1/2 in.			
3/8 in.			
No. 4			
No. 8			
No. 16			
No. 30			
No. 50			
No. 100			
No. 200			

Remarks: \_\_\_\_\_

Verified by: \_\_\_\_\_

Date: \_\_\_\_\_ Next Due Date: \_\_\_\_\_



## INDIANA DEPARTMENT OF TRANSPORTATION DIVISION OF MATERIALS AND TESTS

### VERIFYING BALANCES ITM No. 910-22

#### 1.0 SCOPE.

- 1.1** This test method covers the procedures for verifying the accuracy and off-center error of balances.
- 1.2** This ITM may involve hazardous materials, operations, equipment and may not address all of the safety problems associated with the use of the test method. The user of the ITM is responsible for establishing appropriate safety and health practices and to determine the applicability of regulatory limitations prior to use.

#### 2.0 REFERENCES.

##### 2.1 AASHTO Standards.

M 231 Weighing Devices Used in the Testing of Materials

#### 3.0 TERMINOLOGY. Definitions for terms and abbreviations will be in accordance with the Department's Standard Specifications, Section 101 and the following:

- 3.1** Accuracy. The degree of conformity of a measurement with the true value of the quantity measured.
- 3.2** Off-Center Errors. The differences in indicated weight when a sample weight is shifted to various positions on the weighing area of the sample pan.
- 3.3** National Institute of Standards and Technology (NIST). A federal technology agency that develops and applies technology, measurements, and standards.

#### 4.0 APPARATUS.

- 4.1** Balance, a Class G2, G5, or G20 in accordance with AASHTO M 231.

Class	Readability and Sensitivity	Accuracy <sup>a</sup>
G2	0.1 g	0.2 g or 0.1 %
G5	1 g	2 g or 0.1 %
G20	5 g	5 g or 0.1 %

<sup>a</sup> Accuracy equal to the mass stated or 0.1 % of the test load, whichever is greater, throughout the range of use

**4.2** A set of weights, up to the capacity of the balance, with sufficient subdivisions of weight so that increments of approximately 10 percent of the capacity **of the balance is measured**. The weights shall be a minimum ASTM Class 3 for use on Class G2 or G5 balances and a minimum of NIST Class F for use on Class G20 balances. The Class 3 or Class F weights shall have a calibration report indicating traceability to NIST. The weights shall be calibrated at a minimum frequency of once each 12 months.

**4.3** Thermometer, room temperature, with a resolution of at least **1°C** (2°F).

**5.0 SIGNIFICANCE AND USE.** This ITM is used by laboratory personnel to verify the accuracy and off-center error of balances.

## **6.0 PROCEDURE.**

**6.1 General.** Use the balance in the manner recommended by the manufacturer for each step of the verification procedures.

### **6.2 Accuracy.**

**6.2.1** Clean the balance and standard weights with a lint free dry cloth.

**6.2.2** Place the standard weights near the instrument.

**6.2.3** Allow the balance and the weights to stabilize to the ambient working temperature.

**6.2.4** Place the thermometer on the bench near the balance and record the temperature.

**6.2.5** Place the standard weight(s) in the center of the balance pan in increasing increments of approximately 10 percent of the capacity and record the indications. If possible, the weights should be carefully stacked upon each other.

### **6.3 Off-Center Error.**

(Note 1: Off-Center Error verification may be waived for balances when the weights cannot be placed directly on the pan of the balance. Example: Ignition ovens.)

**6.3.1** Place the standard weight(s) equal to approximately 50 percent of the capacity of the balance on the center of the sample pan and record the indication.

- 6.3.2** Place the same standard weight(s) on each corner of the sample pan and record the indication. For balances with circular pans place the weight(s) toward the edge at 12, 3, 6, and 9 o'clock.
- 6.3.3** Calculate the off center percent error using difference from the weight obtained in 6.3.1 as long as the reading obtained at the center is within the accuracy tolerance.

## **7.0 TOLERANCES.**

### **7.1 G2 Balance.**

- 7.1.1** Within any interval equal to approximately 10 percent of the capacity of the balance, the accuracy shall be equal to 0.2 g or 0.1 percent of the test load, whichever is greater.
- 7.1.2** The maximum off-center error shall be equal to or less than 0.2 g or 0.1 percent of the test load, whichever is greater.

### **7.2 G5 Balance.**

- 7.2.1** Within any interval equal to approximately 10 percent of the capacity of the balance, the accuracy shall be equal to 2 g or 0.1 percent of the test load, whichever is greater.
- 7.2.2** The maximum off-center error shall be equal to or less than 2 g or 0.1 percent of the test load, whichever is greater.

### **7.3 G20 Balance.**

- 7.3.1** Within any interval equal to approximately 10 percent of the capacity of the balance, the accuracy shall be equal to 5 g or 0.1 percent of the test load, whichever is greater.
- 7.3.2** The maximum off-center error shall be equal to or less than 5 g or 0.1 percent of the test load, whichever is greater.

## **8.0 REPORT.** The accuracy and off-center error are reported on the form in Appendix A.

< < The remainder of this page is intentionally blank > >

# **BALANCE VERIFICATION** **ITM 910**

Customer Name & Address: \_\_\_\_\_

<p><b>Balance</b></p> <p>Comm. #: _____</p> <p>Class: _____</p> <p>Model #: _____</p>	<p><b>Standard Weights</b></p> <p>Comm. #: _____</p> <p>Class: _____</p> <p>Date Calibrated: _____</p>
---	--

Temperature: \_\_\_\_\_

Weight Applied (A)	Indication on Balance (B)	Percent of Error [(A - B) / A] x 100

Location	Weight Applied	Indication on Balance	Percent Error from Center [(A - B) / A] x 100
Center		A	
Off Center 1		B	
Off Center 2		B	
Off Center 3		B	
Off Center 4		B	

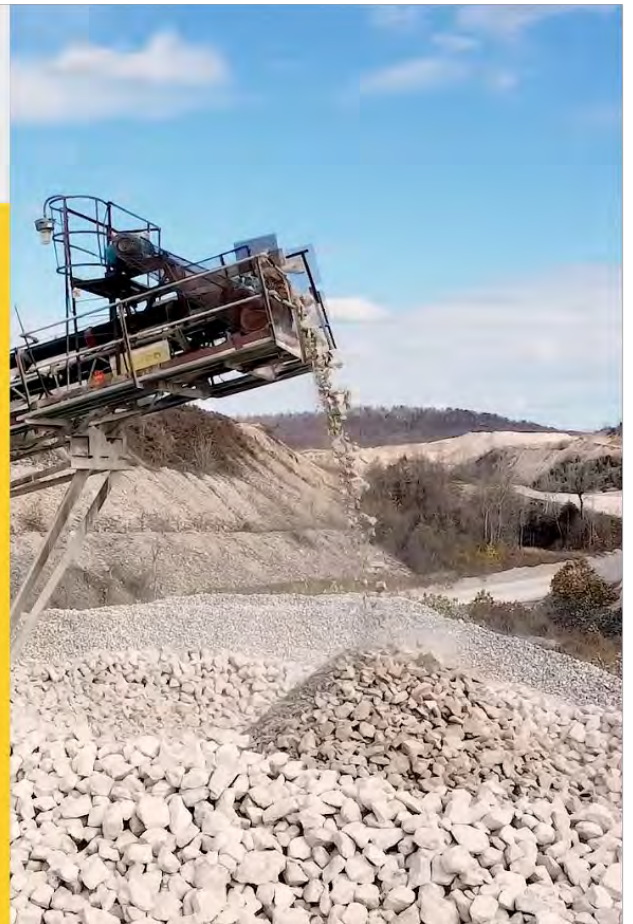
Remarks: \_\_\_\_\_

Verified by: \_\_\_\_\_ Date: \_\_\_\_\_

Next Due Date: \_\_\_\_\_

# INDIANA STANDARD SPECIFICATIONS

## CHAPTER 15





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# STANDARD SPECIFICATIONS REFERENCE

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**SECTION 211 – B BORROW AND STRUCTURE BACKFILL**

**SECTION 301 – AGGREGATE BASE**

**SECTION 302 – SUBBASE**

**SECTION 303 – AGGREGATE PAVEMENTS OR SHOULDERS**

**SECTION 904 - AGGREGATES**

- Floors, roadways, railings, bottom chords, shoes, and seats of bridges shall be cleaned of rubbish, sand, stone, gravel, and dirt. Waterways shall be left unobstructed.
- 25 Culverts and other drainage structures shall be left clean for their entire length.

- If the contract is for construction of a new pavement or for grading and structures only on a right-of-way acquired for divided highway construction, one roadway of which has been constructed and on which the Department has assumed normal
- 30 maintenance, the provisions of this specification shall not apply to the maintained portion except to those areas of such which are disturbed by the operations.

- If the contract is for construction of a portion of a divided highway on a right-of-way on which no previous construction has been done or on a divided highway
- 35 right-of-way on which a previous grading-only contract has been completed, then the provisions of this specification shall apply to the entire right-of-way for the full contract length.

- Unless otherwise set out in the special provisions for a contract which includes
- 40 work for patching, widening, resurfacing, surface treating, undersealing, or for a combination of these, or for a contract through which traffic is being maintained during construction, these requirements will apply only to that portion of the right-of-way disturbed by the operations.

45 **210.03 Method of Measurement**

Final trimming and cleaning will not be measured for payment unless otherwise provided.

50 **210.04 Basis of Payment**

Final trimming and cleaning will not be paid for directly. The cost thereof shall be included in the cost of other pay items.

## **SECTION 211 – B BORROW AND STRUCTURE BACKFILL**

### **211.01 Description**

- This work shall consist of backfilling excavated or displaced peat deposits; filling
- 5 up to designated elevations of spaces excavated for structures and not occupied by permanent work; constructing bridge approach embankment; and filling over structures and over arches between spandrel walls, all with special material.

## **MATERIALS**

10

### **211.02 Materials**

Materials shall be in accordance with the following:

- 15 B Borrow .....904.06
- Flowable Backfill .....213

Geotextile.....	918.02
Structure Backfill .....	904.05

Aggregate for end bent backfill shall be No. 8 or No. 9 crushed stone or ACBF,  
 20 Class D or higher.

The Contractor has the option of either providing B borrow or structure backfill  
 from an established CAPP source or supplying the material from another source. The  
 Contractor has the following options for supplying B borrow or structure backfill from  
 25 a local site:

- (a) the establishment of a CAPP Producer Yard at the  
 local site in accordance with 917 or
- 30 (b) use a CAPP Certified Aggregate Technician or a  
 consultant on the list of Qualified Geotechnical  
 Consultants for Gradation Control Testing.

For material excavated within the project limits, gradation control testing will be  
 35 performed by the Department if the Contractor is directed to use the material as  
 B borrow or as structure backfill.

The frequency of gradation control testing shall be one test per 2,000 t based on  
 production samples into a stockpile or by over the scales measurement, with a  
 40 minimum of two tests per contract, one in the beginning and one near the mid-point.  
 The sampling and testing of these materials shall be in accordance with applicable  
 requirements of 904 for fine and coarse aggregates.

The Contractor shall notify the Engineer in writing of the plan to measure the  
 45 material.

## CONSTRUCTION REQUIREMENTS

### 211.03 General Requirements

50 If B borrow or structure backfill is obtained from borrow areas, the locations,  
 depths, drainage, and final finish shall be in accordance with 203.

Unless otherwise specified, if excavated material complies with 211.02 and if  
 B borrow or structure backfill is required for special filling, the excavated material  
 55 shall be used as such. If there is a surplus of this material, such surplus shall be used  
 in embankment. The provisions of 203.19 shall apply to placing this material at  
 structures. All surplus in excess of the directed or specified use on the right-of-way  
 shall be disposed of in accordance with 201.03.

60 If fill or backfill as described in this specification is within embankment limits,  
 and if it is not required that the entire fill or backfill be of B borrow and placed as such,

then that portion above the free-water level shall be placed in accordance with applicable provisions of 203 and compacted to the required density.

65 If borrow is required outside the specified limits of B borrow, material in accordance with the specifications for B borrow may be furnished at the contract unit price for borrow. However, the quantity of borrow measured for payment outside the limits of structure backfill will not exceed the theoretical quantity of B borrow furnished.

70

Unless otherwise specified, all spaces excavated for and not occupied by bridge abutments and piers, if within embankment limits, shall be backfilled to the original ground line with B borrow, and placed in accordance with 211.04.

75 Where B borrow or structure backfill is required as backfill at culverts, retaining walls, sewers, manholes, catch basins, and other miscellaneous structures, it shall be compacted in accordance with 211.04. B borrow consisting of ACBF or GBF shall not be used within 2 ft of the free water level.

80 Where specified, aggregate for end bent backfill shall be placed behind end bents and compacted in accordance with 211.04. Prior to placing the aggregate, a geotextile shall be installed in accordance with 616.11.

### **211.03.1 Structure Backfill Types**

85 The structure backfill type shall be as specified.

Within each of the following structure backfill types, the Contractor shall choose from the listed options for each type:

#### **90 (a) Type 1**

1. Structure backfill in accordance with 904.05.
2. Non-removable or removable flowable backfill in

95

accordance with 213.

#### **(b) Type 2**

1. Crushed stone aggregate or ACBF structure backfill in

100

accordance with 904.05, except No. 30, No. 4, and 2 in. nominal size aggregate shall not be used.

2. Non-removable or removable flowable backfill in

105

accordance with 213.

#### **(c) Type 3**

Structure backfill nominal size aggregates 1 in. or 1/2 in., or coarse aggregate No. 5, No. 8, No. 9, No. 11, or No. 12 in accordance with 904.05 shall be used.

- 110 AASHTO No. 57 aggregates that comply with 904 and with the coarse aggregate requirements in accordance with 904.05 may also be used.

If ACBF is used, it shall only be used in sizes that meet the size requirements for coarse aggregate No. 5, No. 8, or AASHTO No. 57.

115

A Type A certification in accordance with 916 shall be provided for the additional structure backfill.

The results of the following shall be shown on the certification.

120

Property	Criteria	Test Method
pH (Note 1)	$5 < \text{pH} < 10$	AASHTO T 289
Organic Content (Note 2)	1% max.	AASHTO T 267
Permeability, min. (Note 3)	30 ft/day	AASHTO T 215
Notes: 1. One pH test is required for each bench of stone, each source of air-cooled blast furnace slag, and each source of gravel.  2. One organic content test is required for each source of gravel.  3. One permeability test is required for the smallest aggregate size from each source. Sizes No. 5, No. 8, No. 9, and AASHTO No. 57 do not require a permeability test.		

The gradation shall be performed on the material used in the permeability test. Testing for permeability shall be performed on the sample of the material compacted to 95% in accordance with AASHTO T 99, Method C or D. All of the tests listed above shall be performed a minimum of once every 12 months per source.

125

In addition to the criteria above, structure backfill for retaining wall systems containing metal components in contact with structure backfill shall also be in accordance with the following criteria:

130

Property	Criteria	Test Method
Chlorides	$< 100 \text{ ppm}$	AASHTO T 291
Sulfates	$< 200 \text{ ppm}$	AASHTO T 290
Resistivity, min.	$3,000 \Omega\text{-cm}$	AASHTO T 288
Internal friction angle, $\phi$ , min.	$34^\circ$	AASHTO T 236* or AASHTO T 297*
* under consolidated drained conditions		

If the minimum resistivity exceeds  $5,000 \Omega\text{-cm}$ , the requirement for the testing of

chlorides and sulfates will be waived. The resistivity shall be tested at 100% saturation. All of the tests listed above shall be run a minimum of once every 12 months per source. The Department's Division of Materials and Tests will evaluate the material from each source and determine the appropriate tests to be performed.

Testing for  $\phi$  shall be performed using a sample of the material compacted to 95% in accordance with AASHTO T 99, Method C or D. Testing for  $\phi$  will not be required when using coarse aggregate No. 5, No. 8, No. 9, or AASHTO No. 57.

**(d) Type 4**

Removable flowable backfill in accordance with 213.

**(e) Type 5**

Non-removable flowable backfill in accordance with 213.

**211.04 Compaction**

B borrow and structure backfill Type 1, Type 2, and Type 3 shall be compacted with mechanical tamps or vibrators in accordance with the applicable provisions of 203.23 except as otherwise set out herein.

Aggregate for end bent backfill and coarse aggregate used for structure backfill shall be deposited in layers not to exceed 12 in. loose measurement. Each layer shall be mechanically compacted with a compactor having a plate width of 17 in. or larger that delivers 3,000 to 9,000 lb per blow. Each lift shall be compacted with two passes of the compactor.

**211.05 Embankment for Bridges**

When special filling is required, the embankment for bridges shall be constructed using B borrow within the specified limits shown on the plans. All embankment construction details specifically set out in this specification for embankment for bridges shall be considered in accordance with the applicable requirements of 203.

At the time B borrow is being placed for approach embankment, a well compacted watertight dam shall be constructed in level lifts, the details of which are shown on the plans. Except as hereinafter specified for material to be used in constructing the enclosing dam, and for growing vegetation, and unless otherwise provided, the material for constructing bridge approach embankment shall be B borrow compacted by mechanical methods.

If approach embankment or shoulders are constructed of material not suitable for growing seed or sod, and if one or both of these is required, then such areas shall, unless otherwise specified, be covered with a layer of clay, loam, or other approved material. This layer shall be approximately 1 ft thick after being compacted into place.

**211.06 B Borrow Around Bents**

When specified, B borrow shall be placed around all bents falling within the limits

of the approach grade as shown on the plans. Before placing, the surface of the ground  
 180 on which it is to be placed shall be scarified or plowed as directed. The embankment  
 slope shall be 2:1 on the sides and beneath the structure, and shall be 6:1 from the end  
 of the bridge down to the average ground line, or it may be required to complete the  
 approaches back to the existing grade. An enclosing dam and provisions for growing  
 vegetation shall be constructed in accordance with 211.05.

185

### **211.07 Blank**

### **211.08 Spandrel Filling**

Unless otherwise specified, spandrel fills for arch structures shall be composed of  
 190 B borrow. The fill shall be carried up symmetrically in lifts from haunch to crown and  
 simultaneously over all piers, abutments, and arch rings. Compaction shall be in  
 accordance with 211.04.

### **211.09 Method of Measurement**

195 B borrow, structure backfill Type 1, Type 2, or Type 3, and aggregate for end bent  
 backfill will be measured by the cubic yard as computed from the neat line limits  
 shown on the plans. If cubic yards are set out as the pay item for B borrow or structure  
 backfill in the Schedule of Pay Items and if neat line limits are not specified for  
 measurement of volume for the material, measurement will be made by the cubic yard  
 200 at the loading point in truck beds which have been measured, stenciled, and approved.

The B borrow may be weighed and converted to cubic yards by assuming the  
 weight per cubic foot to be 90% of the maximum wet density in accordance with  
 AASHTO T 99. The material may be cross-sectioned in its original position and again  
 205 after excavation is complete, and the volume computed by the average end area  
 method.

If B borrow is used for backfill in areas where unsuitable material is present or  
 peat excavation has been performed, unless otherwise directed, the B borrow will be  
 210 cross-sectioned, and the volume will be computed by the average end area method.

Structure backfill Type 4 or Type 5 will be measured by the cubic yard as  
 computed from the neat line limits shown on the plans. If neat line limits are not shown  
 on the plans, the volume in cubic yards of flowable backfill furnished and placed as  
 215 structure backfill Type 4 or Type 5 will be computed from the nominal volume of each  
 batch and a count of the batches. Unused and wasted flowable backfill will be  
 estimated and deducted.

If the material is to be paid for by the ton, it shall be weighed in accordance with  
 220 109.01(b).

If the material comes from a wet source such as below water or a washing plant,  
 and weighing is involved in the method of measurement, there shall be a 12 h drainage  
 period prior to the weighing.

225

Geotextile will be measured in accordance with 616.12.

### **211.10 Basis of Payment**

230 The accepted quantities of B borrow will be paid for at the contract unit price per cubic yard or per ton as specified, complete in place.

Structure backfill will be paid for at the contract unit price per cubic yard of the type specified, provided the material comes from outside the permanent right-of-way.

235 B borrow material placed outside the neat lines will be paid for as borrow when such B borrow eliminates required borrow material. Otherwise, no payment will be made for backfill material placed outside the neat lines.

240 Aggregate for end bent backfill will be paid for at the contract unit price per cubic yard, based on the neat line limits shown on the plans.

Geotextile will be paid for in accordance with 616.13.

245 If topsoil, loam, or other suitable material in accordance with 211.05 is used for expediting the growth of seed or sod, it will be paid for at the contract unit price per cubic yard for borrow, unless otherwise provided.

Payment will be made under the following:

250	<b>Pay Item</b>	<b>Pay Unit Symbol</b>
	Aggregate for End Bent Backfill .....	CYS
	B Borrow .....	CYS
		TON
255	Structure Backfill, Type _____ .....	CYS

260 No payment will be made under this section for material obtained within the excavation limits of the project if the Contractor is directed to use the material as B borrow or structure backfill in a pipe trench, culvert, construction of an embankment or fill, or if the Contractor uses the material for its own convenience. Material obtained from within the excavation limits of the project and which the Contractor is directed to use as B borrow or structure backfill for other purposes including replacement of undercut areas, support for a retaining wall system, and end bent fill will be paid for at the contract unit price of \$5.00 per cubic yard for B borrow/structure backfill handling.

265

The cost of disposal of excavated material shall be included in the cost of the pay items in this section.



## DIVISION 300 – AGGREGATE PAVEMENT AND BASES

### SECTION 301 – AGGREGATE BASE

#### 301.01 Description

5 This work shall consist of placing coarse aggregate on a prepared grade in accordance with 105.03.

### MATERIALS

#### 301.02 Materials

10 Materials shall be in accordance with the following:

Coarse Aggregate, Class D or Higher .....	904.03
Geosynthetic Materials .....	918

15 Recycled concrete pavement processed into coarse aggregate-sized material, No. 53, and ACBF, shall not be used when an underdrain is specified.

### CONSTRUCTION REQUIREMENTS

#### 20 301.03 Preparation of Subgrade

Subgrade shall be prepared and proofrolled in accordance with 207.04. When shown on the plans, geosynthetics shall be placed in accordance with 214.03, or as directed. Proofrolling will not be required in trench sections and other areas where proofrolling equipment cannot be used.

25

#### 301.04 Temperature Limitations

Aggregate shall not be placed when the air temperature is less than 35°F. Frozen aggregate shall not be placed. Aggregate shall not be placed on frozen subgrade, subbase, or aggregate base.

30

#### 301.05 Spreading

The moisture content of the aggregate shall be between 4% and 7%. If necessary, the Contractor shall adjust the water content to meet the compaction requirement.

35 Aggregate shall be spread in uniform lifts with a spreading and leveling device approved by the Engineer. The spreading and leveling device shall be capable of placing aggregate to the depth, width, and slope specified. The material shall be placed with spreading equipment, such as a spreader box or paver, capable of placing the material true to line and grade. The material shall be spread such that it minimizes segregation and requires minimal blading or manipulation. The compacted depth of  
40 each lift shall be a maximum of 6 in.

Aggregate shall be transported, handled, and compacted to minimize segregation and the loss of moisture. Aggregate shall be spread in uniform lifts with a spreading

45 and leveling device as approved by the Engineer.

The Contractor may use hand-placing methods, dozers, or graders in small areas, areas of subgrade construction, or where spreading equipment is impractical. Small areas include lane widths less than 12 ft or lengths less than 1,000 ft. In small areas, or areas inaccessible to spreading equipment, each lift shall be a maximum of 4 in.

The material shall be placed in two or more approximately equal lifts when the specified compacted thickness exceeds the maximum allowed.

### 55 **301.06 Compacting**

Each lift shall be compacted immediately after spreading. Dense graded aggregate shall be compacted to achieve the allowable average deflection as determined with LWD testing in accordance with 203.24(b).

60 The allowable average deflection and the maximum deflection for aggregate over the chemically modified soils, untreated soils, and cement stabilized subgrade soils shall be in accordance with the tables shown in 203.24(b). The test section shall be constructed in accordance with ITM 514 for other materials not included in the table shown in 203.24(b).

65 As an alternate to LWD, aggregates shall be compacted to a minimum of 100% of the maximum dry densities in accordance with AASHTO T 99. In situ density will be determined in accordance with 203.24(b). Aggregate shall meet the compaction requirements at the time subsequent courses are placed.

70 In areas inaccessible to compaction equipment, such as private drives and mailbox approaches, the compaction requirements may be accepted by visual inspection.

75 All displacement or rutting of the aggregate shall be repaired prior to placing subsequent material.

### **301.07 Checking and Correcting Base**

80 The top of each aggregate course shall be checked transversely to the cross-section and all deviations in excess of 1/2 in. shall be corrected. If additional aggregate is required, the course shall be remixed and recompacted.

### **301.08 Method of Measurement**

85 Compacted aggregate base will be measured by the cubic yard based on the theoretical volume to the neat lines as shown on the plans. Geosynthetics will be measured in accordance with 214.05.

### **301.09 Basis of Payment**

90 The accepted quantities of compacted aggregate base will be paid for at the contract unit price per cubic yard, complete in place. Geosynthetics will be paid for in accordance with 214.06.

Payment will be made under:

	<b>Pay Item</b>	<b>Pay Unit Symbol</b>
95	Compacted Aggregate, No. 2 .....	CYS
	Compacted Aggregate, No. 5 .....	CYS
	Compacted Aggregate, No. 8 .....	CYS
	Compacted Aggregate, No. 53 .....	CYS

100

The cost of placing, spreading, compacting, water, aggregate placed outside neat lines as shown on the plans, and necessary incidentals shall be included in the cost of the pay item.

105 Payment will not be made for material placed outside of a 1:1 slope from the planned typical section.

Replacement of pavement damaged by the Contractor's operations shall be at no additional payment.

110

## **SECTION 302 – SUBBASE**

### **302.01 Description**

5 This work shall consist of a foundation course of selected materials, placed and compacted on a prepared subgrade in accordance with 105.03.

10 Subbase for PCCP shall consist of a 3 in. layer of coarse aggregate No. 8 as the aggregate drainage layer placed over a 6 in. layer of coarse aggregate No. 53 as the separation layer. Dense graded subbase shall consist of a 6 in. layer of coarse aggregate No. 53.

## **MATERIALS**

### **302.02 Materials**

15 Materials shall be in accordance with the following:

Coarse Aggregate, Class B or Higher, Size No. 8 ..... 904

Coarse Aggregate, Class D or Higher, Size No. 53 ..... 904

20 Coarse aggregate No. 8 used as an aggregate drainage layer shall consist of 100% crushed stone or ACBF.

## **CONSTRUCTION REQUIREMENTS**

### **302.03 Preparation of Subgrade**

Subgrade shall be prepared in accordance with 207.

#### **302.04 Temperature Limitations**

- Aggregate shall not be placed when the air temperature is less than 35°F.
- 30 Aggregate shall not be placed on a frozen subgrade. Frozen aggregate shall not be placed.

#### **302.05 Spreading**

- Aggregate shall be spread in uniform lifts with a spreading and leveling device
- 35 approved by the Engineer. The spreading and leveling device shall be capable of placing aggregate to the depth, width, and slope specified. The compacted depth of each lift shall be a minimum of 3 in. and a maximum of 6 in.

- Aggregate shall be handled and transported to minimize segregation and the loss
- 40 of moisture. In areas inaccessible to mechanical equipment, approved hand spreading methods may be used.

- The moisture content of the aggregate shall be between 4% and the optimum moisture content when the aggregate is delivered to the project. Water shall not be
- 45 added to the aggregate on the grade.

#### **302.06 Compacting**

Subbases shall be compacted as follows:

- 50 **(a) Aggregate Separation Layers and Dense Graded Subbase**

Compaction shall be in accordance with 301.06. All displacement or rutting of the aggregate separation layers shall be repaired prior to placing subsequent material.

#### **(b) Aggregate Drainage Layer**

- 55 Compaction shall consist of two passes with a vibratory roller before trimming, and one pass with the same roller in static mode after trimming. A vibratory roller shall be equipped with a variable amplitude system, speed control device, and have a minimum vibration frequency of 1,000 vibrations per minute. A roller in accordance with 409.03(d)4 may be used.

- 60 Construction traffic shall not be allowed on the aggregate drainage layer, except where placement of the PCCP is restricted. Exceptions shall be subject to approval by the Engineer. Displacement or rutting of the aggregate drainage layers shall be repaired prior to placing subsequent material.

- 65 In areas inaccessible to standard size compacting equipment a specialty roller/compactor in accordance with 409.03(d)7 shall be used.

#### **302.07 Checking and Correcting Subbase**

- 70 The top of each aggregate course shall be checked transversely and all deviations in excess of 1/2 in. shall be corrected. If additional aggregate is required, the course shall be remixed and recompacted.

**302.08 Method of Measurement**

75 Subbase for PCCP or dense graded subbase will be measured by the cubic yard based on the theoretical volume to the neat lines as shown on the plans. The quantity shown in the Schedule of Pay Items will be adjusted if it is shown to be different by more than 2% of the measured quantity.

**302.09 Basis of Payment**

80 The accepted quantities of subbase for PCCP or dense graded subbase will be paid for at the contract unit price per cubic yard, complete in place.

Payment will be made under:

85

**Pay Item****Pay Unit Symbol**

Dense Graded Subbase .....CYS

Subbase for PCCP .....CYS

90

The cost of compacting, water, aggregate placed outside neat lines as shown on the plans, and necessary incidentals shall be included in the cost of the subbase.

**SECTION 303 – AGGREGATE PAVEMENTS OR SHOULDERS****303.01 Description**

5 This work shall consist of placing a dense-graded compacted aggregate on prepared subgrade in accordance with 105.03.

**MATERIALS****303.02 Materials**

10 Materials shall be in accordance with the following:

Coarse Aggregate, Class D or Higher, Size No. 53 ..... 904

Coarse Aggregate, Class D or Higher, Size No. 73\* ..... 904

\* Surface courses only, when specified.

15

**CONSTRUCTION REQUIREMENTS****303.03 Preparation of Subgrade**

20 Subgrade shall be prepared and proofrolled in accordance with 207.04. When shown on the plans, geosynthetics shall be placed in accordance with 214.03, or as directed. Proofrolling will not be required in trench sections and other areas where proofrolling equipment cannot be used.

**303.04 Temperature Limitations**

25 Aggregate shall not be placed when the air temperature is less than 35°F.

Aggregate shall not be placed on a frozen subgrade. Frozen aggregate shall not be placed.

### **303.05 Spreading**

- 30 Aggregate shall be spread in uniform lifts with a spreading and leveling device approved by the Engineer. The spreading and leveling device shall be capable of placing aggregate to the depth, width, and slope specified. The compacted depth of each lift shall be a minimum of 3 in. and a maximum of 6 in., except where utilized as a shoulder. The compacted depth of a lift for a shoulder shall be a minimum of 3 in.  
35 and a maximum of 9 in.

Aggregate shall be handled and transported to minimize segregation and the loss of moisture. In areas inaccessible to mechanical equipment, approved hand spreading methods may be used.

40

The moisture content of the aggregate shall be between 4% and 7%. If necessary, the Contractor shall adjust the water content to meet the compaction requirement.

### **303.06 Compacting**

- 45 Compaction shall be in accordance with 301.06.

- The allowable average deflection and the maximum deflection for the aggregate over the chemically modified soils and untreated soils, shall be in accordance with the tables shown in 203.24(b). The test section shall be constructed in accordance with  
50 ITM 514 for other materials not included in the table shown in 203.24(b). All displacement or rutting of the compacted aggregate shall be repaired prior to placing subsequent material.

- In areas inaccessible to compaction equipment, such as private drives, compaction  
55 requirements may be accepted by visual inspection.

### **303.07 Checking and Correcting Base and Surface**

- The top of each aggregate course shall be checked transversely and all deviations in excess of 1/2 in. shall be corrected. If additional aggregate is required, the course  
60 shall be remixed and recompact.

### **303.08 Dust Palliative**

A dust palliative, if required, shall be in accordance with 407.

- 65 **303.09 Method of Measurement**

Compacted aggregate will be measured by the ton in accordance with 109.01(b) for the type specified. Geosynthetics will be measured in accordance with 214.05.

### **303.10 Basis of Payment**

- 70 The accepted quantities of compacted aggregate will be paid for at the contract unit price per ton, for the type specified, complete in place. Geosynthetics will be paid

for in accordance with 214.06.

Payment will be made under:

75

**Pay Item**

**Pay Unit Symbol**

Compacted Aggregate, No. 53 .....TON

Compacted Aggregate, No. 73 .....TON

80

The cost of placing, spreading, compacting, water, and necessary incidentals shall be included in the costs of the compacted aggregate.

85 Payment will not be made for material placed outside of a 1:1 slope from the planned typical section.

Replacement or repair of pavement or shoulders damaged by the Contractor's operations shall be at no additional payment.

## SECTION 304 – ASPHALT BASES

### 304.01 Description

5 This work shall consist of constructing an HMA base on a prepared surface or preparing an existing asphalt pavement for use as an asphalt base in accordance with 105.03.

## MATERIALS

### 10 304.02 Materials

Materials shall be in accordance with the appropriate sections.

### 304.03 Sealing Cracks and Joints

15 Cracks and joints shall be sealed in accordance with 408.

### 304.04 Partial Depth and Full Depth Patching

20 Areas to be patched will be marked on the surface by the Engineer. The marked pavement shall be removed to the depth shown on the typical section or as directed. A minimum 2 in. vertical joint shall be constructed with the pavement that remains in place. If it is determined that the marked pavement is to be removed full depth, the patch depth shall be to the bottom of the existing asphalt material or as directed.

25 The subgrade of the aggregate base under the patches shall be compacted in accordance with 203.25. If the excavation for patches reveals unsuitable subgrade material, such material shall be removed to a depth of 6 in. and backfilled to the top of subgrade with compacted aggregate in accordance with 301. Unauthorized excavation beyond neat lines shall be replaced with compacted aggregate in accordance with 301.



## SECTION 904 – AGGREGATES

### 904.01 Aggregates

Aggregates shall consist of natural or manufactured materials produced from but  
 5 not limited to limestone, dolomite, gravels, sandstones, steel furnace slag, SF,  
 air-cooled blast furnace slag, ACBF, granulated blast furnace, GBF, wet bottom boiler  
 slag, or other geologic rock types approved by the Engineer.

A source will not be considered for acceptance of material until a preliminary  
 10 investigation has been made. As part of this investigation, samples will be obtained  
 and tests conducted to determine the quality and classification of the aggregates in  
 accordance with ITM 203.

Two types of samples are required for the preliminary investigation: ledge  
 15 samples for crushed stone sources and production samples for crushed stone, natural  
 sand and gravel, and slag sources.

Ledge samples will be obtained from bedrock units as they naturally occur in the  
 proposed working face of the quarry. Ledges will be identified, at a minimum, by their  
 20 differences in color, texture, and geological formation.

Production samples will be obtained from stockpiles of finished materials.

Aggregates, except those used for precast concrete units or fine aggregates used  
 25 for snow and ice abrasive, shall be supplied by a Certified Aggregate Producer in  
 accordance with 917. Structure backfill may be obtained from a non-CAPP source in  
 accordance with 211.02. SF for SMA mixtures shall also require the following:

- 30 (a) Specific gravity quality control tests shall be completed at a  
frequency of one test per 2,000 t produced.
- (b) Target bulk specific gravity shall be established using the average of  
the first four tests.
- 35 (c) Subsequent individual tests shall be within 0.050 of the target bulk  
specific gravity.
- (d) Moving average of four consecutive tests shall be within 0.040 of  
40 the target bulk specific gravity.
- (e) Tests outside these ranges shall require the material to be isolated  
from the approved stockpile until action has been taken to eliminate  
the cause of the non-conformity. Any non-conforming test shall be  
followed immediately by a corrective action. Corrective actions  
45 shall include, but are not limited to, investigation for assignable  
cause, correction of known assignable cause, and retesting.



- (f) If it is determined that a new target is necessary, a request shall be made in writing to the DTE to establish the new target.

50

Dolomite aggregates are defined as carbonate rock containing at least 10.3% elemental magnesium when tested in accordance with ITM 205.

- Polish resistant aggregates are defined as those aggregates in accordance with ITM 214. Aggregates meeting these requirements will be maintained on the QPL of Polish Resistant Aggregate Sources.

- Sandstone aggregates shall only be used in HMA surface or SMA surface mixtures. Sandstone aggregates are defined as a sedimentary rock composed of siliceous sandgrains containing quartz, chert, and quartzose rock fragments in a carbonate matrix or cemented with silica, calcite, or dolomite. The Department's Division of Materials and Tests will determine identification of sandstone.

- SF may be used in aggregate shoulders, HMA surface or SMA surface mixtures, dumped riprap, and snow and ice abrasives. SF coarse aggregate may be used in HMA base and HMA intermediate mixtures if the deleterious content is less than 4.0% when tested in accordance with ITM 219. RAP with SF may be used in accordance with 401.06, 402.08, and 410.06.

- Adjustments in weight will be made for pay items equal to or greater than 500 t to compensate for the difference in specific gravity of slag compared to natural aggregate when payment is on a weight basis. The following typical values for specific gravity will be used:

Slag Material	Typical Values for Specific Gravity
SF Fine Aggregate	3.4
SF Coarse Aggregate	
ACBF Fine Aggregate	2.6
ACBF Coarse Aggregate	2.3
GBF Fine Aggregate	2.1

75

- When slag is furnished as an aggregate, the approximate quantity of tons to be supplied will be determined by multiplying the pay item quantity of tons by the specific gravity of slag divided by 2.6. The adjusted contract quantities will be determined by multiplying the accepted quantity of tons by 2.6 divided by the specific gravity of the slag.

80

At time of use, aggregates shall be free from lumps or crusts of hardened or frozen materials.

- Composite stockpiling of natural sand fine aggregate from multiple sources into

85

- one stockpile will be allowed provided the fine aggregates are within a range of 0.030 for the bulk specific gravity (dry) and a range of 0.5% for the absorption. The range of bulk specific gravity (dry) and absorption values shall be the difference between the average values for each of the fine aggregate sources within the stockpile as determined by the Department's Division of Materials and Tests. A written request for the composite stockpiling shall be made to the Department's Division of Materials and Tests.

#### 904.02 Fine Aggregates

- 95 Fine aggregates are defined as 100% passing the 3/8 in. (9.5 mm) sieve and a minimum of 80% passing the No. 4 (4.75 mm) sieve. Characteristics of fine aggregates are as follows:

Characteristic	PCC	HMA	SMA
Physical			
Organic Impurities, AASHTO T 21 lighter than or equal to, Color Standard (Note 1)... Acid Insoluble, ITM 202.....	3	(Note 2)	
Soundness			
Freeze and Thaw, AASHTO T 103, Procedure A, % max. (Note 3).....	10.0	10.0	10.0
Brine Freeze and Thaw, ITM 209, % max. (Note 3).....	12.0	12.0	12.0
Sodium Sulfate Soundness, AASHTO T 104, % max. (Note 3).....	10.0	10.0	10.0
Notes:			
1. When subjected to the colormetric test for organic impurities and a color darker than the standard is produced, it shall be tested for effect of organic impurities on strength of mortar in accordance with AASHTO T 71. If the relative strength at seven days is less than 95% it shall be rejected.			
2. The fine aggregate, including blended fine aggregate, used in HMA Surface 4.75 mm mixtures shall have a minimum acid-insoluble content of 40%, except when using ACBF or GBF sands, the minimum acid-insoluble content shall be 25%. Acid-insoluble requirements shall not apply to crushed gravel, limestone, or dolomite sands.			
3. AASHTO T 104 and ITM 209 may be run at the option of the Engineer, in-lieu of AASHTO T 103.			

#### 100 (a) For Portland Cement Concrete

Fine aggregate for use in PCCP or bridge decks shall be natural sand. Fine aggregate for other PCC shall be natural sand or crushed limestone, dolomite, gravel, or ACBF.

- 105 Natural sand, which has been used as foundry sand, when tested in accordance with ITM 215 and complying with IDEM Class III or Class IV in accordance with 329

IAC 10-28-8 may be used in precast concrete units or precast concrete pipe. When foundry sand is used, the precast concrete manufacturer shall maintain a copy of the Waste Classification issued by IDEM and an indemnification statement shall accompany the precast items to each contract.

**(b) For HMA Mixtures**

Fine aggregates for use in HMA shall be natural sand or crushed limestone, dolomite, gravel, sandstone, SF, or ACBF. SF sand may be used in HMA surface mixtures. SF sand may only be used in HMA base and HMA intermediate mixtures if SF, in accordance with 904.01, is used to produce the SF sand. The amount of crushed limestone sand shall not exceed 20% by volume of the total aggregate used in HMA surface mixtures with ESAL counts equal to or greater than 3,000,000, except limestone sands manufactured from aggregates on the QPL of Polish Resistant Aggregate Sources will not be limited. If soundness testing cannot be conducted, the aggregate shall come from a Category I source in accordance with ITM 203.

The fine aggregate angularity value of the total blended aggregate material from the fine and coarse aggregates, and recycled materials shall meet or exceed the minimum values for the appropriate ESAL category and position within the pavement structure as follows:

FINE AGGREGATE ANGULARITY		
Traffic ESAL	Depth from Surface	
	≤ 4 in.	> 4 in.
< 3,000,000	40*	40
3,000,000 to < 10,000,000	45	40
≥ 10,000,000	45	40
* For 4.75 mm mixtures, the fine aggregate angularity shall be 45 for < 3,000,000 ESAL.		

Fine Aggregate Angularity, Method A..... AASHTO T 304

The fine aggregate angularity value shall not apply to OG mixtures.

**(c) For SMA Mixtures**

Fine aggregate for SMA shall be limestone, dolomite, crushed gravel, SF, or ACBF. Crushed gravels shall have a minimum fine aggregate angularity of 45 in accordance with AASHTO T 304, Method A. Fine aggregates shall be non-plastic in accordance with AASHTO T 90.

**(d) For Pneumatically Placed Mortar**

Fine aggregate shall be natural sand suitable for use with a pneumatic cement gun. Fine aggregate shall be size No. 15, or size PP in accordance with 904.02(h), or an approved gradation from a CAPP source.

**(e) Mortar Sand**

- 145 Fine aggregate for mortar shall consist of uniformly graded natural sand in accordance with gradation requirements of 904.02(h) for size No. 15 or an approved gradation from a CAPP source.

**(f) Mineral Filler for SMA**

- 150 Mineral filler shall consist of dust produced by crushing stone, portland cement, or other inert mineral matter having similar characteristics. Mineral filler shall be in accordance with the gradation requirements of 904.02(h) for size No. 16 or as approved by the Engineer. Mineral filler shall be in accordance with ITM 203 or from an ACBF source. The sieve analysis of mineral filler shall be conducted in accordance with AASHTO T 37 except as noted in 904.07. Mineral filler shall be non-plastic in accordance with AASHTO T 90.

**(g) Snow and Ice Abrasives**

- 160 Snow and ice abrasives shall be fine aggregates or cinders in accordance with the gradation requirements of 904.02(h) for size S&I.

When **SF** is used for snow and ice abrasives, and payment is on a tonnage basis, the pay quantity shall be adjusted in accordance with 904.01.

165 **(h) Sizes of Fine Aggregates**

SIZES (Percent Passing)						
Sieve Sizes	23	24	15	16	PP	S&I
3/8 in. (9.5 mm)	100	100				100
No. 4 (4.75 mm)	95 - 100	95 - 100			100	
No. 6 (3.35 mm)			100			
No. 8 (2.36 mm)	80 - 100	70 - 100	90 - 100		85 - 95	
No. 16 (1.18 mm)	50 - 85	40 - 80				
No. 30 (600 µm)	25 - 60	20 - 60	50 - 75	100	50 - 65	
No. 50 (300 µm)	5 - 30	7 - 40	15 - 40		15 - 25	0 - 30
No. 80 (180 µm)				95 - 100		
No. 100 (150 µm)	0 - 10	1 - 20	0 - 10		0 - 10	
No. 200 (75 µm)	0 - 3	0 - 6	0 - 3	65 - 100		0 - 7

**(i) Sampling and Testing**

- 170 Sampling and testing will be conducted in accordance with the following AASHTO and ITM procedures.

Acid Insoluble Content..... ITM 202

Amount of Material Finer than

No. 200 (75 µm) Sieve\* ..... AASHTO T 11

- 175 Brine Freeze and Thaw Soundness..... ITM 209

Control Procedures for Classification

of Aggregates..... ITM 203

	Determining the Plastic Limit and Plasticity Index of Soils.....	AASHTO T 90
180	Mortar Strength .....	AASHTO T 71
	Organic Impurities.....	AASHTO T 21
	Sampling Aggregates.....	AASHTO R 90
	Sampling Stockpiled Aggregates.....	ITM 207
	Sieve Analysis of Aggregate* .....	AASHTO T 27
185	Sieve Analysis of Mineral Filler* .....	AASHTO T 37
	Soundness* .....	AASHTO T 103, AASHTO T 104
	Specific Gravity and Absorption, Fine Aggregate.....	AASHTO T 84
	* Except as noted in 904.07.	

190

### **904.03 Coarse Aggregates**

Coarse aggregates are defined as having a minimum of 20% retained on the No. 4 (4.75 mm) sieve. Coarse aggregates shall not contain adherent fines that are detrimental to the end product as defined in ITM 211.

195

The coarse aggregate shall comply with the quality requirements and the additional requirements in accordance with 904.03(a). Coarse aggregate may be rejected based on previous performance service records. Class AP is defined as the highest classification and Class F the lowest.

200

Blending of material for compliance with gradation or crushed particle requirements may be approved when requested in writing.

Blending of aggregate products to improve the quality classification of the finished product will not be allowed.

205

[continued on next page]

## (a) Classification of Aggregates

Characteristic Classes	AP	AS	A	B	C	D	E	F
Quality Requirements:								
Freeze and Thaw Beam Expansion, % max. (Note 1) .....	.060							
Los Angeles Abrasion, % max. (Note 2) .....	40.0	30.0	40.0	40.0	45.0	45.0	50.0	
Freeze and Thaw, AASHTO T 103, Procedure A, % max. (Note 3) .....	12.0	12.0	12.0	12.0	16.0	16.0	20.0	25.0
Sodium Sulfate Soundness, % max. (Note 3) .....	12.0	12.0	12.0	12.0	16.0	16.0	20.0	25.0
Brine Freeze and Thaw Soundness, % max. (Note 3) .....	30	30	30	30	40	40	50	60
Absorption, % max. (Note 4) .....	5.0	5.0	5.0	5.0	5.0			
Additional Requirements:								
Deleterious, % max.								
Clay Lumps and Friable Particles .....	1.0	1.0	1.0	1.0	2.0	4.0		
Non-Durable (Note 5) .....	4.0	2.0	4.0	4.0	6.0	8.0		
Coke .....					(See Note 6)			
Iron .....					(See Note 6)			
Chert (Note 7) .....	3.0	3.0	3.0	5.0	8.0	10.0		
Weight per Cubic Foot for Slag, lb, min. ....	75.0		75.0	75.0	70.0	70.0	70.0	
Crushed Particles, % min. (Note 8)			20.0	20.0	20.0	20.0		
Compacted Aggregates .....								

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## Notes:

1. Freeze and thaw beam expansion shall be tested and re-tested in accordance with ITM 210.
2. Los Angeles abrasion requirements shall not apply to BF.
3. Aggregates may, at the option of the Engineer, be accepted by the Sodium Sulfate Soundness or Brine Freeze and Thaw Soundness requirements.
4. Absorption requirements apply only to aggregates used in PCC and HMA mixtures except they shall not apply to BF. When crushed stone coarse aggregates from Category I sources, in accordance with ITM 203, consist of production from ledges whose absorptions differ by more than two percentage points, the absorption test will be performed every three months on each size of material proposed for use in PCC or HMA mixtures. Materials having absorption values between 5.0 and 6.0 that pass AP testing may be used in PCC. If variations in absorption preclude satisfactory production of PCC or HMA mixtures, independent stockpiles of materials will be sampled, tested, and approved prior to use.
5. Non-durable particles include: a) soft particles as determined by ITM 206, b) other particles which are structurally weak, such as soft sandstone, shale, limonite concretions, coal, weathered schist, cemented gravel, ocher, shells, and wood, and c) other objectionable material. Determination of non-durable particles shall be made from the total weight (mass) of material retained on the 3/8 in. (9.5 mm) sieve. Scratch Hardness Test shall not apply to crushed stone coarse aggregate.
6. ACBF and SF coarse aggregate shall be free of objectionable amounts of coke, iron, and lime agglomerates.
7. The bulk specific gravity of chert shall be based on the saturated surface dry condition. The amount of chert less than 2.45 bulk specific gravity shall be determined on the total weight (mass) of material retained on the 3/8 in. (9.5 mm) sieve for sizes 2 through 8, 43, 53, and 73 and on the total weight (mass) of material retained on the No. 4 (4.75 mm) sieve for sizes 9, 11, 12, and 91.
8. Crushed particle requirements apply to gravel coarse aggregates used in compacted aggregates. Determination of crushed particles shall be made from the weight (mass) of material retained on the No. 4 (4.75 mm) sieve in accordance with ASTM D5821.



**(b) Coarse Aggregate Angularity for HMA and SMA**

The coarse aggregate angularity, CAA, of the total blended aggregate, including recycled materials, shall meet or exceed the minimum values for the appropriate ESAL category and position within the pavement structure as follows:

215

COARSE AGGREGATE ANGULARITY		
Traffic ESAL	Depth from Surface	
	≤ 4 in.	> 4 in.
< 3,000,000	75	50
3,000,000 to < 10,000,000	85/80*	60
≥ 10,000,000	95/90*	95/90*
* Denotes two faced crush requirements.		

For SMA mixtures, the total blended aggregate shall be 100% one face and 95% two face crushed.

220

Coarse Aggregate Angularity..... ASTM D5821

Coarse aggregate angularity requirements do not apply to 4.75 mm HMA mixture designation.

225

**(c) Flat and Elongated**

The coarse aggregate shall contain 10% or less flat and elongated particles. A flat and elongated piece is defined as a particle having a ratio of length to thickness greater than 5. Determination of flat and elongated particles shall be made from the weight (mass) of material retained on the 3/8 in. (9.5 mm) sieve and each sieve size greater than the 3/8 in. (9.5 mm) sieve.

230

Flat and Elongated..... ASTM D4791

Flat and elongated requirements do not apply to 4.75 mm HMA mixture designation.

235

**(d) Surface Aggregate Requirements**

The surface mixture aggregates selection shall be based on the ESAL category as follows:

240

**1. HMA Coarse Aggregate**

245

a. ESAL Category 2 and Type B surface mixtures. All coarse aggregate types including ACBF, SF, sandstone, crushed dolomite, polish resistant aggregate, crushed stone, or gravel may be used.

b. ESAL Category 3 and Type C surface mixtures. ACBF, SF,



250 sandstone, crushed dolomite, polish resistant aggregate or any combination thereof shall be used. Crushed stone or gravel shall not be used unless the aggregate is classified as a crushed dolomite or polish resistant aggregate.

255 c. ESAL Category 4 and Type D surface mixtures. High friction aggregates including ACBF, SF, sandstone, or aggregates in accordance with ITM 221 shall be used and, at a minimum, shall comprise 50% by volume of the coarse aggregate.

260 Crushed dolomite and polish resistant aggregates may be used up to a maximum 50% by volume of the coarse aggregate material retained on the No. 4 (4.75 mm) sieve when blended with a high friction aggregate.

265 Crushed stone and gravel may be used up to a maximum 20% by volume of the coarse aggregate material retained on the No. 4 (4.75 mm) sieve when blended with a high friction aggregate.

## 270 **2. SMA Coarse Aggregate**

SF, sandstone, crushed dolomite, and polish resistant aggregates in accordance with 904.03(a) may be used in SMA mixtures provided the mixtures are designed in accordance with ITM 220.

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Sieve Sizes	COARSE AGGREGATE SIZES (Percent Passing)												
	Coarse Graded											Dense Graded	
	2	5	8	9	11, SC 11 <sup>(5)</sup>	12, SC 12 <sup>(5)</sup>	SC 16 <sup>(5)</sup>	43 <sup>(1)</sup>	91	93PG <sup>(6)</sup>	53 <sup>(1)</sup>	73 <sup>(1)</sup>	
4 in. (100 mm)													
3 1/2 in. (90 mm)													
2 1/2 in. (63 mm)	100												
2 in. (50 mm)	80 - 100												
1 1/2 in. (37.5 mm)		100					100				100		
1 in. (25 mm)	0 - 25	85 - 98	100					70 - 90	100		80 - 100	100	
3/4 in. (19 mm)	0 - 10	60 - 85	75 - 95	100				50 - 70			70 - 90	90 - 100	
1/2 in. (12.5 mm)	0 - 7	30 - 60	40 - 70	60 - 85	100	100	100	35 - 50		98 - 100	55 - 80	60 - 90	
3/8 in. (9.5 mm)		15 - 45	20 - 50	30 - 60	75 - 95	95 - 100	94 - 100			75 - 100			
No. 4 (4.75 mm)		0 - 15	0 - 15	0 - 15	10 - 30	50 - 80	15 - 45	20 - 40		10 - 60	35 - 60	35 - 60	
No. 8 (2.36 mm)		0 - 10	0 - 10	0 - 10	0 - 10	0 - 35		15 - 35		0 - 15	25 - 50		
No. 16 (1.18 mm)							0 - 4						
No. 30 (600 μm)						0 - 4		5 - 20		0 - 5	12 - 30	12 - 30	
No. 200 (75 μm) <sup>(2)</sup>								0 - 6.0			5.0 - 13.0 <sup>(4)</sup>	5.0 - 12.0	
Decant (PCC) <sup>(3)</sup>		0 - 1.5	0 - 1.5	0 - 1.5	0 - 1.5	0 - 1.5			0 - 1.5				
Decant (Non-PCC)	0 - 2.5	0 - 2.5	0 - 3.0	0 - 2.5	0 - 2.5	0 - 2.0			0 - 2.5	0 - 2.0			
Decant (SC)					0 - 1.5	0 - 1.5	0 - 1.5						

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- (1) The liquid limit shall not exceed 25 (35 if slag) and the plasticity index shall not exceed 5. The liquid limit shall be determined in accordance with AASHTO T 89 and the plasticity index in accordance with AASHTO T 90.
- (2) Includes the total amount passing the No. 200 (75  $\mu$ m) sieve as determined by AASHTO T 11 and AASHTO T 27.
- (3) Decant may be from 0 to 2.5 for stone and slag.
- (4) When slag is used for separation layers as defined in 302.01, the total amount passing the No. 200 (75  $\mu$ m) sieve shall be 10.0 to 12.0.
- (5) Seal coat (SC) aggregates shall be 85% one face and 80% two face crushed. The Flakiness Index in accordance with ITM 224 shall be a maximum of 25%.
- (6) Pea gravel shall be generally uncrushed gravel, with a maximum of 20% crushed particles, and shall meet the gradation requirements of 93PG. Determination of crushed particles shall be made from the weight (mass) of material retained on the No. 4 (4.75 mm) sieve in accordance with ASTM D5821.

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Where an AASHTO coarse aggregate is specified as an option, gradation shall be  
 280 in accordance with AASHTO M 43. All other aggregate properties shall be in  
 accordance with 904.

**(f) Sampling and Testing**

Sampling and testing will be in accordance with the following AASHTO, ASTM,  
 285 and ITM procedures.

	Amount of Material finer	
	than No. 200 (75 µm) Sieve* .....	AASHTO T 11
	Brine Freeze and Thaw Soundness .....	ITM 209
290	Clay Lumps and Friable Particles .....	AASHTO T 112
	Control Procedures for Classification of Aggregates .....	ITM 203
	Crushed Particles .....	ASTM D5821
	Dolomite Aggregates .....	ITM 205
	Flat and Elongated Particles .....	ASTM D4791
295	Freeze and Thaw Beam Expansion .....	ITM 210
	Lightweight Pieces in Aggregates* .....	AASHTO T 113
	Los Angeles Abrasion .....	AASHTO T 96
	Micro-Deval Abrasion .....	AASHTO T 327
	Polished Resistant Aggregates .....	ITM 214
300	Sampling Aggregates* .....	AASHTO R 90
	Sampling Stockpiled Aggregates .....	ITM 207
	Scratch Hardness .....	ITM 206
	Sieve Analysis* .....	AASHTO T 27
305	Soundness* .....	AASHTO T 103, AASHTO T 104
	Specific Gravity and Absorption* .....	AASHTO T 85
	Unit Weight and Voids in Aggregates .....	AASHTO T 19
	*Except as noted in 904.07	

**904.04 Riprap**

Riprap shall consist of SF for dumped riprap only, sound stone, stone masonry, or  
 other approved material, free from structural defects and of approved quality. Stone  
 containing shale, unsound sandstone, or other material that will disintegrate readily,  
 shall not be used.

315

**(a) Dumped Riprap**

Dumped riprap shall be broken concrete, masonry, stone removed from an old  
 structure, broken pieces removed from concrete pavement, base, or monolithic brick  
 pavement, broken rock from Class X, Class Y, unclassified excavation, or solid rock  
 320 excavation. Material provided from sources outside the right-of-way shall be coarse  
 aggregate, Class F or higher.

**(b) Grouted Riprap**

Grouted riprap material shall be in accordance with dumped riprap or revetment  
 325 riprap.

**(c) Revetment, Class 1, and Class 2 Riprap**

The material shall be coarse aggregate, Class F or higher. Gradation shall be in accordance with 904.04(f).

330

**(d) Uniform Riprap**

The material shall be coarse aggregate, Class F or higher in accordance with 904.03(a). Gradation shall be in accordance with 904.04(f). Either **Uniform A** or **Uniform B** may be utilized.

335

**(e) Precast Concrete Riprap**

Precast concrete riprap shall consist of unreinforced concrete units of the thickness specified and shall be in accordance with the details shown on the plans. The precast concrete units shall be in accordance with ASTM C139, except the fine aggregates shall be in accordance with 904.02(a) and the coarse aggregates, **Class A** or higher, shall be in accordance with 904.03(a). The minimum compressive strength shall be 2,500 psi for an average of three units and 2,300 psi for individual units. The maximum water absorption shall be 12 lb/cu ft for an average of three units.

340

345

**(f) Sizes of Riprap**

GRADATION REQUIREMENTS					
Percent Smaller					
Size, in.	Revetment	Class 1	Class 2	Uniform A	Uniform B
30			100		
24		100	85 - 100		
18	100	85 - 100	60 - 80		
12	90 - 100	35 - 50	20 - 40		
8				100	
6	20 - 40	10 - 30	0 - 20	35 - 80	95 - 100
3	0 - 10	0 - 10	0 - 10		35 - 80
1				0 - 20	0 - 20
Depth of Riprap, min.	18 in.	24 in.	30 in.		

The maximum dimension of individual pieces shall not be greater than three times the minimum dimension and no dimension shall exceed the maximum size listed for the respective size of riprap. The riprap will be visually inspected for size, shape, and consistency.

350

**904.05 Structure Backfill**

The material shall be of acceptable quality, free from large or frozen lumps, wood, or other extraneous matter. It shall consist of suitable sand, gravel, crushed stone, ACBF, or GBF. Structure backfill shall be in accordance with one of the gradations shown in the table below, or coarse aggregate No. 5, No. 8, No. 9, No. 11, No. 12, No. 53, or No. 73 in accordance with the gradation requirements of 904.03(e). Coarse aggregate No. 5, No. 8, No. 9, No. 11, No. 12, No. 53, No. 73, or AASHTO No. 57

355

360 shall be crushed stone or ACBF, Class D or higher.

Sieve Sizes	Nominal Sizes and % Passing					
	2 in.	1 1/2 in.	1 in.	1/2 in.	No. 4	No. 30
2 1/2 in. (63 mm)	100					
2 in. (50 mm)	90 - 100	100				
1 1/2 in. (37.5 mm)	70 - 100	90 - 100	100	100		
1 in. (25.0 mm)	55 - 95	70 - 100	85 - 100			
3/4 in. (19.0 mm)	45 - 90	55 - 95	70 - 100			
1/2 in. (12.5 mm)	35 - 85	40 - 90	55 - 95	85 - 100	100	100
No. 4 (4.75 mm)	20 - 65	20 - 70	25 - 75	45 - 85	90 - 100	
No. 8 (2.36 mm)	10 - 50	10 - 55	15 - 60	25 - 75	75 - 100	
No. 30 (600 µm)	3 - 35	3 - 35	3 - 35	5 - 45	15 - 70	70 - 100
No. 200 (75 µm)	0 - 8	0 - 8	0 - 8	0 - 8	0 - 8	0 - 8

#### 904.06 B Borrow

365 The material used for special filling shall be of acceptable quality, free from large or frozen lumps, wood, or other extraneous matter and shall be known as B borrow. It shall consist of suitable sand, gravel, or crushed stone ACBF, GBF, or other approved material.

370 The material shall contain no more than 10% passing the No. 200 (75 µm) sieve and shall be otherwise suitably graded. The ratio of the fraction passing the No. 200 (75 µm) sieve to the fraction retained on the No. 30 (600 µm) sieve shall not exceed one-fifth. The use of an essentially one-size material will not be allowed unless approved. B borrow containing greater than 3% by dry weight organic material will not be allowed.

375

Sieve analysis and organic material will be performed in accordance with AASHTO T 11 and AASHTO T 267.

#### 904.07 Exceptions to AASHTO Standard Methods

380

##### (a) Exceptions to AASHTO R 90

Stockpile sampling shall be in accordance with ITM 207, unless otherwise approved.

385

##### (b) Exceptions to AASHTO T 11, T 27, and T 37

390 1. When tests are performed in the field where ovens are not available, test samples may be dried in suitable containers over open flame or electric hot plates with sufficient stirring to prevent overheating, then cooled to constant weight.

2. The balance shall be a Class G2 general purpose balance in accordance with AASHTO M 231.

395           **(c) Exceptions to AASHTO T 27 for Coarse Aggregates**

The size of test samples for coarse aggregate shall be as follows:

	<b>Aggregate Size</b>	<b>Mass of Test Sample</b>
	No. 2 .....	11.3 kg min.
400	No. 5, 8, 43, 53, 73, and 91 .....	6.0 - 8.0 kg
	No. 9 .....	4.0 - 6.0 kg
	Structure Backfill	
	2 in. ....	11.3 kg min.
	1 1/2 in. and 1 in. ....	6.0 - 8.0 kg
405	1/2 in. ....	4.0 - 6.0 kg
	No. 4 and No. 30 .....	300 g

**(d) Exceptions to AASHTO T 85**

410           The in-water weight shall be determined following the 15 h soaking period prior to determining the SSD weight.

**(e) Exceptions to AASHTO T 103 and T 104**

- 415           1. Counting the number of individual particles coarser than the 3/4 in. (19.0 mm) sieve will not be required.
- 420           2. For testing ledge rock, the ledge samples shall be crushed to obtain test samples for the designated increments passing the 1 1/2 in. (37.5 mm) sieve and retained on the No. 4 (4.75 mm) sieve. The factors used to calculate the weighted average loss are 30%, 40%, and 30% of the 1 1/2 in. (37.5 mm) - 3/4 in. (19.0 mm), 3/4 in. (19.0 mm) - 3/8 in. (9.5 mm), and 3/8 in. (9.5 mm) - No. 4 (4.75 mm) increments, respectively.
- 425           3. In the case of ledge rock, modify sections 3.3 and 6.2 of AASHTO T 103 and AASHTO T 104 respectively. When the sample received is deficient in material of a component size of any test portion, that material will be supplemented with the available component size to provide the test portion.
- 430           4. Modify section 8 of AASHTO T 103 and section 10 of AASHTO T 104. For materials designated as a coarse aggregate, the weighted loss will be calculated considering the material retained on the No. 4 (4.75 mm) sieve as 100% of the sample, and only the total weighted loss reported. In AASHTO T 104 sections 10.1.3.2 and 10.1.3.3 shall not apply, and unless
- 435           otherwise noted only new solution will be used.