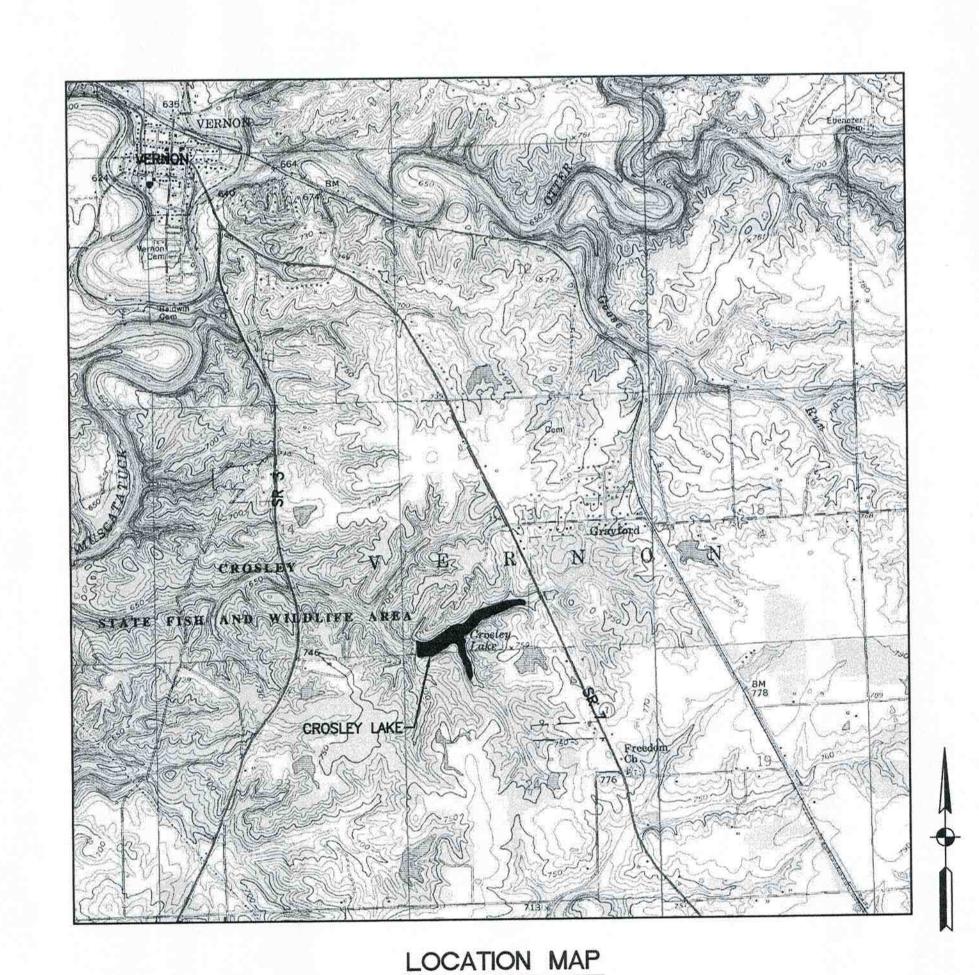
INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH AND WILDLIFE AREA DAM IMPROVEMENTS PROJECT

PROJECT NO. ENG1802321631/E020098

MAY 2018

PLANS FOR PROJECT RE-BID SEPTEMBER 27, 2018



NO SCALE

CROSLEY LAKE DAM **JENNINGS COUNTY**

IDNR DIRECTOR - CAMERON F. CLARK GOVERNOR OF INDIANA - ERIC J. HOLCOMB





DREW O. FLAMION INDIANA P.E. NO. PE11100003

MAY 9, 2018

DATE:



GENERAL LOCATION MAP

CONTRACT NO.: D13038

00-01-13038-TiTLE.dwg

©2018 COMMONWEALTH ENGINEERS, INC.

NOTIFICATION INFORMATION

STATE OF INDIANA VIA DEPARTMENT OF NATURAL RESOURCES DIVISION OF ENGINEERING 402 WEST WASHINGTON STREET, W299 INDIANAPOLIS, IN 46204 (317)232-4150

PROPERTY MANAGER: IDNR CROSLEY FISH AND WILDLIFE 2010 S. STATE HIGHWAY 3 NORTH VERNON, IN 47265

(812)346-5596 CHAD SPRINGER, IDNR CROSLEY MGR. COMMONWEALTH ENGINEERS, INC. 7256 COMPANY DRIVE INDIANAPOLIS, IN 46237

GEOTECHNICAL ENGINEER: EARTH EXPLORATION, INC 7770 W. NEW YORK ST. INDIANAPOLIS, IN 46214 P: (317)273-1690

F: (317)273-2250 STRUCTURAL ENGINEER: CE SOLUTIONS, INC. 10 SHOSHONE DRIVE CARMEL, IN 46032 P: (317)818-1912 F: (317)818-1911

GENERAL NOTES:

(317)888-1177

THE PURPOSE OF THIS PROJECT IS TO BRING CROSLEY LAKE DAM (STATE ID 40-7) TO STATE COMPLIANCE FOR THE SIGNIFICANT HAZARD RATING.

ONLY VISIBLE ABOVE GROUND UTILITY INFRASTRUCTURE OR ACTUAL FIELD MARKINGS DESIGNATING EXISTANCE OF A BURIED UTILITY OR UTILITY PROVIDING AS-BUILT/BEST AVAILABLE MAPPING ARE INCLUDED ON THE PLANS, WITHIN THE SPECIFIC PROJECT LIMITS. BURIED UTILITIES WERE LOCATED AT THE SITE THROUGH INDIANA 811 LOCATING SERVICE (1-800-382-5549). LOCATION OF SUCH UTILITIES, SHOWN WITHIN THESE PLANS, ARE SUBJECT TO THE ACCURACY STANDARDS AS ESTABLISHED BY INDIANA CODE 8-1-26. NO ATTEMPT HAS BEEN MADE AS PART OF THE SURVEY TO OBTAIN OR SHOW DATA CONCERNING SIZE, DEPTH, CONDITION OR CAPACITY OF SAID BURIED UTILITIES, UNLESS SPECIFICALLY NOTED OR SHOWN ON THE PLANS. THE ENGINEER, OWNER OR ASSOCIATED CONSULTANTS/PERSONNEL DO NOT CERTIFY AS TO THE ACCURACY OR COMPLETENESS OF THE BURIED UTILITIES SHOWN HERE-IN.

3. OTHER BURIED UTILITIES MAY BE IN PROJECT AREA, MARK ALL UTILITIES BEFORE CONSTRUCTION. PRIOR TO ANY EXCAVATION TAKING PLACE ON SITE, CONTACT INDIANA UNDERGROUND 811 AND THE APPROPRIATE UTILITY COMPANY FOR LOCATION OF ALL

4. PORTIONS OF THE EXISTING FEATURES SHOWN WERE DRAWN USING 2013 DIGITAL ORTHO-PHOTOGRAPHY DATA. ALL BEARINGS SHOWN ON THE PLANS ARE ASSUMED.

3. VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION AND REPORT ANY DISCREPANCIES TO THE OWNER.

INFORMATION ON THIS DRAWING IS INTENDED ONLY FOR THE IDENTIFIED PROJECT SHOWN ON THIS DRAWING.

7. INFORMATION ON THIS DRAWING IS INTENDED ONLY FOR THE IDENTIFIED PROJECT SHOWN ON THIS DRAWING.

6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING ALL REQUIREMENTS OF GOVERNMENT AGENCIES AND PERMITS.

8. EROSION CONTROL SILT FENCES OR INLET PROTECTION DEVICES SHALL BE PLACED BELOW ALL AREAS DISTURBED DURING

CONSTRUCTION AND THAT DRAINS TO WATERWAYS, PRIOR TO COMMENCING WITH WORK ACTIVITIES.

ANY DISCREPANCIES TO THE ENGINEER IN WRITING.

9. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO CONSTRUCTION AND/OR ORDERING MATERIAL AND REPORT

10. DURING EXCAVATION OPERATIONS, SEGREGATE OUT ALL TOPSOIL AND STOCKPILE FOR SUBSEQUENT USE IN AREAS TO BE SEEDED OR SODDED. EXTRA CARE SHALL BE TAKEN TO AVOID MIXING STOCKPILED MATERIAL WITH OBJECTIONABLE MATERIAL.

11. THE CONTRACTOR SHALL TAKE ALL RESPONSIBILITY FOR ANY INJURY OR DAMAGE RESULTING TO PERSONS OR PROPERTY BECAUSE OF SUCH WORK.

12. REFER TO THE GEOTECHNICAL REPORT FOR INFORMATION RELATING TO THE EXISTING SUBSURFACE SOIL CONDITIONS. SHEET 3 CONTAINS THE SOIL/ROCK BORING LOCATIONS. REFER TO THE SPECIFICATIONS FOR THE GEOTECHNICAL REPORT AND SOIL/ROCK BORING LOCATIONS, LOGS, FINDINGS AND RECOMMENDATIONS.

13. THE REMOVAL OF EXISTING EMBANKMENT SHALL BE BASED ON THE RECOMMENDATIONS NOTED IN THE GEOTECHNICAL REPORT, THESE PLANS AND AS OBSERVED DURING CONSTRUCTION IN THE PRESENCE OF THE GEOTECHNICAL ENGINEER. THE EXISTING SIDE SLOPES OF THE DAM SHALL BE PROTECTED FROM POSSIBLE EROSION TO DOWNSTREAM AREAS.

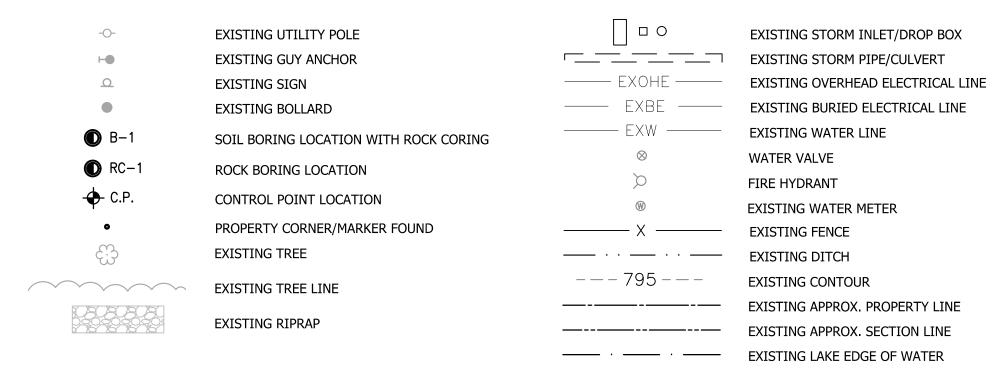
14. CONTRACTOR SHALL PROVIDE A PLAN OF OPERATION, WHICH SHALL INCLUDE: SAFETY MEASURES TO PROTECT THE PUBLIC FROM CONTRACTOR'S WORK, CONSTRUCTION MEANS AND METHODS FOR EACH SECTION OF WORK, CONSTRUCTION PHASING AND PROVISIONS TO MAINTAIN DRAINAGE DURING CONSTRUCTION (INCLUDING HANDLING HIGH INTENSITY STORM EVENTS SUCH AS THE 100-YEAR). ENGINEER AND OWNER MUST APPROVE PLAN OF OPERATION PRIOR TO CONSTRUCTION. EXISTING SPILLWAY SHALL REMAIN IN OPERATION AS LONG AS POSSIBLE DURING CONSTRUCTION, UNTIL SUCH TIME, CONSTRUCTION AND STARTUP OF THE NEW SPILLWAY STRUCTURE IS COMPLETE.

15. CONTRACTOR SHALL BE ALLOWED TO DRAWDOWN AND MAINTAIN THE LAKE AT ELEVATION 684.00. REFER TO CONTRACT DETAILED SPECIFICATIONS FOR DETAILS AND ADDITIONAL REQUIREMENTS.

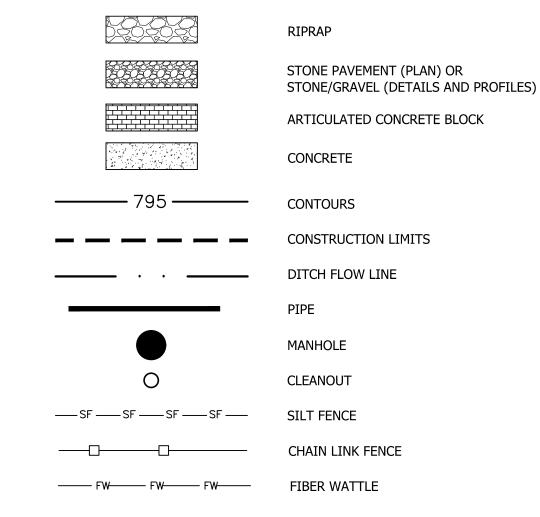
16. IT IS THE INTENT THAT ALL RIPRAP, AND TURF REINFORCEMENT MATERIALS SHALL BE KEYED IN THE SIDE SLOPES ACCORDING TO THE PLANS AND IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. ALL SUCH MATERIALS SHALL BLEND IN SMOOTHLY TO ADJACENT LAND AREAS. ADD ADDITIONAL TOPSOIL WITHIN THE EDGES OF RIP-RAP AREAS WHERE A TRANSITION TO SEEDED AREAS BEGIN IN ORDER TO ACCOUNT FOR SETTLEMENT OF SOIL.

PLAN LEGEND

EXISTING FEATURES



PROPOSED FEATURES



PLAN SHEET INDEX

DRAWING No. DRAWING DESCRIPTION

TITLE SHEET

DRAWING INDEX SHEET, LEGENDS, SYMBOLS, AND PROJECT NOTES

03 SITE SURVEY 04 SITE PLAN

EXISTING PRINCIPAL SPILLWAY CHANNEL IMPROVEMENTS

PRINCIPAL SPILLWAY ARTICULATED CONCRETE BLOCK OVERFLOW LINE 'P' PLAN, PROFILE AND DETAILS

PRINCIPAL SPILLWAY WALL LINE 'W' PLAN AND PROFILE

ACCESS DRIVE & TYPICAL PRINCIPAL SPILLWAY WALL DETAILS

EMERGENCY SPILLWAY LINE 'E' PLAN AND PROFILE

LAKE DRAWDOWN PLAN AND PROFILE

LAKE DRAWDOWN DETAILS

STORM WATER POLLUTION PREVENTION PLAN

STORM WATER POLLUTION PREVENTION DETAILS

MISCELLANEOUS DETAILS MISCELLANEOUS DETAILS

HYDROLOGIC AND HYDRAULICS DATA

17 DAM CENTERLINE PROFILE - LINE 'A'

DAM TYPICAL CROSS SECTIONS - LINE 'A' DAM TYPICAL CROSS SECTIONS - LINE 'A'

DAM TYPICAL CROSS SECTIONS - LINE 'A'

S1-1 GENERAL STRUCTURAL NOTES

S1-2 TYPICAL STRUCTURAL DETAILS - CONCRETE S2-1 OVERALL FOUNDATION PLAN AND RETAINING WALL PROFILE

S2-2 ACCESS DRIVE - ENLARGED FOUNDATION PLAN, SECTIONS, AND DETAILS

S2-3 RETAINING WALL SECTIONS AND DETAILS

S2-4 ISOMETIC VIEWS

<u>1</u> **PROJECT ALTERNATES FOR RE-BID:**

THE PROJECT CONSIST OF FIVE ALTERNATES DISCUSSED IN DETAIL HERE.

ALTERNATE 1: ADD LAKE DRAWDOWN FACILITY

•LOCATION: THROUGH DAM EMBANKMENT - LINE 'D', CROSSING AT DAM CENTERLINE LINE 'A' APPROXIMATE STATION 12+61 (JUST NORTH OF

EMERGENCY SPILLWAY) •SHEET REFERENCES: GENERAL LOCATION SHEET 04, WORK ON SHEETS 10 AND 11

•SPECIFICATION REFERENCE: DETAIL SPECIFICATION SECTION 06 EXCAVATION AND EMBANKMENT, SECTION 08 RIPRAP, SECTION 12 LAKE

DRAWDOWN PRECAST STRUCTURES, PIPE AND APPURTENANCES •WORK DETAIL: INSTALL STR. 101 LAKE DRAWDOWN INTAKE & CONCRETE ANCHOR, REVETMENT RIPRAP LEVELING PAD, NEW STR. 102 60" DIAMETER MH WITH FILLER PIPE AND VALVE, NEW STR. 103 48" DIA. MH WITH VALVE, AND 8" PIPE WITH COLLARS AND INLET SCREEN. NOTE: NEW STR. 104 OUTLET

HEADWALL SHALL REMAIN PART OF THE BASE BID, WHICH SHALL INCLUDE A FOUR (4) FOOT 8" DUCTILE IRON PIPE STUBBED AND CAPPED BEHIND $^{\prime}$ WALL FOR FUTURE CONNECTION. DUCTILE IRON PIPE SHALL MEET LAKE DRAWDOWN PIPE MATERIAL SPECIFICATIONS.

ALTERNATE 2: ADD EMERGENCY SPILLWAY UTILIZING HYDROTURF CS SYSTEM

•LOCATION: DOWNSTREAM DAM EMBANKMENT ON THE SOUTH END OF DAM; CENTERLINE OF EMERGENCY SPILLWAY AT DAM CENTERLINE LINE 'A' APPROXIMATE STATION 13+18.

•SHEET REFERENCES: GENERAL LOCATION SHEET 04, WORK ON SHEETS 09, 12, 15 AND 17. NOTE: ARTICULATED CONCRETE BLOCK MATS ARE SHOWN IN THE ACTUAL PLAN SET. WITHIN DETAILED SPECIFICATION SECTION 19, HYDROTURF CS SYSTEM PRODUCT INSTALLATION DETAILS AND REQUIREMENTS ARE PROVIDED. IN ADDITION, THE HYDROTURF CS SYSTEM EXTENDS BEYOND THE TOE OF THE DAM (WHICH DIFFERS FROM THE ARTICULATED CONCRETE BLOCK MAT INSTALLATION SHOWN ON SHEET 09). FOR THAT REASON, THE HYDROTURF CS SYSTEM, SUBSEQUENTLY SHIFTS THE CONCRETE ARMOR UNIT INSTALLATION DOWNSTREAM AND LESS RIPRAP WILL BE UTILIZED. THE END OF THE RIPRAP SHALL REMAIN AT LINE 'E' STATION 32+15 (AS SHOWN ON SHEET 09).

•SPECIFICATION REFERENCE: DETAIL SPECIFICATION SECTION 06 EXCAVATION AND EMBANKMENT, SECTION 08 RIPRAP, SECTION 10 CONCRETE

ARMOR UNITS, SECTION 17 PERMANENT SEEDING AND SECTION19 HYDROTURF CS SYSTEM •WORK DETAIL: INSTALL EMERGENCY SPILLWAY WITH HYDROTURF CS HARD ARMOR PROTECTION, CONCRETE ARMOR UNITS ENERGY DISSIPATORS AND 42" THICKNESS OF CLASS II RIPRAP. NOTE: BASE BID SHALL INLOUDE PROVIDING A POSITIVE DRAINAGE FOR TWO STAGE FILTER DRAINS FROM

NEW STR. 104 TO OUTLET CHANNEL AS SHOWN ON SHEET 09. BASE BID SHALL MAINTAIN NEW 12'-0" DAM CREST TO ELEV. 702.00.

ALTERNATE 3: ADD ARTICULATED CONCRETE BLOCK MATS IN-LIEU OF HYDROTURF CS SYSTEM FOR EMERGENCY SPILLWAY

•LOCATION: DOWNSTREAM DAM EMBANKMENT ON THE SOUTH END OF DAM; CENTERLINE OF EMERGENCY SPILLWAY AT DAM CENTERLINE LINE 'A' APPROXIMATE STATION 13+18.

•SHEET REFERENCES: GENERAL LOCATION SHEET 04, WORK ON SHEETS 09, 12, 15 AND 17.

•SPECIFICATION REFERENCE: DETAIL SPECIFICATION SECTION 06 EXCAVATION AND EMBANKMENT, SECTION 08 RIPRAP, SECTION 09 ARTICULATED CONCRETE BLOCK MATS, SECTION 10 CONCRETE ARMOR UNITS AND SECTION 17 PERMANENT SEEDING

•WORK DETAIL: THIS ALTERNATE SUBSTITUTES ARTICULATED CONCRETE BLOCK MATS HARD ARMOR IN-LIEU OF THE HYDROTURF CS SYSTEM, AS GENERALLY SHOWN WITHIN THIS PLAN SET. THE TOTAL COST DIFFERENCE, MATERIAL AND INSTALLATION SHOULD BE INCLUDED WITHIN THIS ALTERNATE.

ALTERNATE 4: ADD RIPRAP ON FILTER FABRIC

•LOCATION: ADD UPSTREAM DAM EMBANKMENT, EAST OFFSETS GIVEN; DAM CENTERLINE LINE 'A', FROM APPROX. STATION 10+75, APPROX. 27 FEET LEFT; TO APPROX. STATION 11+00, APPROX. 23 FEET LEFT; TO APPROX. STATION 12+95 APPROX. 23 FEET LEFT; TO APPROX. STATION ENDING AT APPROX. 13+42 STATION 37 FEET LEFT.

•SHEET REFERENCES: GENERAL LOCATION SHEET 04, WORK ON SHEETS 12, 14, 18, 19 AND 20.

•SPECIFICATION REFERENCE: DETAIL SPECIFICATION SECTION 08 RIPRAP

· •WORK DETAIL: INSTALLATION OF RIPRAP (APPROX. 330 SYS PLAN VIEW 2D SCALE), FROM TWO (2) FEET BELOW NORMAL POOL (ELEV.693.65). CONTINUING DOWN THE DOWNSTREAM EMABKMENT AS REQUIRED TO PROVIDE MIN. 2:1 SLOPE. NOTE: BASE BID SHALL BE FOR RIPRAP FROM ELEV. 700.50 TO ELEV. 693.65 (APPROX. 555 SYS PLAN VIEW 2D SCALE).

ALTERNATE 5: ADD PRESSURE GROUTING

•LOCATION: AT PRINCIPAL SPILLWAY CREST AND DAM CREST; DAM CENTERLINE LINE 'A' APPROXIMATE STATION 10+00 TO APPROXIMATE STATION

•SHEET REFERENCES: GENERAL LOCATION SHEET 04, WORK ON SHEETS 05 AND 17.

•SPECIFICATION REFERENCE: DETAIL SPECIFICATION SECTION 13 PRESSURE GROUTING

•WORK DETAIL: INSTALLATION OF ALL PRESSURE GROUTING

DAM IMPROVEMENTS PROJECT

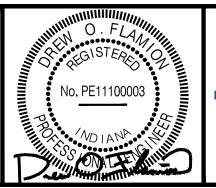
INDIANA DEPARTMENT OF NATURAL RESOURCES

CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE

PROJECT NO. ENG1802321631/E020098

REVISIONS DESCRIPTION REVISED B 1 9/27/2018 REVISE PROJECT ALTERNATES FOR RE-BID DOF **Call** before you dig. OR 1-800-382-5544 (ITS THE LAW)

© 2018 COMMONWEALTH ENGINEERS, INC.





(260) 494-3223

DRAWN BY:

ESIGNED BY:

HECKED BY:

DRAWING NO.

3 OF 26

SCALE: AS NOTED

OR 1-800-382-5544

(ITS THE LAW)

5 9/27/2018 ALTERNATE 4 - UPSTREAM RIPRAP BELOW EL. 693.65

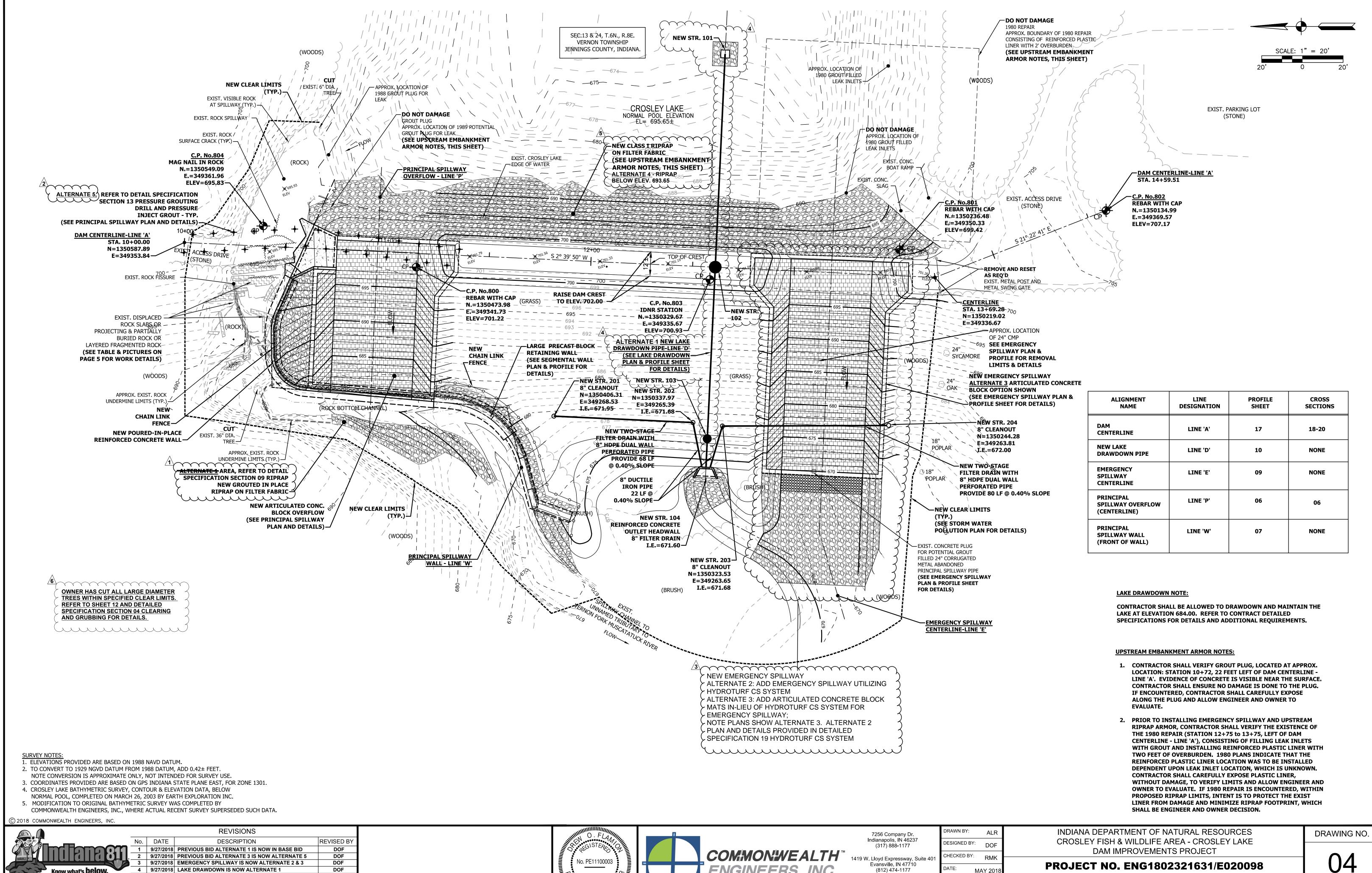
6 9/27/2018 OWNER HAS COMPLETED TREE CUTTING TASK

OR 1-800-382-5544

(ITS THE LAW)

DOF

DOF



9604 Coldwater Road, Suite 203

Fort Wayne, IN 46825

(260) 494-3223

JOB NO:

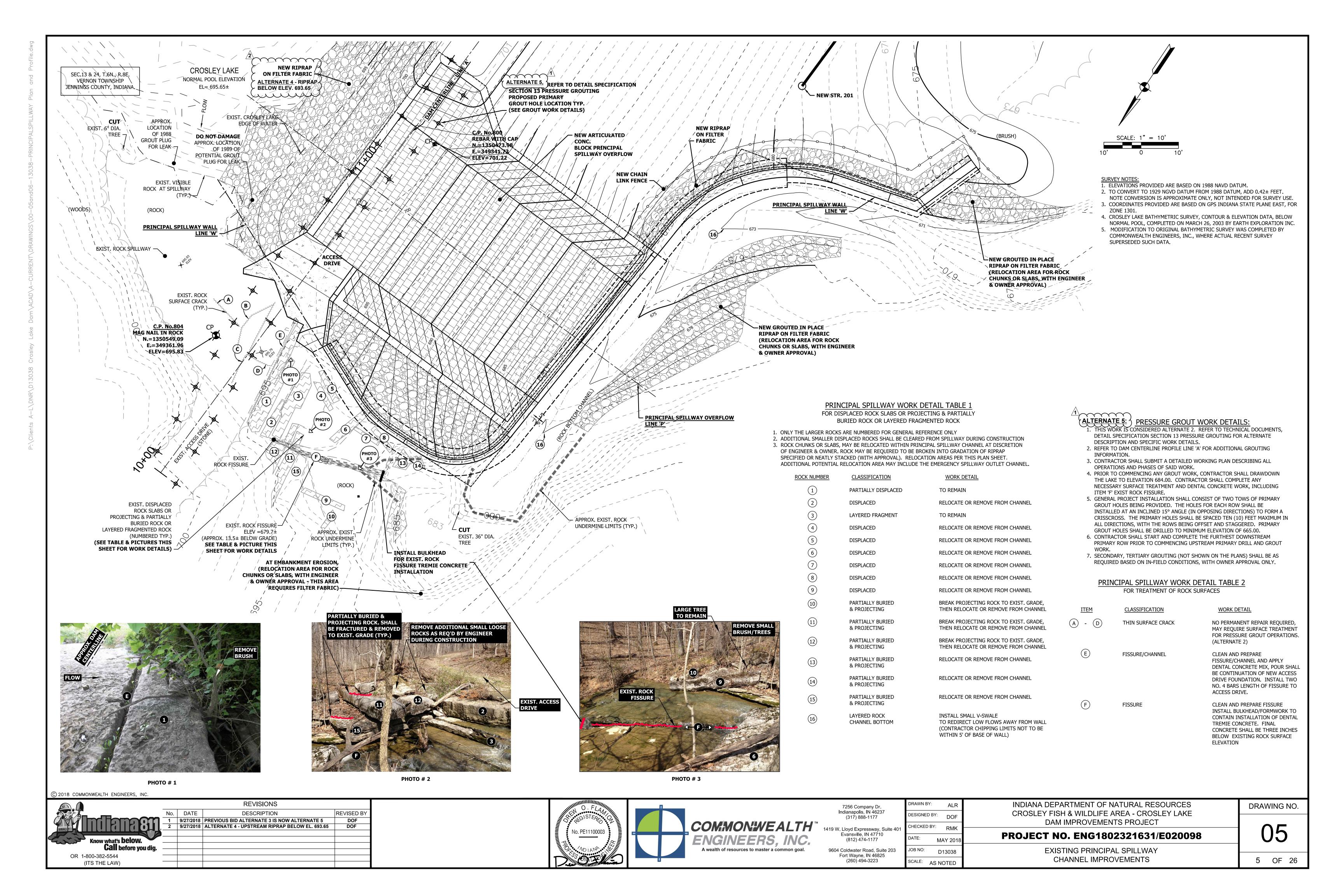
D13038

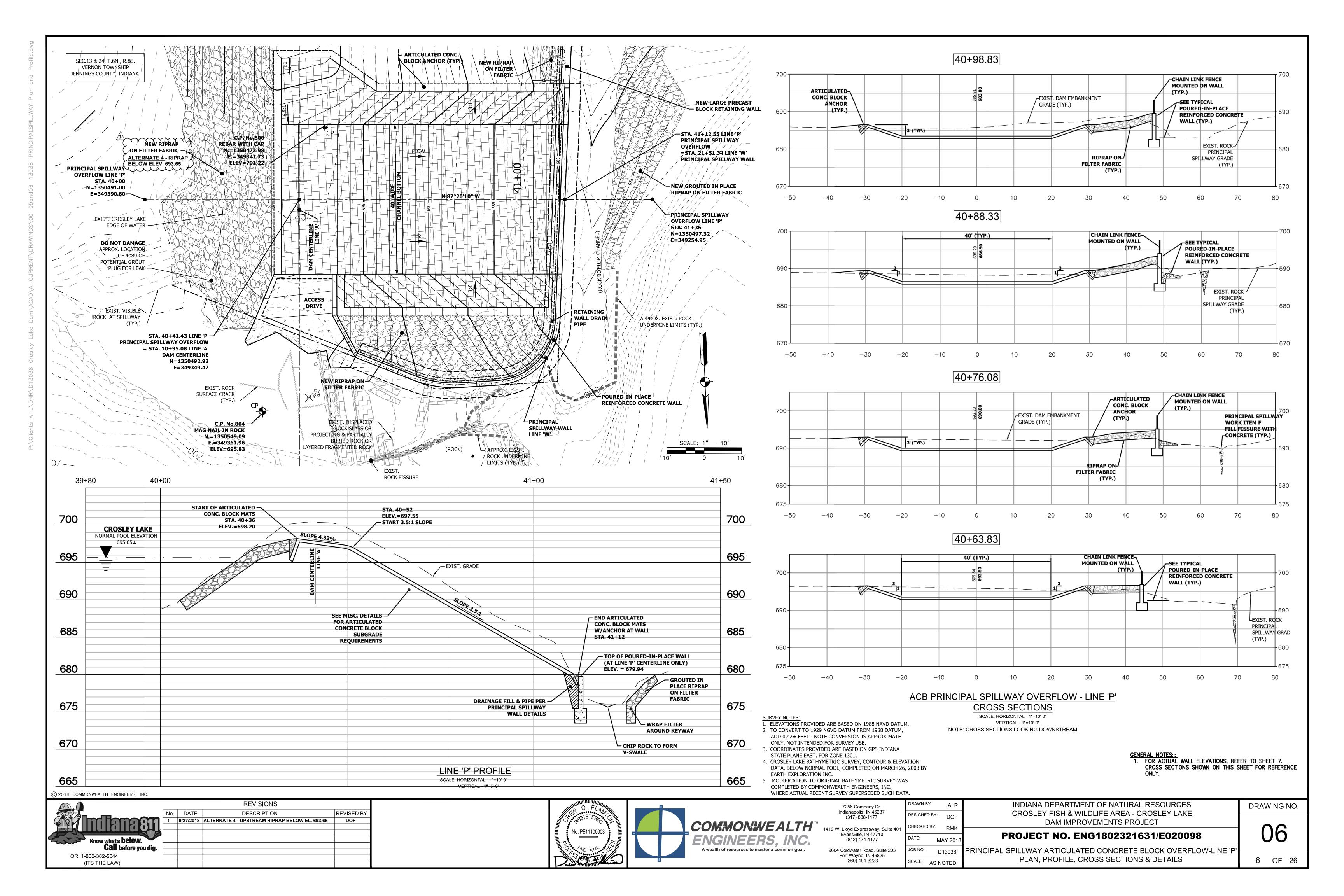
SCALE: AS NOTED

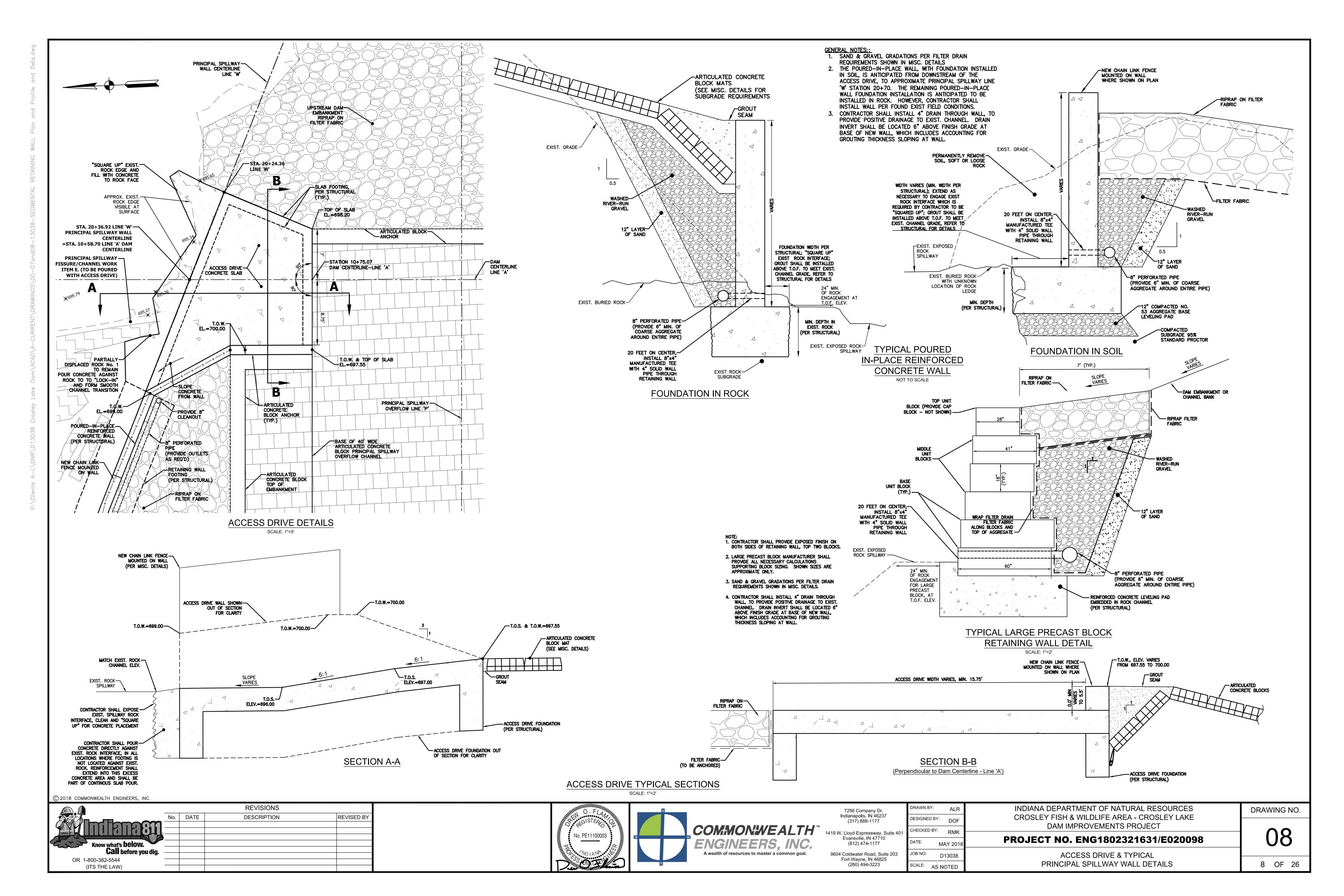
SITE PLAN

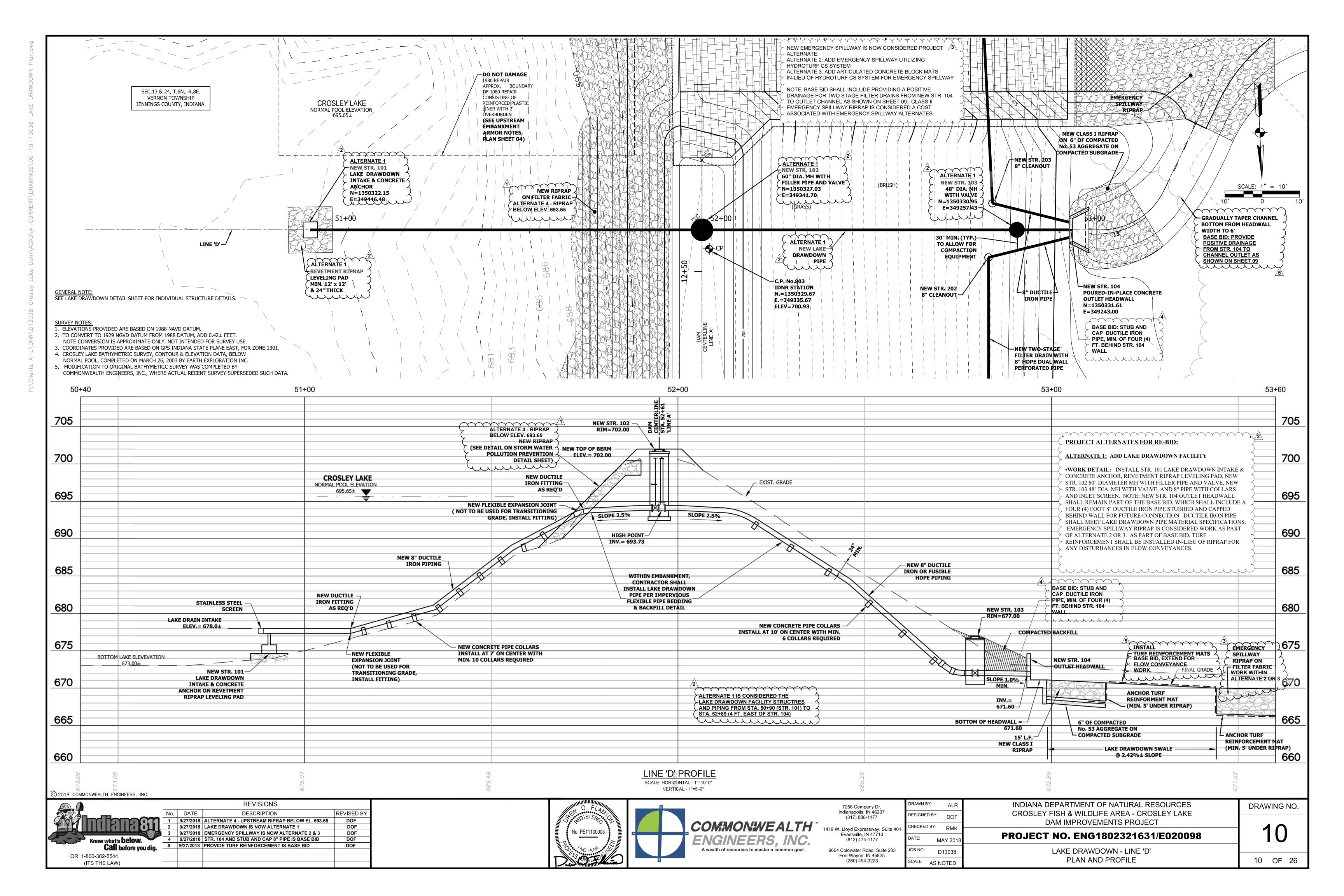
4 OF 26

A wealth of resources to master a common goal.

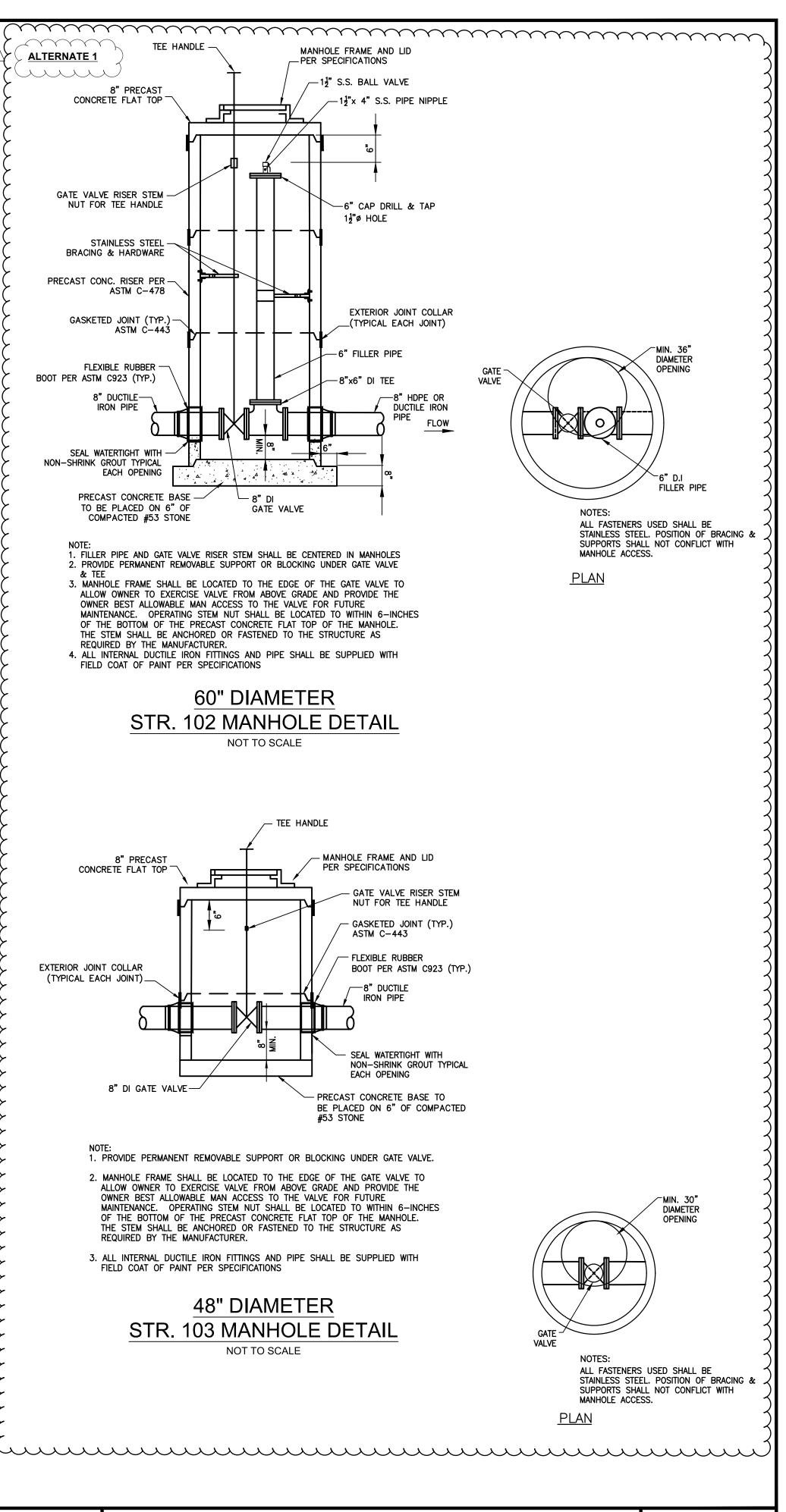








(ITS THE LAW)









7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177 1419 W. Lloyd Expressway, Suite 40 Evansville, IN 47710 (812) 474-1177

9604 Coldwater Road, Suite 203

Fort Wayne, IN 46825

(260) 494-3223

6"x 3/4" S.S. ANCHOR BOLT

_ **(4)** #5

2'-0"

#5 BARS @12" O.C.

-NEW 4" PVC WATERSTOP TO BE PROVIDED AT

CONSTRUCTION JOINTS

-NEW 8" LAKE DRAWDOWN

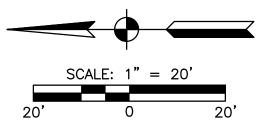
> DRAWN BY: DESIGNED BY: DOF HECKED BY: MAY 20 JOB NO: D13038 SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT

PROJECT NO. ENG1802321631/E020098

LAKE DRAWDOWN DETAILS

DRAWING NO.



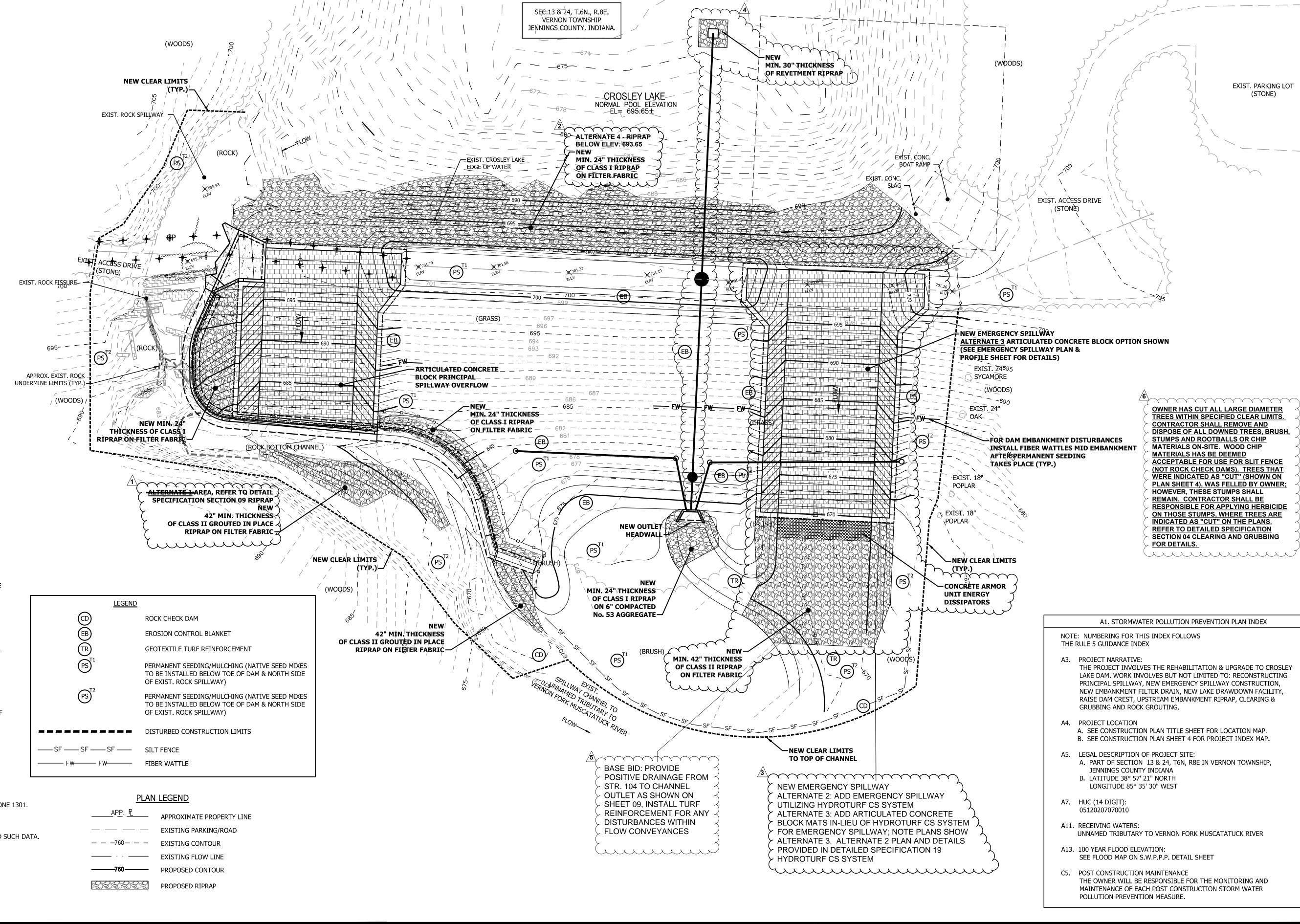
- NOTE: THE FOLLOWING IS A PARTIAL LIST OF THE MEASURES REQUIRED. SEE PLANS AND SPECIFICATIONS FOR ADDITIONAL MEASURES.
- . CONTRACTOR TO COORDINATE LOCATION OF STAGING AREA WITH THE OWNER. CONTRACTOR SHALL ALSO PROVIDE A CONCRETE WASHOUT LOCATION AND VEHICLE FUELING/MAINTAINENCE LOCATION WITH A SYSTEM USED TO CONTAIN THE POLLUTANTS ASSOCIATED WITH THESE ACTIVITIES.
- . CONTRACTOR TO PROVIDE A STABLE CONSTRUCTION ENTRANCE WHERE CONSTRUCTION TRAFFIC WILL BE TRAVERSING BETWEEN SOIL SURFACES AND PAVED ROADWAYS (SEE S.W.P.P.P. DETAILS).
- TEMPORARY CULVERTS SHALL BE REQUIRED AT ALL AREAS WITH CONCENTRATED FLOWS. HEAVY CONSTRUCTION TRAFFIC SHALL HAVE LIMITED ACCESS ALONG THE CREST OF THE DAM.
- 1. ALL EXPOSED FINISH GRADE DISTURBED AREAS, SHALL BE STABILIZED WITH PERMANENT SEEDING, TEMPORARY EROSION CONTROL BLANKETS SHALL BE USED IN CONCENTRATED FLOW AREAS SUCH AS DITCHES, SWALES, AND STREAM BANKS AREAS THAT ARE NOT SUBJECT TO CONCENTRATED FLOWS, BUT HAVE SLOPES THAT ARE 4:1 OR STEEPER SHALL BE MULCH SEEDED. IF DISTURBED AREAS REMAIN INACTIVE FOR MORE THAN 15 DAYS, TEMPORARY SEED SHALL BE INSTALLED FOR TEMPORARY SURFACE STABILIZATION.
- SILT FENCE SHALL BE USED AS A SEDIMENT BARRIER WHERE STORM WATER RUNOFF FROM DISTURBED AREAS ARE EXPECTED TO SHEET DRAIN TO UNDISTURBED AREAS.
- . TEMPORARY SEDIMENT TRAPS SHALL BE USED AT THE DOWNSTREAM END OF DISTURBED AREAS WITHIN DITCHES OR SWALES WHERE CONCENTRATED FLOW OCCURS AND IS EXPECTED TO DRAIN TO UNDISTURBED AREAS OR STORM SEWER END SECTIONS.
- . INLET PROTECTION SHALL BE USED ON STORM STRUCTURES RECEIVING STORM WATER THROUGH OPEN CASTINGS. SILT FENCE INLET PROTECTION SHALL BE USED WHEN SURFACE AROUND INLET IS UNPAVED, AND SAND BAGS SHALL BE USED WHEN SURFACE AROUND INLET IS PAVED.
- B. SEE MISCELLANEOUS DETAIL SHEETS FOR ADDITIONAL STORM WATER POLLUTION PREVENTION MEASURE DETAILS.
- O. AFTER COMPLETION OF CONTRACT, OR AS REQUESTED BY OWNER, THE CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL ITEMS AND ALL ACCUMULATED DEPOSITS. AS REQUIRED, CONTRACTOR SHALL PROVIDE MULCHED SEEDING OR SOD TO ESTABLISH ORIGINAL CONDITIONS PRIOR TO CONSTRUCTION.
- O. ADDITIONAL DISTURBED AREAS, NOT SHOWN WITHIN THIS PLAN, SHALL BE PROVIDED PERMANENT SEEDING PER THE DETAILED SPECIFICATION REQUIREMENTS. THIS MAY INCLUDE, BUT NOT LIMITED TO: ANY BORROW MATERIAL SITES, ACCESS AND STAGING AREAS, AND ANY TEMPORARY CONSTRUCTION ACCESS DRIVES.

GENERAL POLLUTION PREVENTION NOTES:

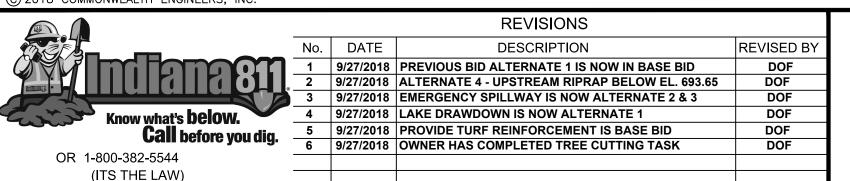
- . OTHER BURIED UTILITIES MAY BE IN PROJECT AREA. THE LOCATION OF UNDERGROUND UTILITIES SHALL BE VERIFIED BY CALLING TOLL FREE 1-800-382-5544 AT LEAST 2 DAYS PRIOR TO ANY DIGGING IN THE AREA.
- . EROSION CONTROL SILT FENCES SHALL BE PLACED BELOW ALL AREAS THAT ARE DISTURBED DURING CONSTRUCTION AND THAT DRAIN TO WATERWAYS.
- DURING EXCAVATION OPERATIONS, SEGREGATE OUT ALL TOPSOIL AND STOCKPILE FOR SUBSEQUENT USE IN AREAS TO BE SEEDED. EXTRA CARE SHALL BE TAKEN TO AVOID MIXING STOCKPILED MATERIAL WITH OBJECTIONABLE MATERIAL.
- ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS. DRAWINGS SHALL NOT BE SCALED FOR CONSTRUCTION
- 5. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- 5. NEITHER THE OWNER NOR THE ENGINEER WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS. SITE OBSERVATION VISITS BY THE ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE SAFETY ITEMS.

SURVEY NOTES:

- L. ELEVATIONS PROVIDED ARE BASED ON 1988 NAVD DATUM
- 2. TO CONVERT TO 1929 NGVD DATUM FROM 1988 DATUM, ADD $0.42\pm$ FEET. NOTE CONVERSION IS APPROXIMATE ONLY, NOT INTENDED FOR SURVEY USE.
- 3. COORDINATES PROVIDED ARE BASED ON GPS INDIANA STATE PLANE EAST, FOR ZONE 1301. 4. CROSLEY LAKE BATHYMETRIC SURVEY, CONTOUR & ELEVATION DATA, BELOW
- NORMAL POOL, COMPLETED ON MARCH 26, 2003 BY EARTH EXPLORATION INC. MODIFICATION TO ORIGINAL BATHYMETRIC SURVEY WAS COMPLETED BY
- COMMONWEALTH ENGINEERS, INC., WHERE ACTUAL RECENT SURVEY SUPERSEDED SUCH DATA.



© 2018 COMMONWEALTH ENGINEERS, INC.







7256 Company Dr. Indianapolis, IN 46237 1419 W. Lloyd Expressway, Suite 40 Evansville, IN 47710 (812) 474-1177 9604 Coldwater Road, Suite 203

Fort Wayne, IN 46825

(260) 494-3223

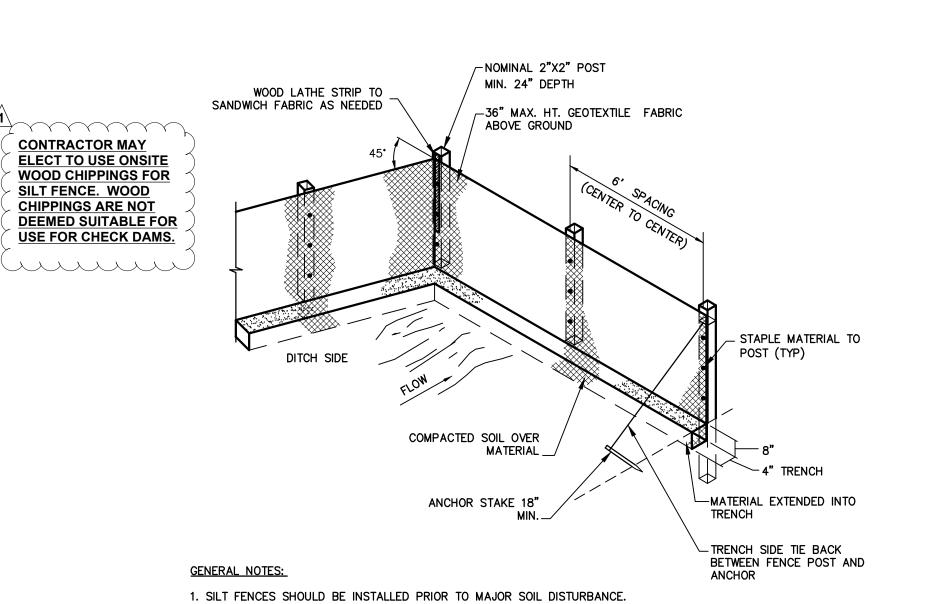
DRAWN BY: DESIGNED BY: DOF HECKED BY: MAY 201 JOB NO: D13038 SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT

PROJECT NO. ENG1802321631/E020098

STORM WATER POLLUTION PREVENTION PLAN

DRAWING NO.



2. FENCES SHALL BE INSTALLED BETWEEN THE TRENCH AND ANY DRAINAGE

3. FENCES SHALL ALSO BE INSTALLED AROUND THE STOCKPILED SOILS. 4. THE GEOTEXTILE SHALL BE FREE FROM DEFECTS, TEARS, PUNCTURES, FLAWS, DETERIORATION OR DAMAGE INCURRED DURING MANUFACTURE,

TRANSPORTATION, STORAGE, OR INSTALLATION.

5. TIE BACKS SHALL BE PLACED AS REQUIRED.

DITCHES OR SWALES.

WOOD STAKE-2"x 2"x 2'-6" MIN. FIBER WATTLE

ROLLED EROSION CONTROL PRODUCT (SILT FENCE ALTERNATIVE)

NOT TO SCALE

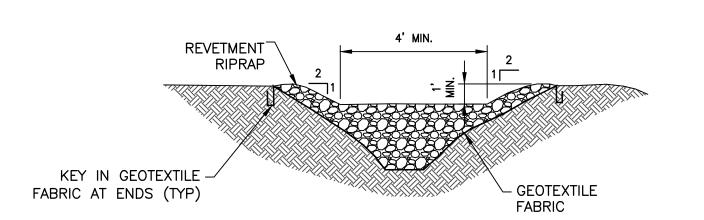
Map Unit Legend

Jennings County, Indiana (IN079)				
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
BlcC2 Blocher, soft black shale substratum-Jennings-Deputy silt loams, 6 to 12 percent slopes, eroded		0.9	4.3%	
BlkE2	Bonnell-Blocher-Hickory silt loams, 12 to 25 percent slopes, eroded	0.5	2.3%	
CcaG	Caneyville-Rock outcrop complex, 25 to 60 percent slopes	2.2	10.4%	
CcbC2	Caneyville-Zenas silt loams, karst, rolling, eroded	0.2	1.2%	
CcgD2	Caneyville and Grayford silt loams, 12 to 25 percent slopes, eroded	8.7	41.5%	
DtwC2	Deputy silt loam, 6 to 15 percent slopes, eroded	4.3	20.8%	
W	Water	2.6	12.6%	
ZnsB	Zenas silt loam, karst, undulating	1.5	7.0%	
Totals for Area of Interest		20.9	100.0%	

SOILS MAP LEGEND



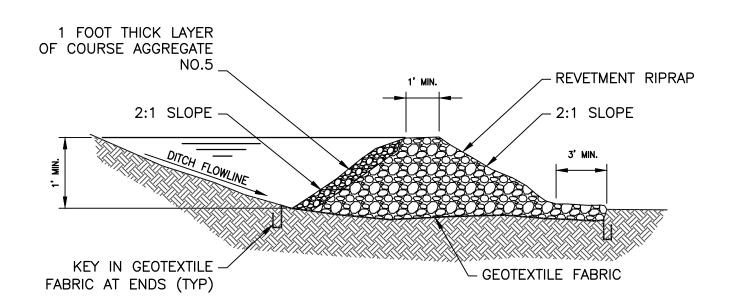
SOILS MAP



SILT FENCE

NOT TO SCALE

SECTION PERPENDICULAR TO FLOWLINE



SECTION PARALLEL TO FLOWLINE

GENERAL NOTES:

- 1. RIPRAP DITCH CHECK DAMS SHALL BE PLACED SUCH THAT THE TOP OF THE DOWNSTREAM CHECK DAM IS AT THE SAME ELEVATION AS THE TOE OF THE ADJACENT UPSTREAM CHECK DAM.
- 2. AFTER COMPLETION OF CONTRACT, OR AS REQUESTED BY OWNER, THE CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL ITEMS, REMOVE ALL ACCUMULATED DEPOSITS AND, AS REQUIRED, SEED AND MULCH OR SOD AS REQUIRED TO ESTABLISH AREA TO CONDITION PRIOR TO CONSTRUCTION.

OF SAND BAGS AND EXTEND OVER THE SAND BAGS. 10'-0" MIN. -SANDBAGS - STACK IN A "RUNNING BOND" -POLYETHYLENE LINING (2 LAYERS - 10 MILLIMETERS THICK MIN.) THE LINING SHOULD BE HELD IN PLACE BY A LAYER OF SAND BAGS AND EXTEND OVER THE SAND BAGS. -WHITE BACKGROUND & RED TEXT PLAN VIEW __2"x2" POST NOTE: DUE TO SITE CONSTRAINTS THE MINIMUM INTERIOR DIMENSION MAY BE ADJUSTED TO FIT THE SITE. THE STRUCTURE'S INTERIOR FOOTAGE OF 100 S.F. MUST BE FINISH GRADE MAINTAINED AND THE CONTRACTOR SHALL SUBMIT ANY DESIGN ALTERATIONS TO THE ENGINEER. CONCRETE WASHOUT STRUCTURE SHALL BE RE-LOCATED CLOSE TO AREAS RECEIVING CONCRETE, AS CONSTRUCTION PROGRESSES. 36" MIN. OR STABLE WEIGHTED -EARTH OR PAVEMENT BASE ON PAVEMENT

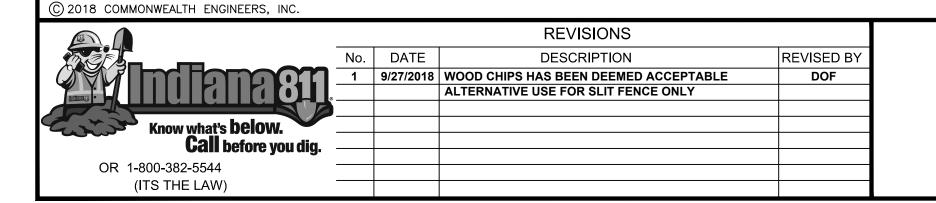
CONCRETE WASHOUT DETAIL NOT TO SCALE

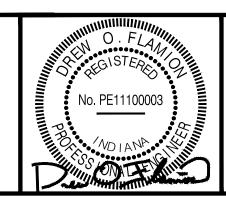
2000 0 100 S. Rd. FLOOD HAZARD BOUNDARY MAP JENNINGS COUNTY INDIANA 150 S. Rd UNINC. AREAS PAGE 4 OF 6 (SEE MAP INDEX FOR PAGES NOT PRINTED) STATE GAME RESERVE CROSLEY MAP REVISED: AUGUST 26, 1977 CONVERTED BY LETTER EFFECTIVE 11/1/95 COMMUNITY-PANEL NUMBER 180108 0004 A S. DEPARTMENT OF HOUSING ND URBAN DEVELOPMENT 300 S. This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.go

FLOOD MAP

ROCK CHECK DAM DETAIL

NOT TO SCALE







-SANDBAGS - STACK IN

(2 LAYERS - 10 MILLIMETERS THICK MIN.) THE LINING SHOULD BE HELD IN PLACE BY A LAYER

A "RUNNING BOND"

POLYETHYLENE LINING

7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177 419 W. Lloyd Expressway, Suite 401 Evansville, IN 47710 (812) 474-1177	DR
	DE
	СН
	DA
9604 Coldwater Road, Suite 203	JO

Fort Wayne, IN 46825

(260) 494-3223

DRAWN B	Y :	ALR	
DESIGNED	BY:	DOF	
CHECKED BY:		RMK	
DATE:	M	AY 2018	
JOB NO:	D	13038	
SCALE:	AS N	OTED	

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT

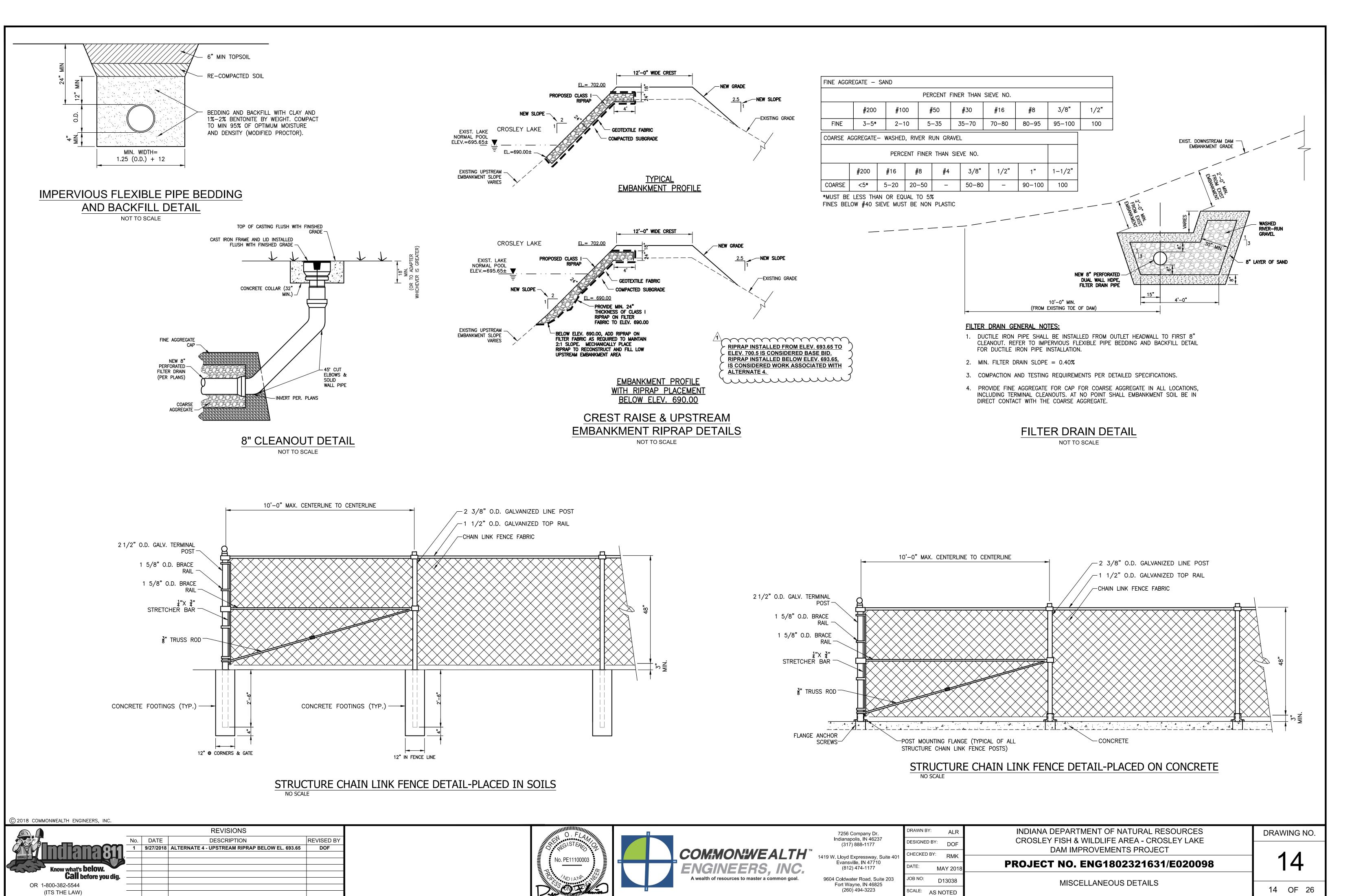
PROJECT NO. ENG1802321631/E020098

DRAWING NO.

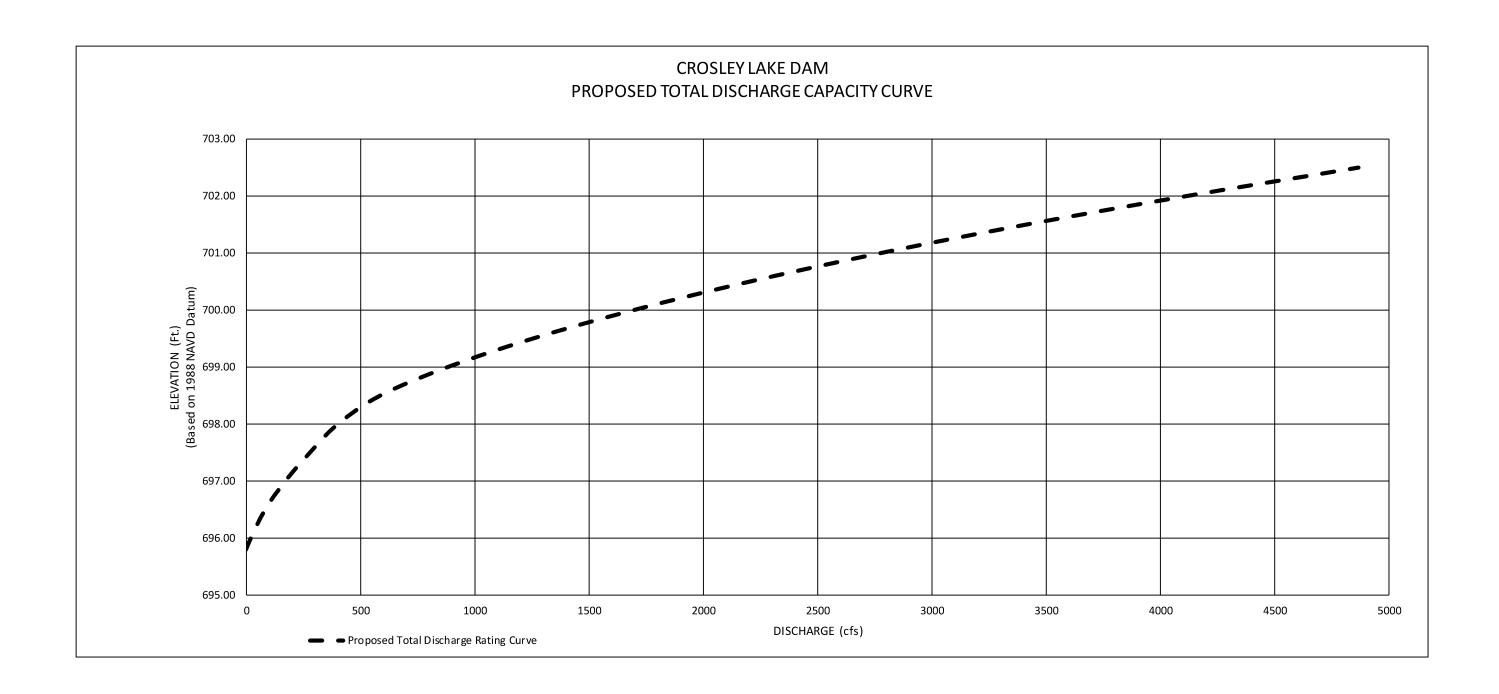
APPROXIMATE SCALE IN FEET

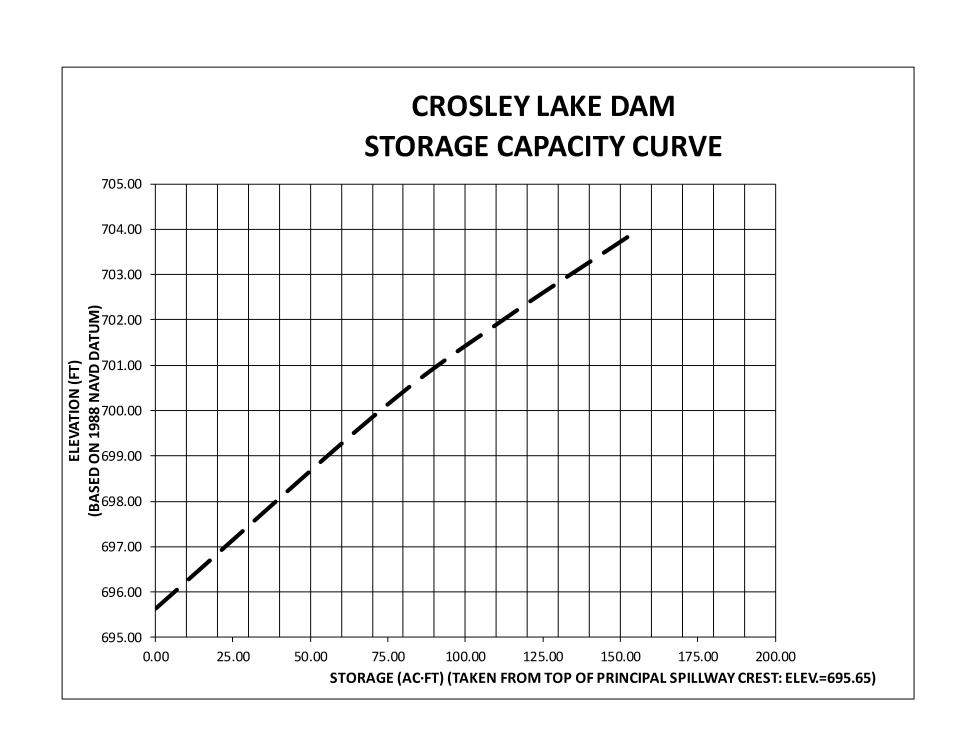
STORM WATER POLLUTION PREVENTION 13 OF 26 **DETAILS**

(ITS THE LAW)



(260) 494-3223





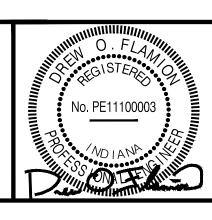
HAZARD CLASSIFICATION	SIGNIFICANT
DRAINAGE AREA (SQ MI)	APPROX. 1.01 SQ MI
TIME OF CONCENTRATION, TC (HRS)	0.82 HRS.
RUNOFF CURVE NUMBER	74
(AMC II CONDITIONS)	II
STRUCTURAL DATA	
PRINCIPAL ROCK SPILLWAY	DOCK/CONCRETE/DIDDAD
MATERIAL	ROCK/CONCRETE/RIPRAP 240 FT.
LENGTH	
SIZE	VARIES - IRREGULAR CHANNEL
HEIGHT	WALL HEIGHT VARIES: 3 FT TO 8 FT
WIDTH	VARIES: 25 FT TO 35 FT
CREST ELEV/AREA WEIR LENGTH	695.65 EX. ROCK CHANNEL 32 FT EX. ROCK CHANNEL
WEIN LENGTH	3211 EX. NOCK CHANNEL
PRINCIPAL SPILLWAY OVERFLOW MATERIAL	ARTICULATED CONCRETE BLOCK
BASE WIDTH	40 FT
SIDE SLOPE	3:1
CREST ELEV/AREA/STOR	698.20
SHAPE	TRAPEZOID
N' VALUE	0.025 ACB
LENGTH/SLOPE OF SPILLWAY	
ARMORING	ARTICULATED CONCRETE BLOCK
EMERGENCY SPILLWAY	ARTICLE ATER CONCRETE BLOCK/CONCRETE LINETS/RIPPA
MATERIAL	ARTICULATED CONCRETE BLOCK/CONCRETE UNITS/RIPRA
BASE WIDTH	40 FT
SIDE SLOPE	3:1
CREST ELEV/AREA/STOR	698.20
SHAPE	TRAPEZOID
N' VALUE	0.025 ACB
LENGTH/SLOPE OF SPILLWAY ARMORING	/ 160 FT/1.0% CREST TO 3.25:1 SLOPE TO 0.001% AT TOE ARTICULATED CONCRETE BLOCK/CONCRETE UNITS/RIPRA
ANWONING	ANTICOLATED CONCRETE BLOCK/CONCRETE ONITS/KIPKA
TOP OF DAM	
ELEV	702.00
AREA	NORMAL POOL (EL. 695.65±) = 14.6± AC
-	MAX WATER SURFACE (EL. 702.00±)= 21.1± AC
STORAGE	92.95 AC/FT FROM NORMAL POOL (695.65) TO
	CREST OF DAM -MAX WATER SURFACE (702.00)
FREEBOARD	3.8 FT (100-YR FREQUENCY)
	0.6 FT (50% PMP)
LENGTH OF DAM	340 FT
CHANNEL OUTLET ELEV	670.00

HYDROLOGIC/HYDRAULIC	<u>DATA</u>
DESCRIPTION (UNITS)	VALUE
BASIN HYDROLOGY	RURAL
RAINFALL (IN)	13.75 IN
DURATION (HRS)	6 HRS
%PMP OR - YR FREQUENCY	50% PMP
RUNOFF (IN)	25.29 IN
RUNOFF (ACRE-FT)	1,736 AC-FT.
PEAK INFLOW (CFS)	3,714 CFS
PRINCIPAL/EMERGENCY SPILLWAY	
MAX. DISCHARGE - 50% PMP (CFS)	
PRIMARY ROCK SPILLWAY	1,639 CFS
40' WIDE PRINCIPAL SPILLWAY OVERFLOW	754 CFS
40' WIDE EMERGENCY SPILLWAY	873 CFS
MAX. VELOCITY - AT OPENING (FPS)	
PRIMARY ROCK SPILLWAY	8 FPS
40' WIDE PRINCIPAL SPILLWAY OVERFLOW	6 FPS
40' WIDE EMERGENCY SPILLWAY	6 FPS
MAX. VELOCITY - EXIT CHANNEL (FPS)	
PRIMARY ROCK SPILLWAY	18 FPS
40' WIDE PRINCIPAL SPILLWAY OVERFLOW	25 FPS
40' WIDE EMERGENCY SPILLWAY	27 FPS
BREACH ANALYSIS	N/A
SHAPE OF BREACH	N/A
WIDTH (FT)	N/A
SIDESLOPE	N/A
TIME TO FAILURE (HRS)	N/A
IMMEDIATE DOWNSTREAM ELEV. (FT)	
PRIMARY ROCK SPILLWAY	670.00
40' WIDE PRINCIPAL SPILLWAY OVERFLOW	675.00 - 678.00
40' WIDE EMERGENCY SPILLWAY	670.00

IF EMERGENCY SPILLWAY IS NOT CONSTRUCTED WITH ALTERNATE 2 OR 3, FACILITY WILL NOT SAFELY PASS 50% PMP EVENT. Turumum

© 2018 COMMONWEALTH ENGINEERS, INC. **REVISIONS** No. DATE

REVISED BY DESCRIPTION 9/27/2018 FACILITY HYDRAULIC CAPACITY NOTE DOF Know what's **below. Call** before you dig. OR 1-800-382-5544 (ITS THE LAW)







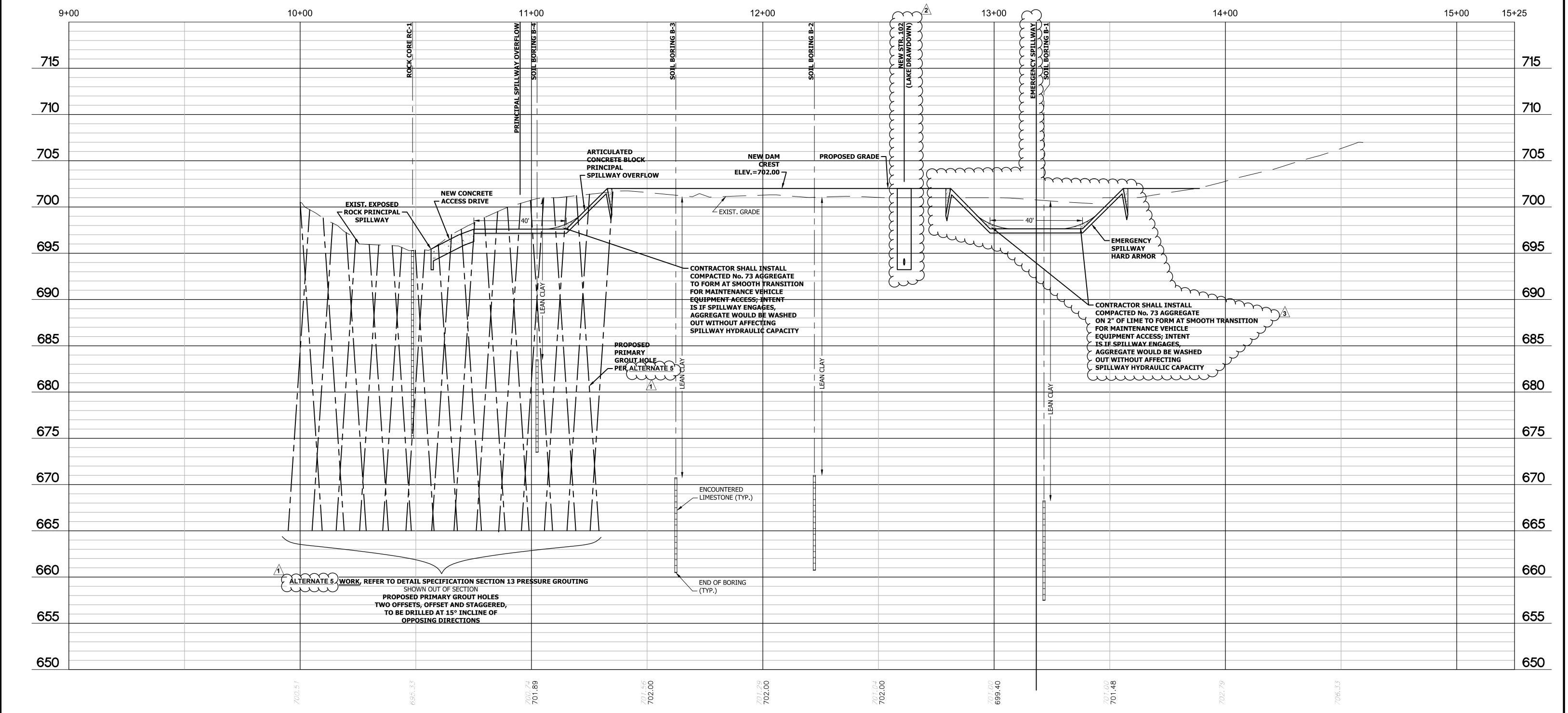
DRAWN BY: DESIGNED BY: DOF CHECKED BY: RMK MAY 2018 JOB NO: D13038

SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT PROJECT NO. ENG1802321631/E020098

DRAWING NO.

HYDROLOGIC AND HYDRAULICS DATA 16 OF 26



DAM CENTERLINE - PROFILE LINE 'A' SCALE: HORIZONTAL - 1"=20'-0"

VERTICAL - 1"=5'-0"

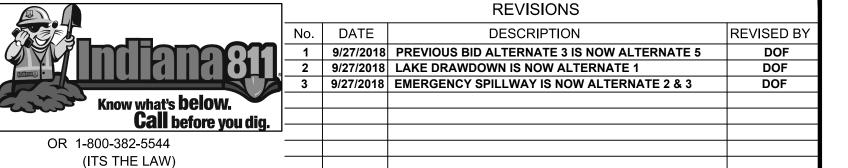
SURVEY NOTES:

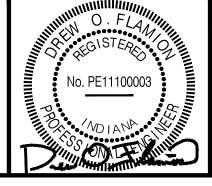
1. ELEVATIONS PROVIDED ARE BASED ON 1988 NAVD DATUM.

- 2. TO CONVERT TO 1929 NGVD DATUM FROM 1988 DATUM, ADD 0.42± FEET. NOTE CONVERSION IS APPROXIMATE ONLY, NOT INTENDED FOR SURVEY USE. 3. COORDINATES PROVIDED ARE BASED ON GPS INDIANA STATE PLANE EAST, FOR
- ZONE 1301. 4. CROSLEY LAKE BATHYMETRIC SURVEY, CONTOUR & ELEVATION DATA, BELOW NORMAL POOL, COMPLETED ON MARCH 26, 2003 BY EARTH EXPLORATION INC
- 5. MODIFICATION TO ORIGINAL BATHYMETRIC SURVEY WAS COMPLETED BY

COMMONWEALTH ENGINEERS, INC., WHERE ACTUAL RECENT SURVEY SUPERSEDED SUCH DATA.

© 2018 COMMONWEALTH ENGINEERS, INC.







7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177	DRA
	DES
19 W. Lloyd Expressway, Suite 401 Evansville, IN 47710 (812) 474-1177	CHE
	DATI
9604 Coldwater Road, Suite 203 Fort Wayne, IN 46825	JOB

(260) 494-3223

DRAWN BY:	ALR	
DESIGNED BY:	DOF	
CHECKED BY:	RMK	
DATE:	MAY 2018	
JOB NO:	D13038	

SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT

PROJECT NO. ENG1802321631/E020098

DAM CENTERLLINE PROFILE - LINE 'A'

17 OF 26

DRAWING NO.

(ITS THE LAW)

GENERAL

- 1. The structure has been designed for the in-service loads only. The methods, procedures, and sequences of construction are the responsibility of the Contractor. Supporting formwork for the concrete construction shall not be removed before the concrete has gained sufficient strength to safely support the dead and superimposed loads which will be subsequently applied. The Contractor shall take all necessary precautions to maintain and ensure the integrity of the structure at all stages of construction.
- 2. All work shall be performed in accordance with the Indiana Building Code, 2014 Edition (2012 International Building Code, first printing, with Indiana Amendments).
- 3. Do not determine dimensions by "scaling" off the plans. The Contractor shall accept all risk associated with "scaling" and shall be responsible for all inadequate work resulting therefrom. Questions regarding missing or conflicting dimensions shall be directed, in writing, to the Structural Engineer.
- 4. Principal openings in the structure are indicated on the structural drawings. Refer to the architectural. mechanical, electrical, and plumbing drawings for sleeves, curbs, inserts, etc. not herein indicated. Openings in slabs with a maximum side dimension or diameter of 10 inches or less shall not require additional framing or reinforcement, unless noted otherwise. The location of sleeves or openings not shown in structural members shall be approved by the Structural Engineer.
- 5. The location of sleeves or openings not shown in structural members shall be approved by the Structural Engineer.

FOUNDATIONS

- 1. Exterior footings shall bear 3'-0" minimum below finish grade and shall bear on undisturbed soil.
- 2. Foundation excavation and all other soils related work shall be performed in accordance with the geotechnical engineering report prepared by Earth Exploration, Inc. dated April 3, 2009 and all associated supplements (EEI Report No. E019901B).
- 3. Foundation and soils related work shall be performed under the direct supervision of a qualified Geotechnical Engineer.
- 4. Foundation excavations shall be made to plan elevations. The soil conditions beneath foundations shall then be inspected by a qualified Geotechnical Engineer. If the underlying soils are found to be unacceptable, one of the following procedures shall be followed:
- A. Remove the unacceptable soil and backfill with an engineered structural fill in accordance with the geotechnical engineering report or inspecting Geotechnical Engineer.
- B. Lower the footing to an acceptable soil. Contact the Structural Engineer for potential modifications to the foundation system.
- 5. Subgrade structural elements subjected to differential lateral soil pressure shall be adequately braced until the structural elements which provide lateral restraint have been placed and allowed to cure for a minimum of 7 days.
- 6. Excavations for foundations shall be cleaned and hand tamped to a uniform surface. Foundation excavations shall be adequately protected against detrimental change in condition from disturbance, rain, freezing, etc. Surface runoff shall not be allowed to enter the excavation.
- 7. Foundation conditions noted during construction, which differ from those described in the geotechnical report shall be reported to the Structural Engineer and Geotechnical Engineer before further construction is attempted.
- 8. Center all column and wall footings under the column or wall above unless otherwise indicated.
- 9. Subgrade structural elements subjected to differential lateral soil pressure shall be adequately braced until the structural elements which provide lateral restraint have been placed and allowed to cure for a minimum of 7 days or reached 70-percent of its specified 28-day compressive strength (fc).

CONCRETE

- 1. Reinforced concrete has been designed in accordance with the latest editions of the Building Code Requirements for Reinforced Concrete (ACI 318) and Environmental Engineering Concrete Structures (ACI 350R) by the American Concrete Institute (ACI).
- 2. Slabs-on-grade shall be constructed in accordance with the latest edition of the Guide for Concrete Floor and Slab Construction (ACI 302.1R).
- 3. Mixing, transporting, and placing of concrete shall conform to the latest edition of the Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete (ACI 211.1) and the Standard Specifications for Structural Concrete (ACI 301). Concrete curing shall conform to the latest editions of the Standard Practice for Concrete Curing (ACI 308) and the Standard Specification for Curing Concrete (ACI 308.1). In case of a discrepancy, the plans and specifications shall govern.
- 4. Unless noted otherwise, concrete shall have natural sand fine aggregate and normal weight coarse aggregates conforming to ASTM C33, and Type II Portland Cement conforming to ASTM C150. The Contractor shall submit a mix design for each proposed class of concrete. Mix designs shall indicate proportions by weight, water-cement ratio, slump, air content, synthetic fiber size and quantity, sieve analyses of fine and coarse aggregates, standard deviation analysis, and required average strength and documentation of average strength verifying compliance with ACI 318. The Contractor shall not vary from the mix design without approval from the Structural Engineer.
- 5. Unless noted otherwise, fly ash may be used as a pozzolan to replace a portion of the Portland Cement in a concrete mix. Fly ash, when used, shall conform to ASTM C618, Type C. Concrete mixes using fly ash shall be proportioned to account for the properties of the specific fly ash used and to account for the specific properties of the fly ash concrete thus resulting. The ratio of the amount of the fly ash to the total amount of fly ash plus cement in the mix shall not exceed 20 percent.
- 6. Ground granulated blast-furnace slag (GGBFS) may be used to replace a portion of the Type II Portland Cement in a concrete mix. Ground granulated blast-furnace slag, when used, shall conform to ASTM C989, Grade 100 or 120. Concrete mixes using GGBFS shall be proportioned to account for the properties of the specific GGBFS used and to account for the specific properties of the GGBFS concrete thus resulting. The ratio of the amount of the GGBFS to the total amount of GGBFS plus cement in the mix shall not exceed 40 percent.
- 7. Water-reducing admixtures conforming to ASTM C494 may be used in the concrete mix design. Maximum slump shall be 5 inches for mixes containing water-reducing admixtures and 5 to 8 inches for mixes containing high range water-reducing admixtures.
- 8. Concrete compressive strength tests shall be performed in accordance with ASTM C39. Copies of the test results shall be forwarded to the Structural Engineer. One set of specimens shall be taken for each day's pour of appreciable size and for each 50 cubic yards in accordance with the latest edition of ASTM C31. Each set shall include one specimen tested at 7 days, 2 specimens tested at 28 days and one specimen retained in reserve. These test cylinders shall be laboratory cured.
- 9. When the ambient temperature is expected to fall below 40 degrees during the course of a concrete pour or subsequent curing period, it shall be placed and cured in accordance with the latest edition of Cold Weather Concreting (ACI 306R) and an additional set of concrete test cylinders shall be made. These cylinders shall be stored immediately adjacent to, and cured under the same conditions as the building concrete. Special curing boxes are not permitted for these test cylinders.
- 10. Concrete mixed, transported, placed, and cured under conditions of high ambient temperature, low humidity, solar radiation, or high winds shall conform to the latest edition of Hot Weather Concreting (ACI 305R) and an additional set of concrete test cylinders shall be made. These cylinders shall be stored immediately adjacent to, and cured under the same conditions as the building concrete. Special curing boxes are not permitted for these test cylinders.
- 11. Slump tests shall be made prior to and following the addition of plasticizers. Where concrete is placed by pumping methods, concrete for test cylinders and slump tests shall be taken at the point of final

- 12. Water shall not be added to the concrete at the job site. The Contractor is responsible for coordinating a pumpable and workable mix without the addition of water at the job site. The use of plasticizers, retardants and other additives shall be at the option of the Contractor subject to the approval of the Structural Engineer. Follow the recommendations of the manufacturer for the proper use of additives. Use of calcium chloride or other chloride bearing salts is prohibited.
- 13. Place concrete in a manner so as to prevent segregation of the mix. Delay floating and troweling operations until the concrete has lost surface water sheen or all free water. Do not sprinkle free cement on the slab surface. Finishing of slab surfaces shall conform to the latest editions of ACI 302.1R and ACI 304R (Guide for Measuring, Mixing, Transporting and Placing Concrete).
- 14. Maintain concrete in a moist condition for at least 5 days at ambient temperatures above 70 degrees, and at least 7 days at ambient temperatures above 50 degrees. Curing compounds or moisture retention covers shall be used for all non-formed surfaces. Formed surfaces shall be cured by leaving forms in place. During hot, dry weather, keep forms moist by sprinkling. When forms are removed prior to the end of the curing period, apply curing compound to the exposed surfaces.
- 15. Protect finished concrete surfaces from damage, rain, hail, running water, other injurious effects.
- 16. Protect the concrete surface between finishing operations on hot, dry days or any time plastic shrinkage cracks could develop by using wet burlap, plastic membranes or fogging.
- 17. Horizontal and vertical joints are not permitted in concrete construction except where indicated.
- 18. Construction joints and/or contraction joints at locations other than where indicated shall be submitted to the Structural Engineer for approval.
- 19. Construction joints shall be prepared by roughening the contact surface in an approved manner to a full amplitude of approximately 1/4 inch leaving the contact surface clean and free of laitance.
- 20. Provide 3/4 inch chamfers on all exposed corners of concrete except those abutting masonry
- 21. Earth cuts shall not be used as forms ("bank forming") for vertical or sloping surfaces unless otherwise approved by the Structural Engineer. Where bank forming is permitted, the concrete element shall be increased at least 3 inches on all sides exposed to earth to account for possible soil contamination during concrete placement.

CONCRETE SCHEDULE

CONCRETE SCHEDULE							
CLASS	f' _C	AIR CONTENT	MIN. CEMENT: LB/CY (SACKS/CY)	MAX. WATER/ CEMENT: RATIO	CONCRETE PLACEMENT	REMARKS	
А	4,500 psi	6% ± 1%	611 (6.5)	0.45	footings and retaining walls		

REINFORCING STEEL

- 1. Reinforcing bar detailing, fabricating, and placing shall conform to the latest edition of the following standards: Specifications for Structural Concrete for Buildings (ACI 301), ACI Detailing Manual (SP66). The latest editions of Concrete Reinforcing Steel Institute's Reinforcing Bar Detailing and Placing Reinforcing Bars may also be used.
- 2. Provide standard bar chairs, slab bolsters, spacers, etc. as required to maintain concrete protection specified. Reinforcing steel shall be tied to prevent displacement during concrete placement.
- 3. Reinforcement bars shall not be tack welded, welded, heated or cut unless otherwise indicated or approved by the Structural Engineer.
- 4. Welding of reinforcement bars, when approved by the Structural Engineer, shall conform to the latest edition of American Welding Society Standard D1.4. Electrodes for shop and field welding of reinforcement bars shall conform to ASTM A233. Class E90XX.
- 5. Concrete cover over reinforcement, unless otherwise noted, shall be as specified in the latest editions of ACI 318 and ACI 350 with the most stringent requirements governing.
- 6. Unless noted otherwise, splicing of reinforcing bars shall conform to the latest edition of ACI 318.

CONCRETE REINFORCING STEEL LAP SPLICE SCHEDULE					
BAR SIZE	TENSION	COMPRESSION			
BAR SIZE	TOP BAR	OTHER	SPLICE		
#3	21"	16"	12"		
#4	28"	24"	15"		
#5	35"	30"	19"		
#6	42"	36"	23"		
#7	49"	42"	26"		
#8	56"	48"	30"		
#9	63"	57"	34"		
#10	76"	66"	38"		
#11	93"	72"	42"		

- 7. Horizontal bars in walls and continuous wall footings shall be bent at corners and intersections in such a way that continuity is provided through the joint. Separate corner bars of the same size and spacing as the horizontal reinforcing may be substituted for the bent portion of the continuous bars.
- 8. Unless noted otherwise, provide 2-#5 bars (one each face) around unframed openings and diagonally at reentrant corners of vertical height offsets in concrete walls. Place bars parallel to the sides of the opening and extend 24 inches beyond corners.
- 9. The Contractor shall prepare detailed working or shop drawings to enable him to fabricate, erect and construct all parts of the work in accordance with the drawings and specifications and shall submit one reproducible copy and one blue line copy to the Structural Engineer for review prior to fabrication. These shop drawings will be reviewed for design concepts only. The Contractor shall be responsible for all dimensions, accuracy, and fit of work.

POST-INSTALLED EXPANSION/ADHESIVE ANCHORS

- 1. Post-installed anchors shall only be used where specified on the Construction Documents. The Contractor shall obtain approval from the Structural Engineer prior to installing the post-installed anchors in place of missing or misplaced cast-in-placed anchors.
- 2. Care shall be taken in placing post-installed anchors to avoid conflicts with existing reinforcing steel.
- 3. Post-installed anchors shall be installed by qualified personnel in accordance with the drawings and
- 4. Post-installed anchors shall be installed by qualified personnel in accordance with the Manufacturer's Printed Installation Instructions (MPII), the drawings and specifications. Installation of adhesive anchors shall be performed by personnel trained to install adhesive anchors. Contractor shall submit installer training cards with anchor package.
- 5. Post-installed anchors shall be HILTI type as manufactured by HILTI Fastening Systems or approved equivalent. Substitution requests must be submitted by the Contractor to the Structural Engineer for review. Provide back-up technical data that demonstrates that the substituted product is capable of achieving the equivalent performance values (minimum) of the specified products using the appropriate design procedure and/or standard(s) as required by the building code.
- 6. Masonry cores receiving post-installed anchors shall be filled with course grout. Grout must comply with IBC Section 2103.12 or IRC Section R609.1.1, as applicable. Alternatively, the grout must have a minimum compressive strength, when tested in accordance with ASTM C1019, equal to its specified strength, but not less than 2,000 psi. Post-installed anchors shall not be installed in a masonry mortar joints.
- 7. The Contractor shall inspect the masonry or concrete surface at each proposed post-installed anchor location prior to installation. If the anchor locations align with mortar joints or the masonry or concrete is honeycombed, cracked or otherwise unsound, the post-installed anchors shall be repositioned so as to be located in sound material and be in accordance with the manufacturer's minimum spacing and edge distance requirements.
- Adhesive anchors shall be subject to the following additional requirements:
- A. Anchors shall meet the requirements of ACI 355.2 (mechanical anchors) and ACI 355.4 (adhesive anchors)
- B. Proof loading of adhesive anchors is not required.
- C. Anchors shall not be installed in concrete cured less than 21-days D. Anchors shall not be installed until the concrete has reached a minimum compressive strength of
- E. Concrete temperature must be greater than 50 °F and less than 80 °F prior to installation of the
- anchors unless otherwise permitted by the MPII.
- F. Anchors shall be installed in holes drilled with the HILTI Hollow Drill Bit (TE-CD (SDS Plus) or TE-YD (SDS Max)) and HILTI VC 20/40 Vacuum (VC 20-U or VC 40-U). Follow the MPII for size and
- depth of holes required. G. The acceptability of certification other than the ACI/CRSI Adhesive Anchor Installer Certification shall be the responsibility of the Structural Engineer
- H. Adhesive anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads shall be continuously inspected during installation by an inspector specially approved for that purpose by the building official. The special inspector shall furnish a report to the licensed design professional and building official that the work covered by the report has been performed and that the materials used and the installation procedures used conform to the approved contract documents and MPII.

NON-SHRINK GROUT

- 1. Grout shall be a high early strength, non-metallic, shrinkage resistant (when tested in accordance with the latest edition of ASTM C827 or CRD-C621), premixed, non-corrosive, non-staining, synthetic fiber reinforced product conforming to the requirements of the latest edition of ASTM C1107 and containing Portland Cement, silica sands, shrinkage compensating agents and fluidity improving compounds.
- 2. Synthetic fibers shall be virgin (non-recycled) nylon or polypropylene fibers conforming to ASTM C1116, Type III. Fibers shall be introduced into the mix at the plant in accordance with the manufacturer's recommendations. The Contractor shall submit the mix design, including the fiber size and quantity, to the Structural Engineer for approval prior to construction. The Contractor shall take adequate measures to manage any difficulty in concrete finishing associated with the use of the fibers. Apply at a rate of 1.5-LBS/CYD.
- 3. Grout compressive strength tests shall be performed in accordance with the latest edition of ASTM C109, with a restraining plate placed over the molds.
- 4. Grout shall be installed in accordance with the manufacturer's instructions.
- 5. Grout shall be placed in a non-sag, flowable state. Grout shall be cured according to manufacturer's

1. Building Code: Indiana Building Code, 2014 Edition (2012 International Building Code, first printing, with Indiana Amendments).

2.	Soil information: Allowable net bearing pressure: Unit weight of soil Active lateral earth pressure coefficient, Ka Coefficient of friction between soil and concrete footing	2000 psf (assumed) 125 pcf (assumed) 0.46 (assumed) 0.30 (assumed)
3.	Concrete: 28 day compressive strength (f'c)	See Schedule

Reinforcing steel (deformed bars of new billet steel): ASTM A615, Grade 60 Stirrup and tie ASTM A706, Grade 60 Weldable (Low-Alloy) ASTM A615, Grade 60 Otherwise

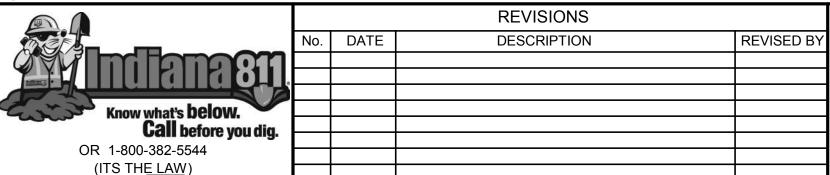
Non-shrink grout: 6,500 psi 28 day compressive strength Wind loads: Basic wind speed (3-second gust) 115 mph Importance factor, lw

Exposure 7. Seismic loads: Seismic importance factor. le Mapped Spectral Response Acceleration at Short Periods, Ss 16.6% g Mapped Spectral Response Acceleration at 1 Second, S1 9.1% g D (assumed) Design Spectral Response Acceleration at Short Periods, Sds 17.7% g 14.6% g Design Spectral Response Acceleration at 1 Second, Sd1 Seismic Design Category

SPECIAL NOTES TO THE OWNER

- 1. Under normal conditions and for conventional structures such as the subject structure, reinforced concrete will develop cracks. The cracks are due to inherent shrinkage of the concrete, creep, ambient temperature variation, and restraining effects of vertical and other structural elements.
- 2. The cracks formed are normally cosmetic. The concrete maintains its serviceability and strength requirements. It is possible that a number of hairline cracks, which would normally spread over a wide area, will integrate into a single crack with a width exceeding 0.01 inch. It is emphasized that although special effort is made to reduce the potential causes and number of such cracks, it is not practical to provide total articulation and thereby achieve complete inhibition of all cracks.
- 3. The majority of these cracks develop within the first three years of service. Cracks which are wider than 0.01 inch may require sealing or epoxy injection.
- 4. The object of the joints provided in the structure is to allow movement. Movements due to creep and shrinkage may be noticeable at joints up to two years after construction, beyond which movements due to variations in temperature will persist.

© 2017 COMMONWEALTH ENGINEERS, INC.





317.818.1912

cesolutionsinc.com





7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177
1419 W. Lloyd Expressway, Suite 401 Evansville, IN 47710 (812) 474-1177
9604 Coldwater Road, Suite 203

(260) 494-3223

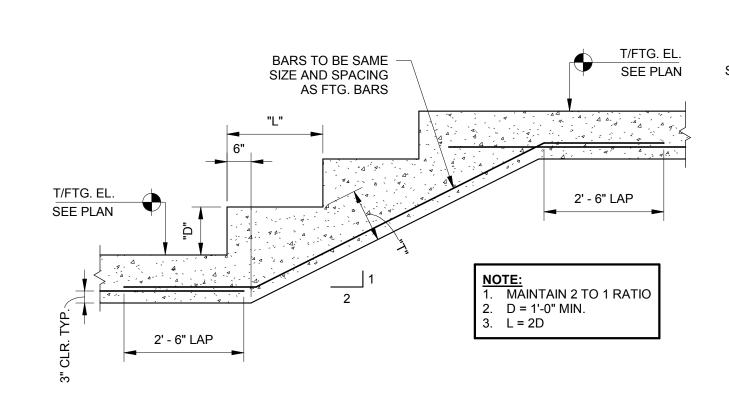
DESIGNED BY: CHECKED BY: DATE: 01/19/18 IOB NO: 16-107 SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE **GENERAL STRUCTURAL NOTES**

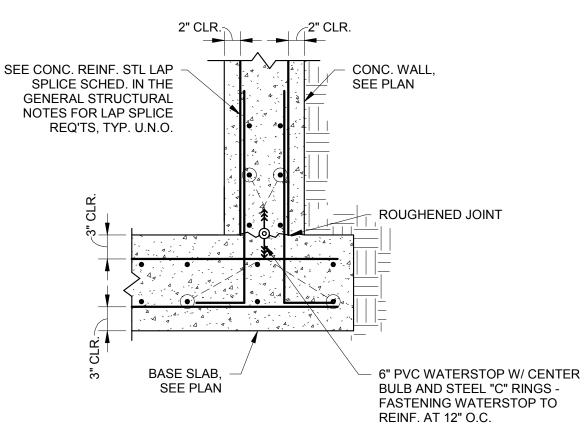
DAM IMPROVEMENTS PROJECT PROJECT NO. E020098

21 OF 26

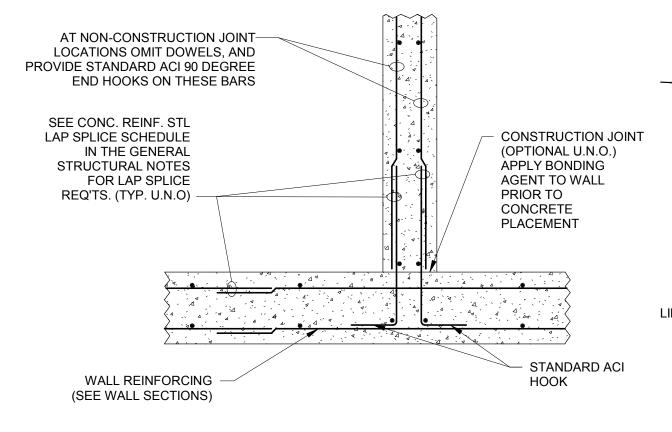
DRAWING NO



9 TYPICAL STEP FOOTING



6 TYPICAL WALL TO BASE SLAB S1-2 1" = 1'-0"



WALL REINFORCING - PLAN VIEW (BOTH FACES)

> 6" PVC WATERSTOP W/ CENTER BULB AND STEEL "C" RINGS -FASTENING WATERSTOP TO

> > SEE NOTE 4

REINF. AT 12" O.C.

TYP. CONC. WALL INTERSECTION S1-2 1/8" = 1'-0"

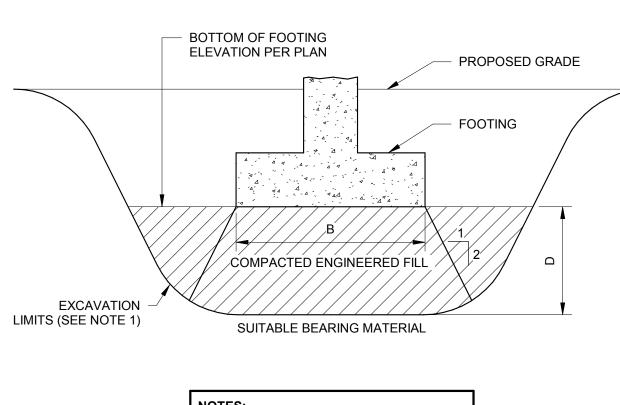
3/4" CHAMFER, EACH FACE

SEE PLANS AND SECTIONS

ROUGHENED JOINT

FOR WALL THICKNESS

AND REINFORCING



NOTES:
1. DEWATER EXCAVATION AS REQUIRED. WIDTH OF EXCAVATION SHALL EQUAL WIDTH OF FOOTING PLUS DEPTH OF UNDERCUT (W=B+D).

1 TYP. FOOTING IN UNDERCUT AREA S1-2 1" = 1'-0"

OPENING

(2) - #5 x 4'-0"

(1 EACH FACE) (TYP.)

(1 EACH FACE) (TYP.)

(2) - #5 (1 EA. FACE)

AT TOP OF OPENING

(2) - #5 (1 EA. FACE)

AT BOTTOM OF OPENING

(1 EACH FACE) (TYP.)

(1 EACH FACE) (TYP.)

(2) - #5 (1 EA. FACE) AT TOP OF OPENING

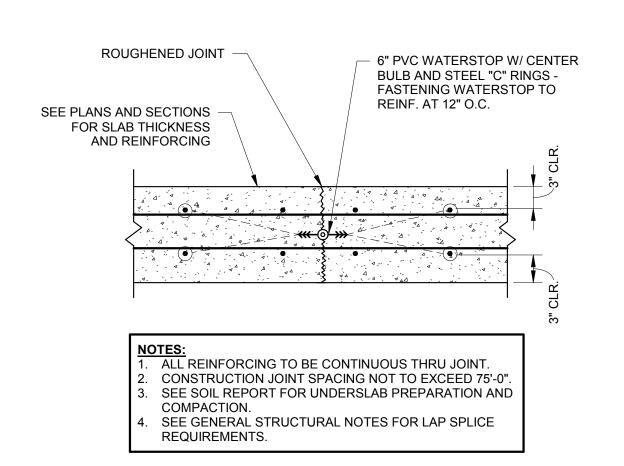
2'-0" TYPICAL OR -PROVIDE A STANDARD

90 DEGREE HOOK

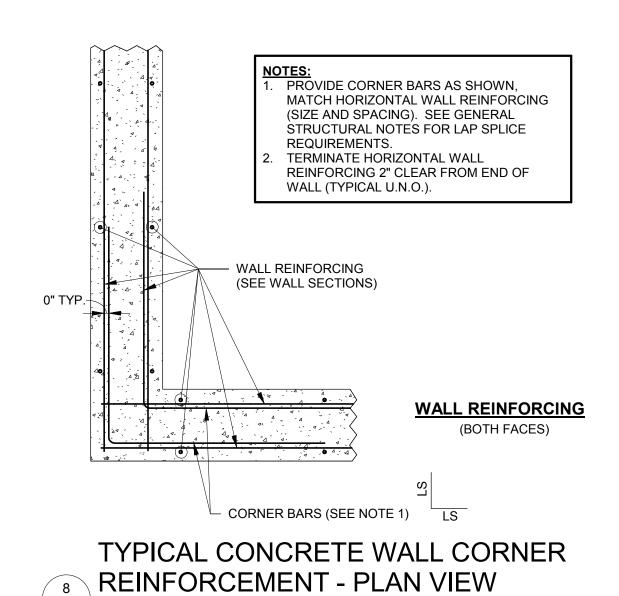
(2) - #5 (1 EA. FACE) ÀT BOTTOM OF OPÉNING

2'-0" TYPICAL OR --PROVIDE A STANDARD

90 DEGREE HOOK



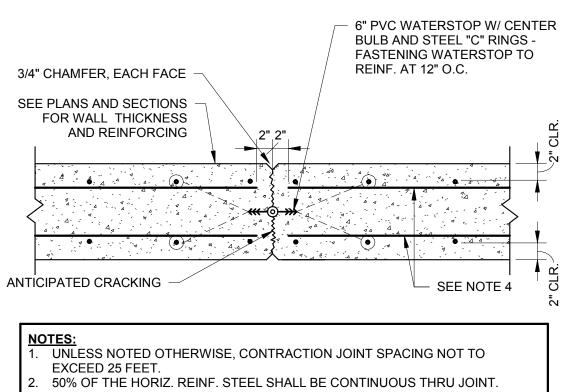
7 TYP. BASE SLAB CONSTRUCTION JT.



NOTES:

1. UNLESS NOTED OTHERWISE, CONSTRUCTION JOINT SPACING NOT TO EXCEED 75 FEET. 50% OF THE HORIZ. REINF. STEEL SHALL BE CONTINUOUS THRU JOINT. HORIZONTAL BARS THAT STOP SHORT OF JOINT SHALL BE FABRICATED ACCORDINGLY AND SHALL NOT BE FIELD CUT. CONTINUOUS AND DISCONTINUOUS HORIZ. BARS SHALL BE ALTERNATED ALONG WALL HEIGHT. CONSTRUCTION JOINTS SHALL NOT OCCUR WITHIN 5'-0" OF A CORNER. SEE GENERAL STRUCTURAL NOTES FOR LAP SPLICE REQUIREMENTS.

4 TYP. WALL CONSTRUCTION JOINT S1-2 1" = 1'-0"



HORIZONTAL BARS THAT STOP SHORT OF JOINT SHALL BE FABRICATED ACCORDINGLY AND SHALL NOT BE FIELD CUT. CONTINUOUS AND DISCONTINUOUS HORIZ. BARS SHALL BE ALTERNATED ALONG WALL HEIGHT CONTRACTION JOINTS SHALL NOT OCCUR WITHIN 5'-0" OF A CORNER. SEE GENERAL STRUCTURAL NOTES FOR LAP SPLICE REQUIREMENTS.

NOTES:

1. WHERE VERTICAL REINFORCING IS INTERRUPTED BY THE OPENING, ONE HALF OF THE INTERRUPTED STEEL SHALL BE ADDED TO EACH SIDE OF THE OPENING. USE FULL LENGTH BARS. THIS DETAIL APPLIES TO ALL OPENINGS IN CONCRETE WALLS UNLESS DETAILED OTHERWISE ON THE PLANS.

OPENING

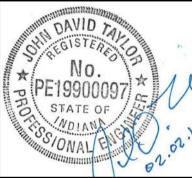
5 TYP. WALL CONTRACTION JOINT \S1-2 \ 1" = 1'-0"

TYP. OPENING IN CONCRETE WALL S1-2 1/8" = 1'-0"

© 2017 COMMONWEALTH ENGINEERS, INC.

	_					
		REVISIONS				
	No.	DATE	DESCRIPTION REV	ISED BY		
Know what's below.						
Call before you dig.						
OR 1-800-382-5544						
(ITS TH <u>E LAW</u>)						

structural engineers | 10 Shoshone Drive Carmel, IN 46032 317.818.1912 cesolutionsinc.com



S1-2 1/8" = 1'-0"



7256 Company Dr.	
Indianapolis, IN 46237 (317) 888-1177	
1419 W. Lloyd Expressway, Suite 401	
Evansville, IN 47710 (812) 474-1177	

Fort Wayne, IN 46825

(260) 494-3223

7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177 419 W. Lloyd Expressway, Suite 401	DRAWN BY: MAH
	DESIGNED BY: JDT
	CHECKED BY: JDT
Evansville, IN 47710 (812) 474-1177	DATE: 01/19/18
9604 Coldwater Road, Suite 203	JOB NO: 16-107

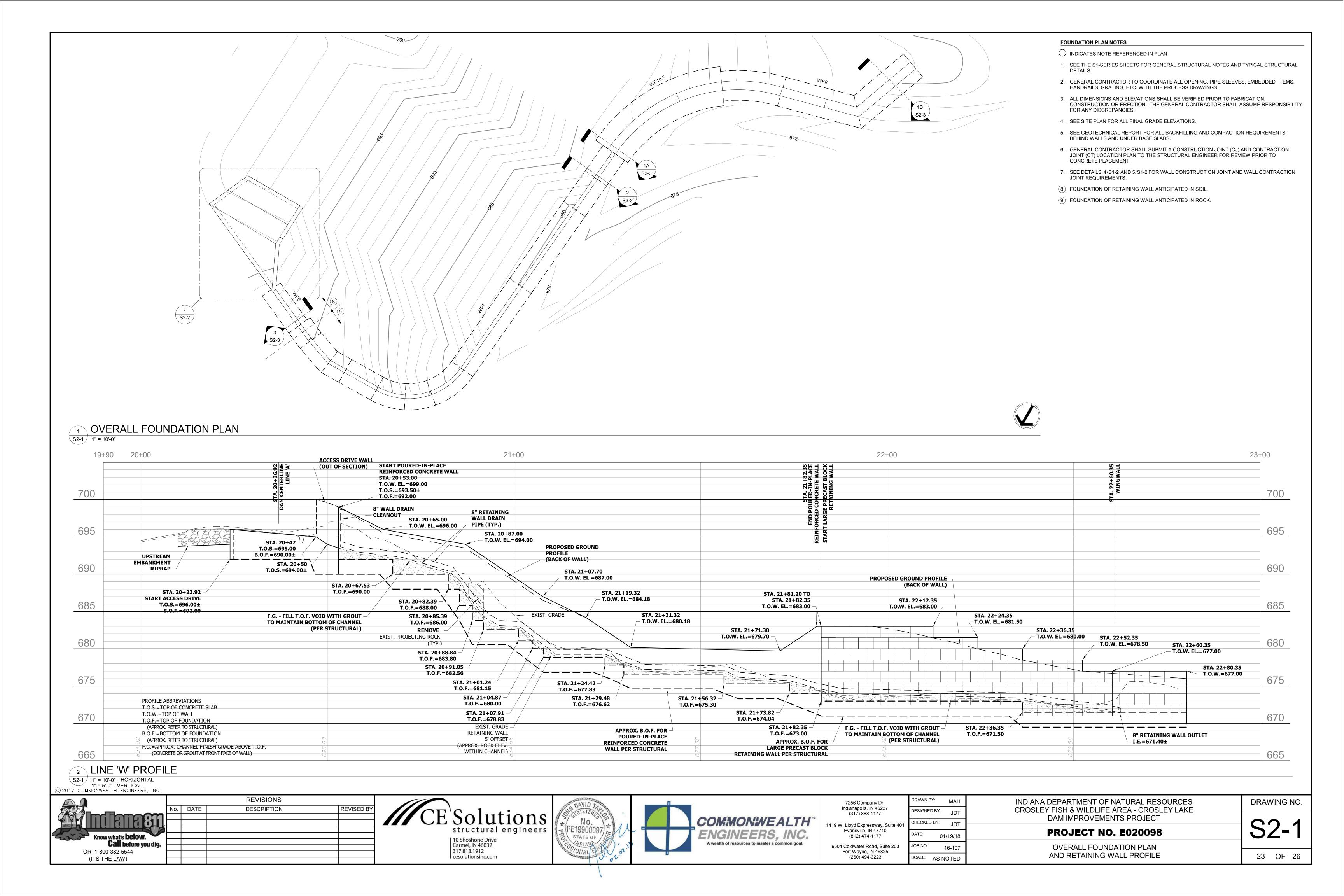
SCALE: AS NOTED

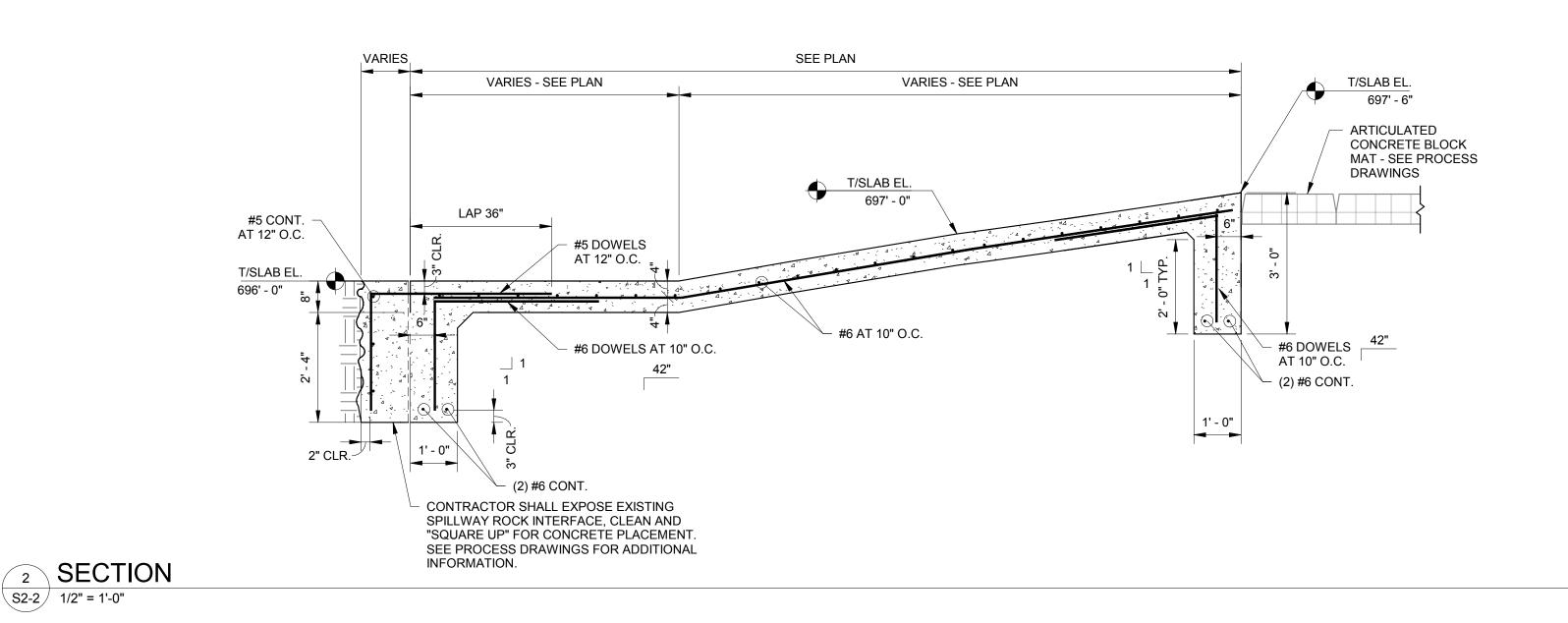
INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE

DAM IMPROVEMENTS PROJECT PROJECT NO. E020098

DRAWING NO.

TYPICAL STRUCTURAL DETAILS - CONCRETE

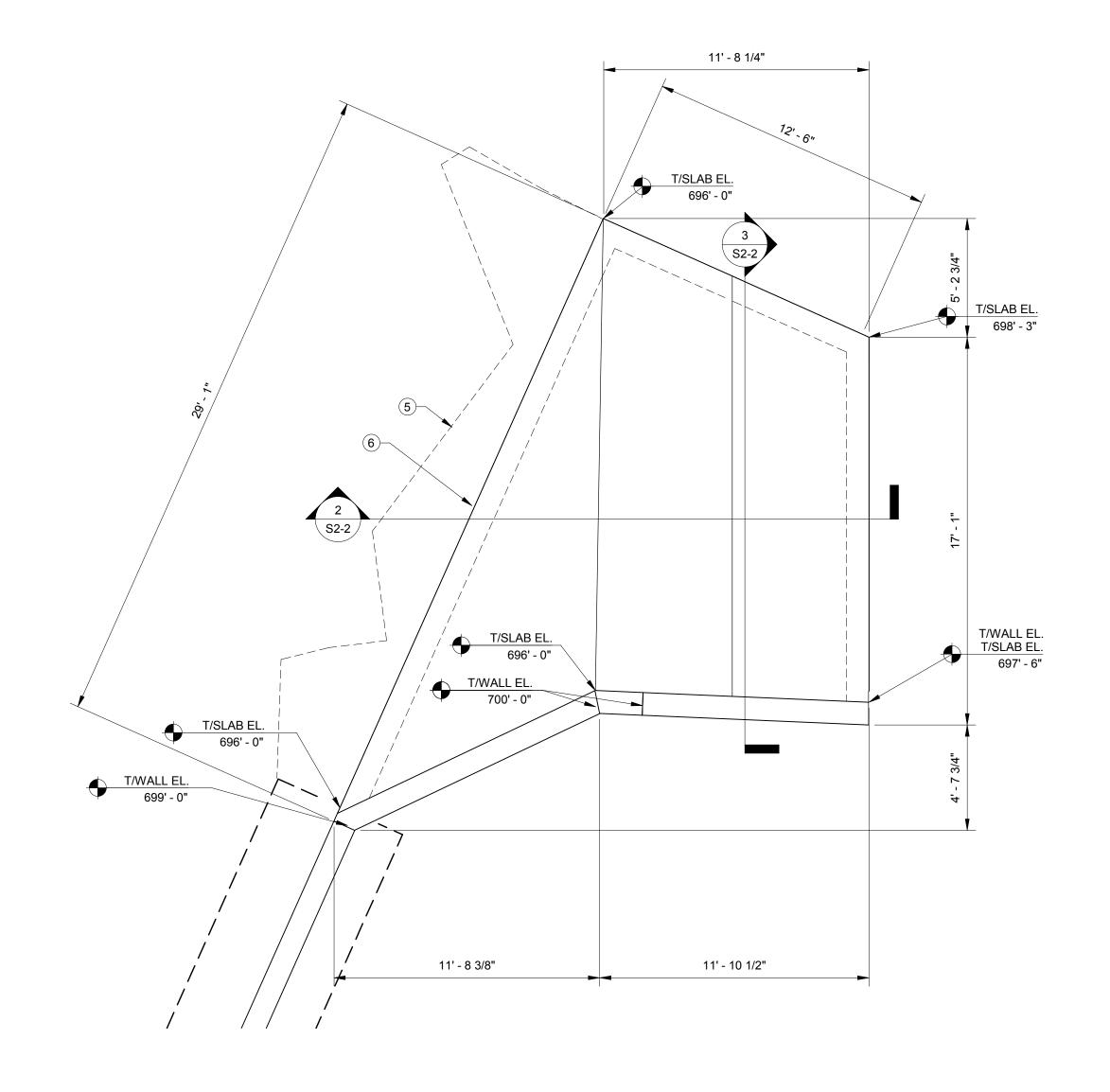




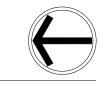
VARIES - #5 AT 12" O.C. - GROUT SEAM, SEE PROCESS DRAWINGS #5 DOWELS AT 12" O.C. DRILL
AND EPOXY INTO CONCRETE
USIN HILTI HIT-HY 200
ADJUSTIVE (6" EMPED)
2" CLR., T/WALL EL RIPRAP ON FILTER VARIES FABRIC, SEE PROCESS DRAWINGS ADHESIVE (6" EMBED) 6" PVC WATERSTOP T/SLAB EL. VARIES #6 DOWELS AT 10" O.C. – #6 AT 10" O.C. #6 DOWELS AT 10" O.C. - ARTICULATED CONCRETE BLOCK 1' - 0" (2) #6 CONT. MAT - SEE PROCESS - FILTER FABRIC, SEE PROCESS DRAWINGS

3 SECTION S2-2 1/2" = 1'-0"

S2-2 1/2" = 1'-0"



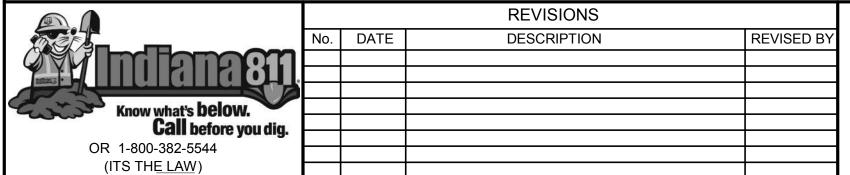
ACCESS DRIVE - ENLARGED FOUNDATION PLAN



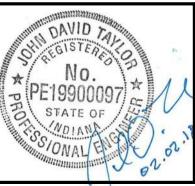
FOUNDATION PLAN NOTES

- O INDICATES NOTE REFERENCED IN PLAN
- 1. SEE THE S1-SERIES SHEETS FOR GENERAL STRUCTURAL NOTES AND TYPICAL STRUCTURAL
- 2. ALL DIMENSIONS AND ELEVATIONS SHALL BE VERIFIED PRIOR TO FABRICATION, CONSTRUCTION OR ERECTION. THE GENERAL CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR ANY DISCREPANCIES.
- 3. SEE SITE PLAN FOR ALL FINAL GRADE ELEVATIONS.
- 4. SEE GEOTECHNICAL REPORT FOR ALL BACKFILLING AND COMPACTION REQUIREMENTS BEHIND WALLS AND UNDER BASE SLABS.
- (5.) APPROXIMATE EXISTING ROCK EDGE. SEE PROCESS DRAWINGS FOR ADDITIONAL
- (6.) GENERAL CONTRACTOR FIELD VERIFY TOP OF CONCRETE SLAB ELEVATIONS PRIOR TO CONCRETE PLACEMENT. TOP OF CONCRETE SLAB ELEVATION TO MATCH EXISTING TOP OF ROCK ELEVATIONS.

© 2017	COMMONWEALTH	ENGINEERS.	INC.









7256 Company Dr. Indianapolis, IN 46237 (317) 888-1177	
1419 W. Lloyd Expressway, Suite 401 Evansville, IN 47710	
(812) 474-1177 9604 Coldwater Road, Suite 203 Fort Wayne, IN 46825	Ŀ

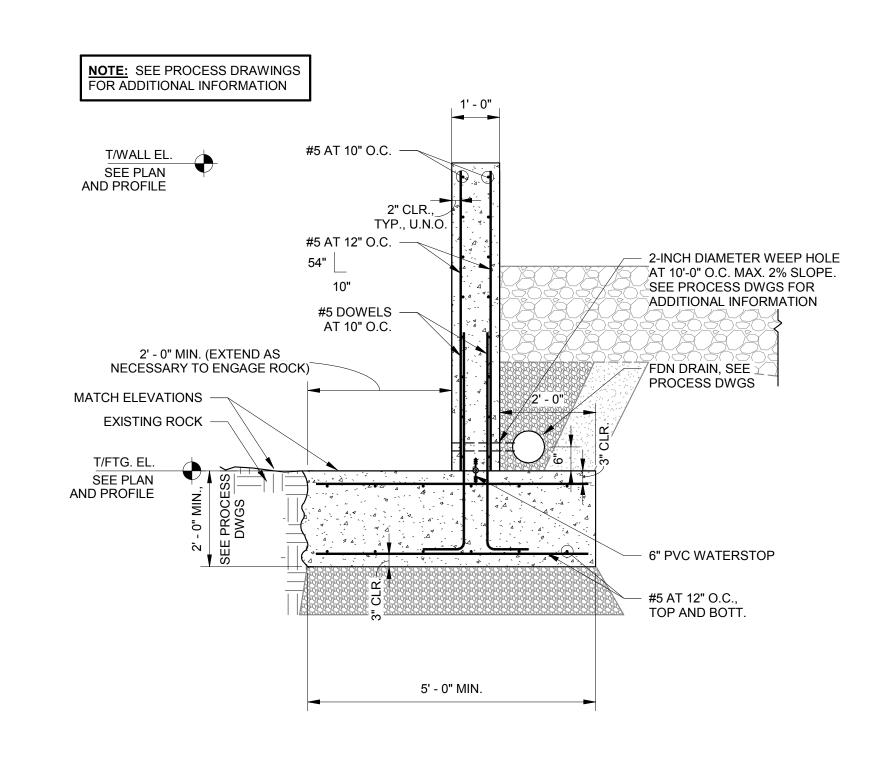
(260) 494-3223

DRAWN BY:	MAH	
DESIGNED B	^{BY:} JDT	
CHECKED B	Y: JDT	
DATE:	01/19/18	
JOB NO:	16-107	1

SCALE: AS NOTED

INDIANA DEPARTMENT OF NATURAL RESOURCES CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT	
PROJECT NO. E020098	
ACCESS DRIVE - ENLARGED FOUNDATION PLAN, SECTIONS, AND DETAILS	

DRAWING NO.



NOTE: SEE PROCESS DRAWINGS FOR ADDITIONAL INFORMATION T/WALL EL. SEE PLAN AND PROFILE #5 AT 10" O.C. TYP., U.N.O 2-INCH DIAMETER WEEP HOLE AT 10'-0" O.C. MAX. 2% SLOPE. SEE PROCESS DWGS FOR ADDITIONAL INFORMATION #5 DOWELS -FDN DRAIN, SEE PROCESS DWGS NON-SHRINK GROUT SEE PROCESS DWGS T/FTG. EL. SEE PLAN AND PROFILE 6" PVC WATERSTOP - EXISTING ROCK - #5 AT 12" O.C., EXCAVATE ROCK AND -"SQUARE UP" ROCK TOP AND BOTT. INTERFACE AND SLOPE CONCRETE AT MIN. 2.0% SLOPE

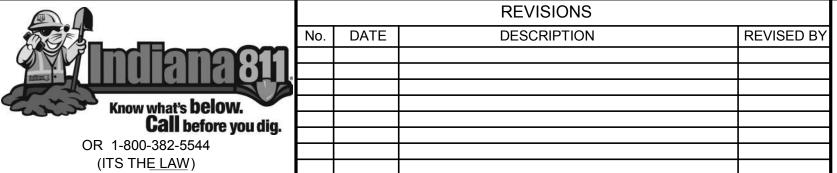
NOTE: SEE PROCESS DRAWINGS FOR ADDITIONAL INFORMATION PRECAST BLOCK WALL, SEE PROCESS DWGS. 2-INCH DIAMETER WEEP HOLE AT 10'-0" O.C. MAX. 2% SLOPE. SEE PROCESS DWGS FOR ADDITIONAL INFORMATION FDN DRAIN, SEE PROCESS DWGS #5 CONT. NON-SHRINK GROUT SEE PROCESS DWGS T/FTG. EL. SEE PLAN AND PROFILE #5 DOWELS -AT 12" O.C. #5 AT 12" O.C.,
 TOP AND BOTT. 2' - 9" AT 1A 2' - 9" AT 1A 1' - 6" AT 1B 1' - 6" AT 1B 10' - 6" AT 1A 8' - 0" AT 1B

FOUNDATION WALL IN SOIL S2-3 1/2" = 1'-0"

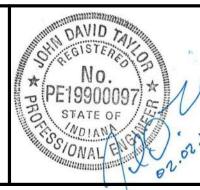
FOUNDATION WALL IN ROCK S2-3 1/2" = 1'-0"

PRECAST BLOCK RETAINING WALL S2-3 1/2" = 1'-0"

© 2017 COMMONWEALTH ENGINEERS, INC.







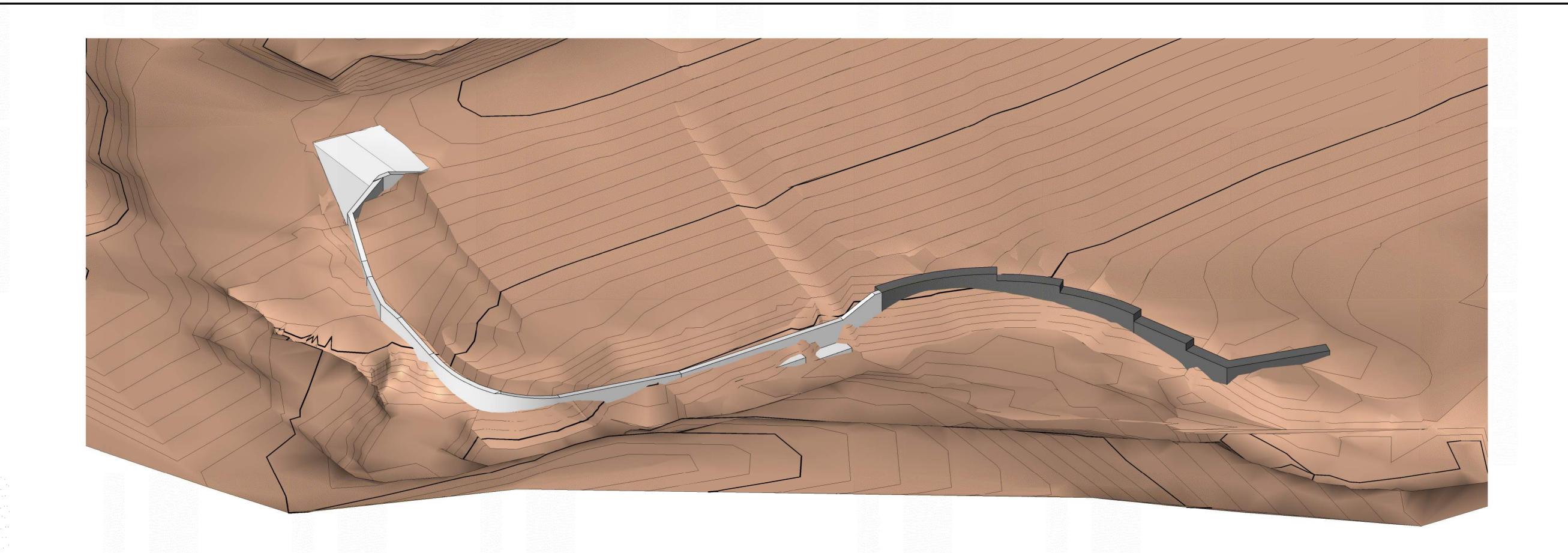


	_
7256 Company Dr.	
Indianapolis, IN 46237 (317) 888-1177	D
1419 W. Lloyd Expressway, Suite 401	С
Evansville, IN 47710 (812) 474-1177	D
9604 Coldwater Road, Suite 203 Fort Wayne, IN 46825	J
(260) 494-3223	s

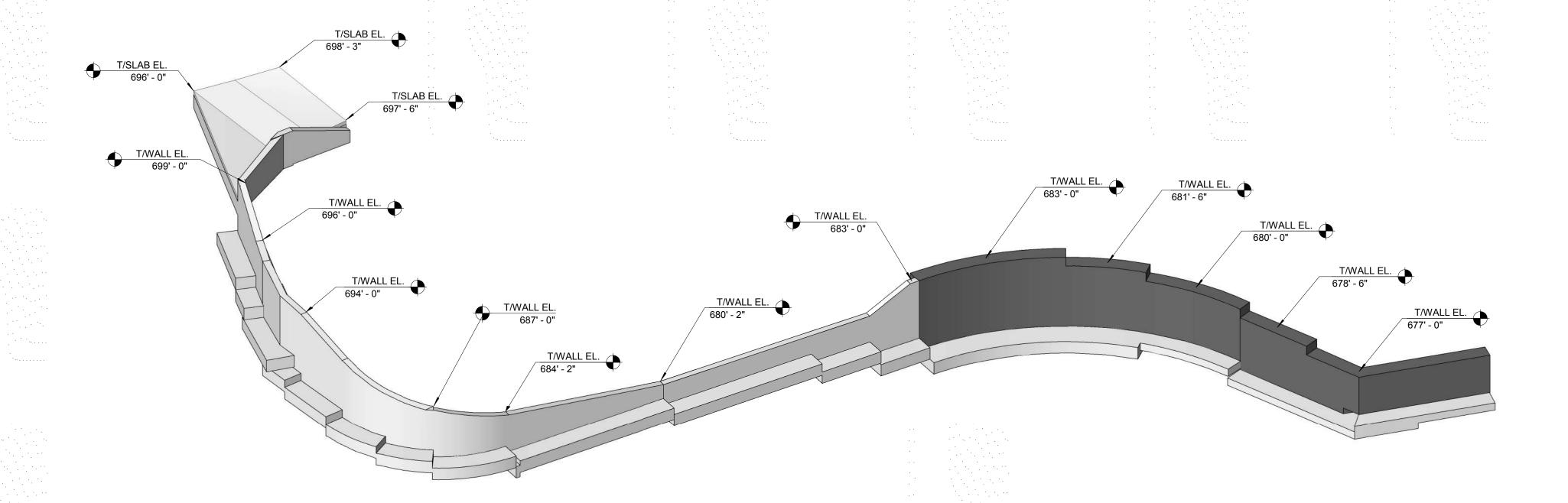
SCALE: AS NOTED

PRAWN BY: MAH	INDIANA DEPARTMENT OF NATURAL RESOURCES
DESIGNED BY: JDT	CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE DAM IMPROVEMENTS PROJECT
CHECKED BY:	DAM IMPROVEMENTS PROJECT
HECKED BY: JDT	
OATE: 01/19/18	PROJECT NO. E020098
OB NO: 16-107	RETAINING WALL
SCALE: AS NOTED	SECTIONS AND DETAILS

DRAWING NO.
S2-3



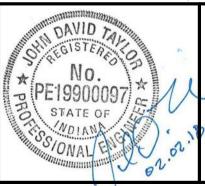
1 ISOMETRIC



2 ISOMETRIC S2-4

© 2017 COMMONWEALTH ENGINEERS, INC.		1. - 1.	• •			
		. •	REVISION	IS		
	No.	DATE	DESCRIPTION	ON	REVISED BY	
	M					
	8					
Know what's below.						
Call before you dig.						
OR 1-800-382-5544		·	<u> </u>			
Market and the second of the s			•	A Commence of the Commence of		
(ITS TH <u>E LAW</u>)	٠.,			1		Ť.





		COMMON!WEALTH™
, •	J	ENGINEERS, INC. A wealth of resources to master a common goal.

	_
7256 Company Dr.	
Indianapolis, IN 46237 (317) 888-1177	
1419 W. Lloyd Expressway, Suite 401	C
Evansville, IN 47710 (812) 474-1177	<u>-</u> [
9604 Coldwater Road, Suite 203 Fort Wayne, IN 46825	·J
(260) 494-3223	S

	. 4		_
	.DRAWN BY:	MAH	
· · · · · · · · · · · · · · · · · · ·	DESIGNED E	^{3Y:} JDT	
)1	CHECKED BY: JDT		
· · · ·	DATE:	01/19/18	
 	JOB NO:	16-107	
	SCALE: A	S NOTED	

PROJECT NO. E020098
DAM IMPROVEMENTS PROJECT
CROSLEY FISH & WILDLIFE AREA - CROSLEY LAKE
INDIANA DEPARTMENT OF NATURAL RESOURCES

IMPROVEMENTS PROJECT	S2-4	
DJECT NO. E020098		52-4
ISOMETRIC VIEWS	·	

26 OF 26

DRAWING NO.