



New AASHTO Guide Specs. For Removal of FCM Designation

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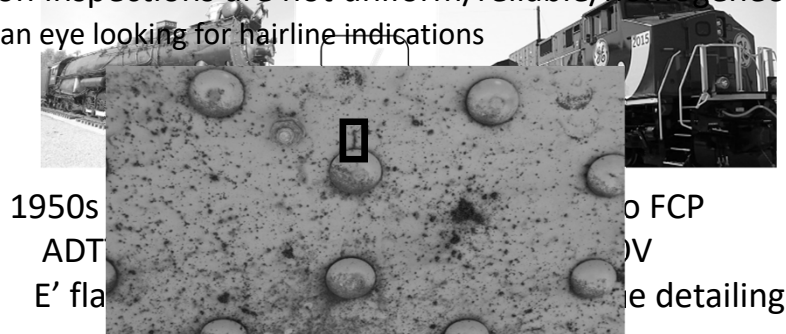
FCM Designation and Consequences

- Fracture-Critical Member (FCM): Steel tension member/component which failure results in collapse/loss of serviceability
- FCM involves fabrication per AASHTO/AWS D1.5 Section 12
 - Fracture Control Plan = Base metal, process, consumable, inspection reqs.
 - One time expense
 - These have been successful in **PREVENTING** brittle fractures
- FCM involves Fracture-Critical Member Inspection (NBIS)
 - 24-month default interval, hands-on along the length of the member
 - FCM Inspection is expensive through the life of the bridge
 - These might not be effective in **DISCOVERING** signs of future fracture

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Issues with FCM Inspection

- What do you get from FCP + NBIS? **MORE INSPECTIONS**
- Differences between bridges are not factored in
- Hands-on inspections are not uniform/reliable/homogeneous
 - Human eye looking for hairline indications



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Redundancy: Are all FCMs “Critical”?

- FCM fracture is rare, collapse due to FCM fracture is most rare
- Most of the bridges the underwent FCM failures provided service
 - Exc: Silver Bridge hanger fracture led to collapse (1967)
- Most FCM failures would not have been detected by inspection
 - Exc: Lafayette St Bridge (St Paul, MN) fracture stemmed from a fatigue crack
- Fracture triggers (CIF, poor welds, brittle steel) are not allowed
- We take advantage of redundancy:
 - Assume the failure happens and check capacity in the faulted state.**

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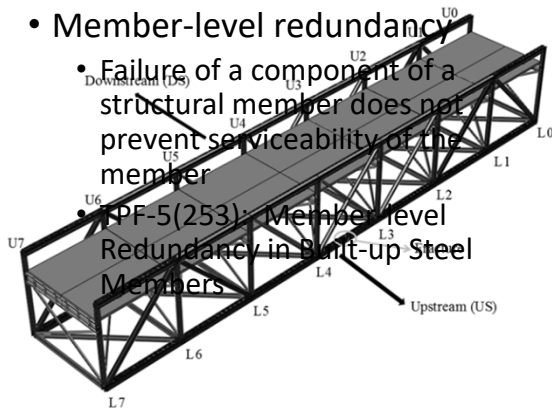
Research Projects in Steel Bridge Redundancy

- Two main types of redundancy

- Member-level redundancy

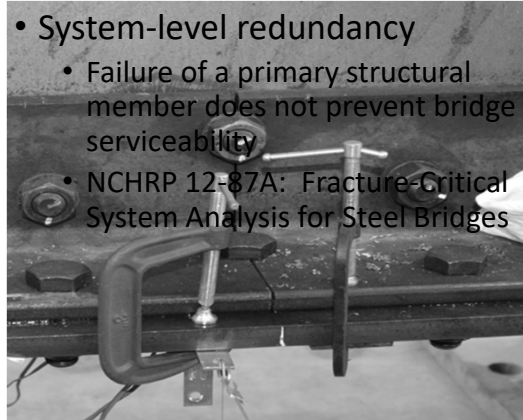
- Failure of a component of a structural member does not prevent serviceability of the member

- TPF-5(253): Member-level Redundancy in Shift-up Steel Members



- System-level redundancy

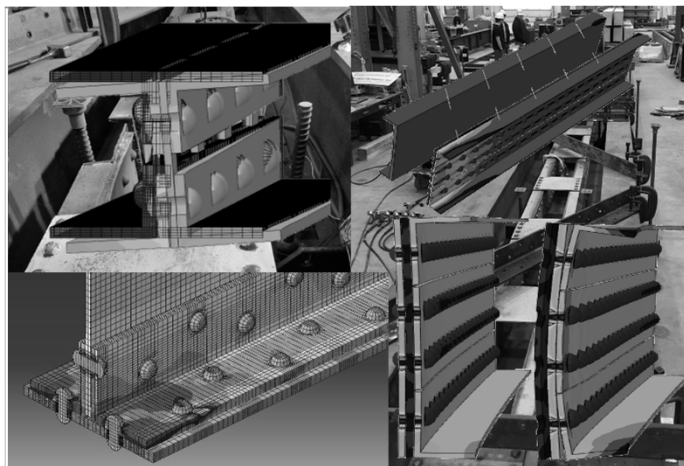
- Failure of a primary structural member does not prevent bridge serviceability
- NCHRP 12-87A: Fracture-Critical System Analysis for Steel Bridges



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TPF-5(253): Research Program

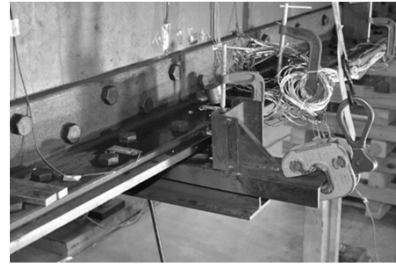
- Multiple fracture tests
 - Flexural Members
 - Axial Members
- Fatigue after fracture
 - Only flexural
- Load-transfer test
 - Truss Chord
- Dozens of FEA models to develop provisions



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TPF-5(253): Fracture Tests

- Notch a component
 - Controlled location (angle/cover plate)
 - Not looking at initial fatigue life – already documented
 - Crack growth through fatigue to critical length (LEFM)
- Cool beam → ensured lower shelf behavior
 - Warmest was -60F....some as cold as -120F
 - Eliminates “but you had good steel” comment
- Apply load to induce a fracture
 - And then....nothing happened
 - Needed to drive a “wedge” into the crack!!



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NCHRP 12-87A: Research Program

- Research stems from FHWA 2012 Memo: SRM vs. FCM
- Develop advanced analysis methods (FEA) applicable to inventory:
 - Old and new, two-girder bridges to tied-arch bridges
 - Benchmarked with available data from actual FCM failures:
 - Neville Island Bridge, Hoan Bridge, etc.
- Load combinations for evaluation of redundancy:
 - Take into consideration that the bridge is in the faulted state
- Performance criteria in the faulted state
- Published as NCHRP Report 883

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Load Combos for Redundancy Evaluations

- Structures built to the FCP
 - *Redundancy I** $(1 + DA_R)(1.05 DC + 1.05 DW + 0.85 LL)$
 - *Redundancy II*** $1.05 DC + 1.05 DW + 1.30 (LL + IM^{***})$

- Structure not built to the FCP
 - *Redundancy I** $(1 + DA_R)(1.15 DC + 1.25 DW + 1.00 LL)$
 - *Redundancy II*** $1.15 DC + 1.25 DW + 1.50 (LL + IM^{***})$

* Applies to SRMs only

** Applies to SRMs and IRMs

*** 15%

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IRM Guide Specification: Fundamentals (I)

- Existing members and new designs (riveted or bolted)
- Flexural and axial members
- Strength criteria to assess internal redundancy
- Fatigue criteria to determine inspection interval (not FCM inspection)
- Provisions “keep you in a box” in terms of:
 - General criteria
 - Member proportions AND condition
 - Must have remaining fatigue life in “unfaulted condition”
 - Faulted condition = one component failed

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IRM Guide Specification: Fundamentals (II)

- Not all members will meet provisions
- Passing member classification: Internally Redundant Member (IRM)
- Easy application based on P/A, Mc/I type calculations
 - Stress amplification
 - Or addition of secondary moments
- Determine interval for “Special Inspection of IRMs”
 - Objective to identify broken components
 - Depth of Special Inspections determined by owner
- Routine safety inspections are not changed
- **Not** intended to be used to justify leaving a broken component in place for extended period

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IRM Spec: Application

- Simple analysis methodology
 - P/A, Mc/I type of calculations
 - Spreadsheet might be all needed
- Specific provisions for different member types:
 - Flexural vs. axial
 - Multi- vs. two-component
 - Numerous illustrations
- Application examples

Table 2.2.1.1—Area

Case	Description
1a	Built-up single or plates and may be lacing or contribute full- and plates w shown in line.
1b	

Figure 1. Cross Section of Built-up Member.

Web Plate: 20-1/2x1/2"
Angles: 8x4x5/8"
Total Member Gross Area, $A_g = 38.7 \text{ in}^2$
Total Member Net Area, $A_n = 35.4 \text{ in}^2$

$x \leq 6$ and the full thickness, then, $\frac{A_n}{A_g}$

+ full-depth plates, then, $\left(1 - \frac{A_n}{A_g}\right)$

erior plate,

Strength in the Factored State:
The first step is to calculate the maximum net section stress to evaluate the strength of the member placed in the factored state. In the factored state, the total factored axial load in the member, P_u , shall be calculated as follows using the load combination for a structure not built to the ASSESS/3049 DLS Fracture Control Plan (FCP), per Table 1.4.1.3:

$$P_u = 7.5P_D + P_{E1} + P_{E2} + P_{E3} + P_{E4} + P_{E5}$$

where:

- $P_D = 1.05(24.3 \text{ kips}) = 25.515 \text{ kips}$ (Table 1.4.1.3)
- $P_{E1} = 1.95(22.3 \text{ kips}) = 43.485 \text{ kips}$ (Table 1.4.1.3)
- $P_{E2} = 1.50(24.3 \text{ kips}) = 36.45 \text{ kips}$ (Table 1.4.1.3)
- $P_{E3} = 23.2 \text{ kips}$ (given)
- $P_{E4} = 22.3 \text{ kips}$ (given)
- $P_{E5} = 165.7 \text{ kips}$ (given)

Hence, the resulting total factored axial load is:

$$P_u = 1.05(24.3 \text{ kips}) + 1.95(22.3 \text{ kips}) + 1.50(24.3 \text{ kips}) + 23.2 \text{ kips} + 22.3 \text{ kips} + 165.7 \text{ kips} = 474.3 \text{ kips}$$

The second step is to calculate the maximum stress in the member after the failure of the component having the largest gross area. In this case it is the web plate. The factored stress in the factored state for the diagonal member can be calculated as follows for the net section and the gross section, respectively:

$$f_u = \frac{P_u}{A_n} = \frac{474.3 \text{ kips}}{35.4 \text{ in}^2} = 13.4 \text{ ksi}$$

$$f_g = \frac{P_u}{A_g} = \frac{474.3 \text{ kips}}{38.7 \text{ in}^2} = 12.26 \text{ ksi}$$

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IRM Spec: Impact on Inspection Intervals

- Case I members:

- Infinite unfaulted fatigue life

- Ia: Infinite faulted fatigue life
- Ib: Finite faulted fatigue life

Calculate Y_{REM} in faulted state (N_f)

Calculated Remaining Minimum Fatigue Life N_f (Years)	Maximum Permitted Interval (Years)
$N_f < 20$	Larger of 2 years or $0.5N_f^*$
$N_f \geq 20$	10

*The calculated inspection interval may be rounded up to the next even year interval.

- Case II members:

- Finite unfaulted fatigue life

$$N_f = Y_f(1.0 - N_u/Y_u)$$

Calculated Remaining Minimum Fatigue Life N_f (Years)	Maximum Permitted Interval (Years)
$N_f \leq 5$	Smaller of 2 years or $0.5N_f^*$
$5 < N_f < 20$	$0.5N_f^{**}$
$N_f \geq 20$	10

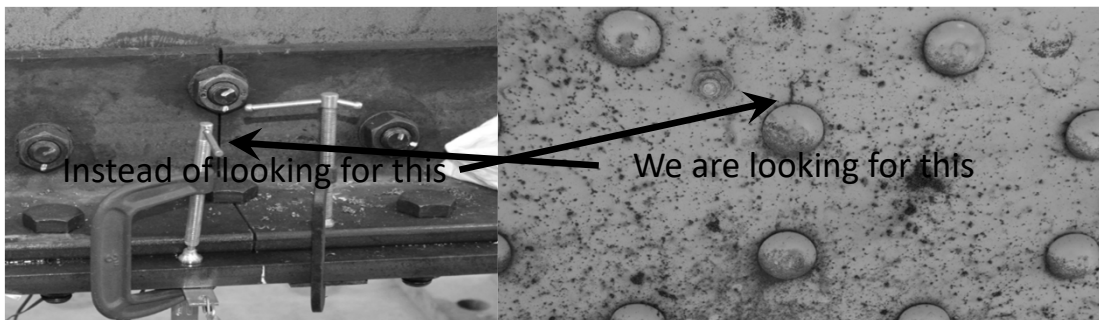
*The calculated inspection interval may be rounded up to the next half-year interval.

**The calculated inspection interval may be rounded up to the next even year interval.

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IRM Spec: The Real Major Advantage

- Inspection is targeting broken (cut, severed) components
 - No need to look for hairline minuscule crack = higher detection rates
 - Inspection effort on par with potential consequences



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SRM Guide Specification: Fundamentals

- Existing bridges and new designs
- Applies to the majority of the inventory:
 - Girder (I-, tub-, through-), truss, tied-arch
- Strength evaluation in the faulted state (two load combinations)
- If member is **SRM** there is **no need to perform any “special” inspection**
- Provisions “keep you in a box” in terms of:
 - General criteria
 - Bridge condition
 - Must not have details known to be problematic
 - Faulted condition = one member failed

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SRM Guide Specification: Application

- Analysis requires use of advanced FEA tool
 - Originally Abaqus, but other software packs are being evaluated
- Guidance for material models, meshing, analytical procedures, failure scenarios, interaction (contact) modeling, connections, etc.
 - Shear stud tensile behavior research
- Performance criteria in the faulted state tailored for FEA results
 - Ex: Effective slab width in composite section in faulted state?
- The application of the guide specification is complex but
WISCONSIN DOT GOT 20+ BRIDGES OF THE FCM INSPECTION LIST!!!

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Moving Forward

- Both Guide Specifications have been approved by AASHTO SCOBS
 - Supporting documents are available too
 - WisDOT has already utilized Guide Specs
- We can design/evaluate for fracture
 - Rational decisions supported on available data and analysis
 - There are no buckling critical members!
- Approaching an integral/unified approach to fracture
 - Better allocation of bridge owner's resources
 - Encourage good practices against fracture (HPS, built-up members, etc.)
 - Allow to focus on potential problems

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Redundancy is not New

- Two-winged aircraft are acceptable as failure RISK is low
 - Consequence high
 - Likelihood low
- Modern steel bridges?
 - Likelihood low (FCP)
 - Consequence low (IRM/SRM)



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S-BRITE

- Steel Bridge Research, Inspection, Training, and Engineering Center

S-BRITE TRAINING COURSES

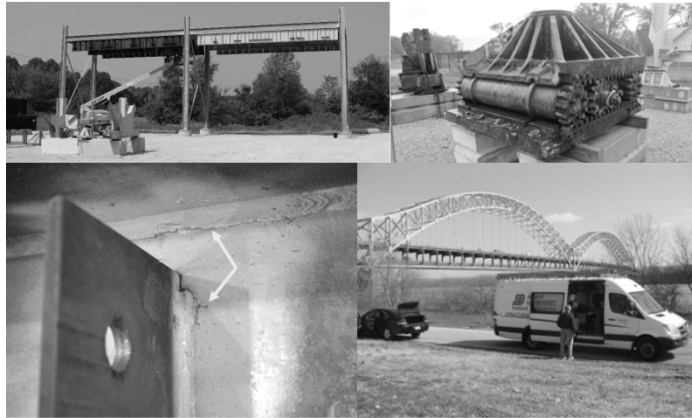
 Bridge Inspection Certification for INDOT Fracture-Critical and Complex Bridge Inspection



Course Offerings

Course Title	Course Date	Host Location	Comment
Design of Steel Bridges for Fatigue & Fracture	TBD		2-1/2 day course at Purdue or 2-day course out-of-state
High Strength Structural Bolts	TBD		1-1/2 day course at Purdue or 1-day course out-of-state
Implementing Effective Retention on Selected Steel Bridge Details	TBD	Purdue University	2 full day course currently by invitation only
 Inspecting Steel Bridges for Lapsing	October 15-16, 2018 Registration Open	Purdue University	1-1/2 day course at Purdue or 1-day course out-of-state
Welding in an Infrastructure System	TBD		1-day course

Participants of each course will receive breakfast, lunch, booklet of training materials, case studies, handson exercises, and CE/LEP's for the course. Each course is designed around experience-based learning, taking advantage of the separate S-BRITE Center [Digital Component Gallery](#) and the Purdue University [Dowson Laboratory](#) resources to provide a comprehensive training experience.



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Questions?

Thank you very much!



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