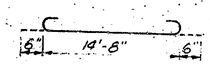
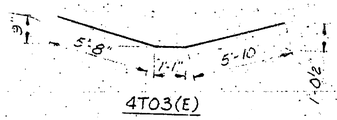
BAR BENDING DIAGRAMS

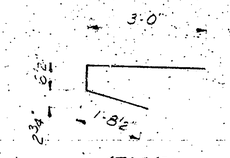
NOTE: Number preceding letter denotes bar size.
All dimensions are out to out.
All bar bending diagrams shall be in accordance with A.C.I. Standards.



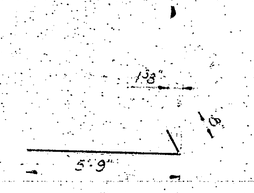
4T02



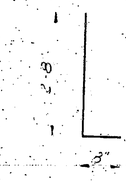
4T03(E)



4T05(E)

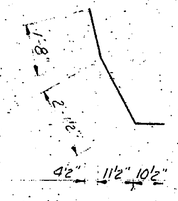


4T08(E)

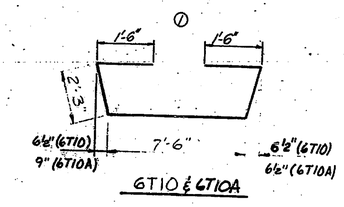


4T06

5T06(E)

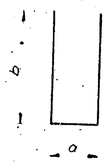


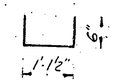
5T07(E)



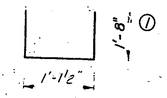
6T10 & 6T10A

Bar	a	b
4T07	6"	3'-3"
5T01	1'-1½"	7'-4"
5T03(E)	1'-1½"	3'-5"
① 5T08	2'-8"	1'-8"
5T09	1'-6"	3'-0"
①		
①		
△ 6T01	2'-8"	1'-10"
9T01	2'-8"	7'-4"
9T02(E)	2'-8"	5'-4"
10T01	2'-8"	7'-4"
10T02(E)	2'-8"	5'-9"
6T05	1'-1½"	7'-4"
6T06(E)	1'-1½"	4'-0"





3S01



5T10

① 4T07, 5T01, 5T03(E), 5T08, 5T09, 6T01, 9T01, 9T02(E), 10T01, 10T02(E), 6T05, 6T06(E)

△ NOTE #: See Sheet No. 3C8 for bar bending diagrams.

BILL OF MATERIAL

	Bar	No	Length	Bent	Str
	3S01	730	2'-2"	•	
	4T01	537	76'-10"		•
	4T02	437	5'-8"	•	
	4T03(E)	670	3'-8"	•	
	4T04(E)	476	88'-2"		•
	4T05(E)	356	5'-3"	•	
△	4T06	734	3'-4"	•	
	4T07	58	3'-[illegible]	•	
	4T08(E)	504	6'-5"	•	
	4T09(E)	[illegible]	2'-8"		•
*	4T10(E)	96	3'-[illegible]	•	
	4L01(E)	[illegible]	12'-6"		•
△	4L04	232	62'-0"		•
	4L04(E)	[illegible]	26'-[illegible]		•
	4L05	[illegible]	37'-[illegible]		•
	4L06	[illegible]	50'-3"		•
△	4L07	[illegible]	40'-10"		•
	4L08	[illegible]	4'-[illegible]		•
	4L09	[illegible]	[illegible]'-8"		•
	4L10	[illegible]	[illegible]		•
	4L11	[illegible]	3'-[illegible]		•
	4L12	[illegible]	1'-0"		•
	4L13	[illegible]	[illegible]'-4"		•
	4L14	[illegible]	26'-3"		•
	4L15(E)	[illegible]	46'-3"		•
△					
	4L17	[illegible]	19'-0"		•
	5T01	[illegible]	5'-3"	•	
	5T03(E)	[illegible]	3'-0"	•	
	5T04(E)	[illegible]	10'-8"		•
	5T05(E)	[illegible]	8'-0"		•
	5T06(E)	[illegible]	4'-3"	•	
	5T07(E)	[illegible]	4'-8"	•	
①	5T08	612	6'-0"	•	
	5T09	[illegible]	7'-6"	•	
①	5T10	[illegible]	4'-6"	•	
	6L01		17'-0"		•
△	6T01	92	6'-4"	•	
①					
①					
	6T04	[illegible]	[illegible]		•
	6T05	[illegible]	15'-10"	•	
	6T06(E)	[illegible]	[illegible]	•	
②	6T10	28	15'-0"	•	
①	6T11	114	7'-4"		•
△②	6T10A	42	15'-0"	•	
①	9T01	162	17'-4"	•	
①	9T02(E)	162	13'-4"	•	
①	9T03	36	7'-4"		•
	10T01	162	17'-4"	•	
	10T02(E)	36	14'-4"	•	

NOTES: All reinforcing shall conform to the requirements of ASTM Grade 60.

Minimum compressive strength of Superstructure Conc. shall be 5,500 psi

ESTIMATED QUANTITIES

Item	Unit	Total
Superstructure Conc.	Cu Yd	976.9
△②① Reinforcing Steel	Lb	68,358.32
① Reinforcing Steel (Epoxy Coated)	Lb	74,527.19

Note: Quantity of Reinforcing Steel changed from 11,446.51 Lbs. to 69,728.16 Lbs. on 11/9/82.
Additional quantity of reinforcement steel due to revision ① = 4433.82 lb.
Additional quantity of reinforcement steel (Epoxy Coated) due to revision ① = 1493.93 lb.
Quantity of reinforcement steel due to revision ② changed from 69,728.16 lbs. to 70,043.58 lbs.

DESIGNED M.M. C'K'D. M.M.
DRAWN R.E.T. C'K'D. M.M.
TRACED C'K'D.

APPROVED FOR CONSTRUCTION
F. L. Derthick
F. L. DERTHICK
Engineer of Bridge Plans
MAR 23 1983

△ Revised March, 1983
① Revised 11/9/82
② Revised 12/15/82

Figg and Muller Engineers Inc.
228 North Cedar, Suite
Milwaukee, Florida 32301

BAR BENDING DIAGRAMS & BILL OF MATERIAL

INDIANA STATE HIGHWAY COMMISSION
LAKE COUNTY - RAMP:- B SECTION:-
SCALE:- N.T.S. DATE:- July 29, 1981
SUBMITTED FOR APPROVAL Eugene C. Figg
DRAWING - OF SHEET:- 305 OF
PROJECT:- MM 220-1(8)
CONTRACT NO. B-12752
BRIDGE FILE:- 912-45-2546 RWN

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