



**SECTION 5 - EAST END BRIDGE
OVER OHIO RIVER**

KYTC ITEM NO. 5-745.00

**SECTION 4 SCOUR ANALYSIS
(Supplement to Section 5 Hydraulics Report)**

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**Prepared for:
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1.0 INTRODUCTION

This report is a supplement to the Section 5 East End Bridge - Hydraulic and Scour Analysis Report, dated September 20, 2011. Its purpose is to document the scour analysis for the **Section 4** approach span piers on the Kentucky Approach. These piers lie between stations 167+81.96 (End Bent 1) and 187+50.96 (Common Pier).

2.0 DATA COLLECTION

2.1 General

The hydraulic data necessary for the scour analysis was extracted from the 2-D SMS/FESWMS model (documented in the Hydraulic and Scour Report for **Section 5**). The representative pier configuration was obtained from Lochner for **Section 4** (see **Appendix A**), and the geotechnical boring data for **Section 4** was provided by Stantec (see **Appendix B**).

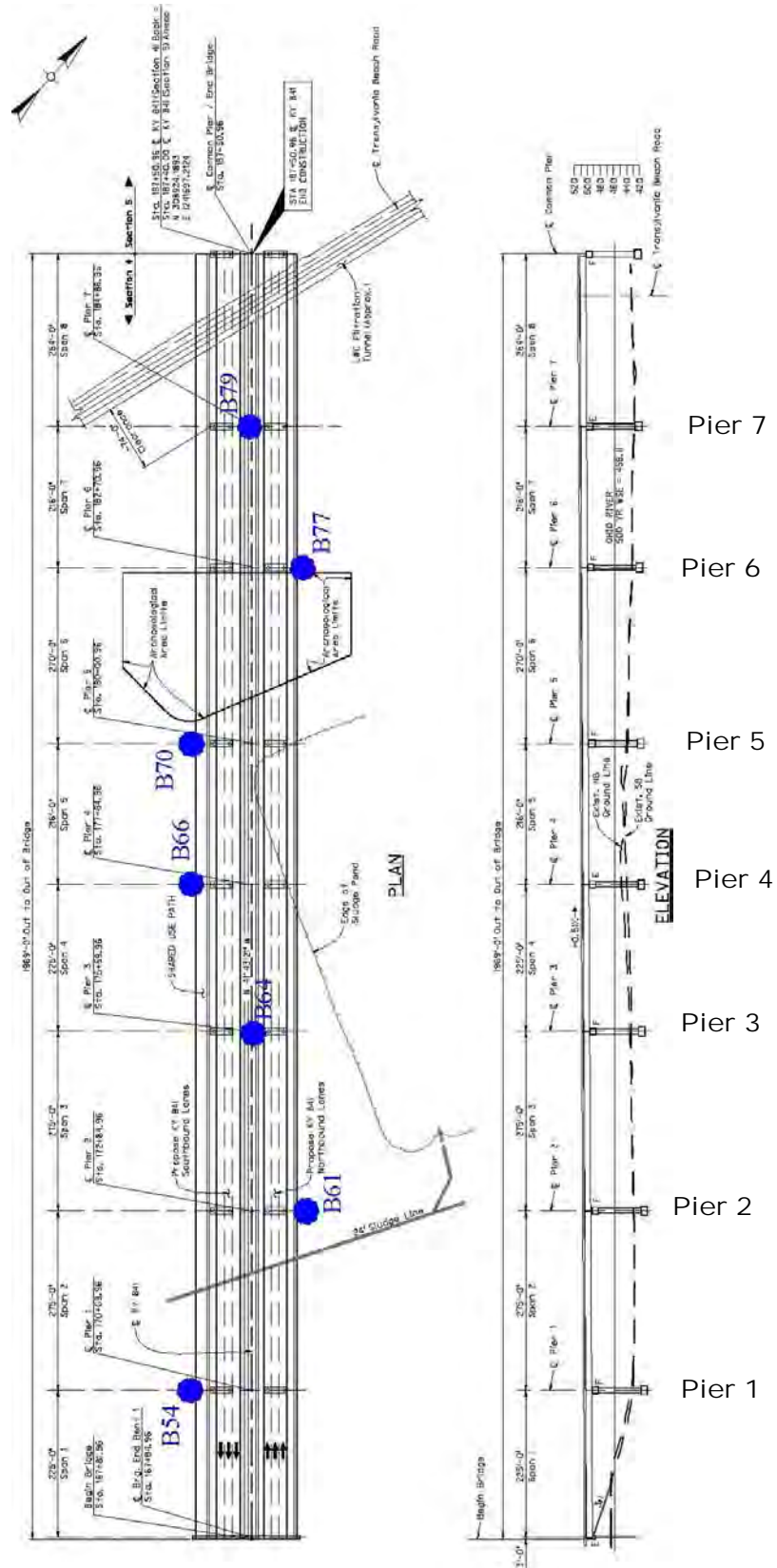
2.2 Soil Particle Sizes

Data from draft geotechnical borings was used in the scour analysis. The bed soil consists mainly of sands and clays with d50 ranging from 0.0039 mm to 0.13 mm and d95 from 0.049 mm to 12.0 mm, as shown in **Table 1**. The general soil boring plan is shown in **Figure 1**. This drawing was prepared from the Stantec boring log location information for this study and is for illustrative purposes only. Excerpts from the boring logs and lab testing pertinent to the bridge crossing are presented in **Appendix B**.

Table 1: Grain Size Summaries at Proposed Bridge Crossing

Structure Element	Boring Number	Station	Offset	Sample Depth (ft)	d50 (mm)	d95 (mm)
Pier 1	B54	170+10	86' LT	1.2 / 3.5	0.13	12.0
Pier 2	B61	172+85	69' RT	3	0.0053	0.26
Pier 3	B64	175+60	6' RT	3	0.0065	0.079
Pier 4	B66	177+85	86' LT	4.6	0.041	0.31
Pier 5	B70	180+01	86' LT	3	0.007	0.2
Pier 6	B77	182+71	69' RT	3.6 / 5.0	0.069	0.32
Pier 7	B79	184+87	6' LT	3	0.0039	0.049

Figure 1: General Soil Boring Locations



The Section 4 piers (Kentucky approach) in the overbank area are typically dry at the normal pool stage and begin to experience flow before the 10-yr (10%) storm discharge. For purposes of scour analysis, **Table 2** summarizes maximum velocities within the vicinity of each of the approach piers for the 100-yr (1%) and the 500-yr (0.2%) storms.

Table 2: Summary of Maximum Velocities and Corresponding Flow Depths

Pier No.	Node No.	100-Year Event		500-Year Event	
		Velocity (ft/s)	Depth (ft)	Velocity (ft/s)	Depth (ft)
Pier 1	4605	0.417	7.995	1.381	13.159
Pier 2	4113	1.076	24.685	1.972	29.852
Pier 3	4116	2.142	15.981	3.025	21.128
Pier 4	4120	3.563	6.601	4.367	11.706
Pier 5	4617	2.717	13.504	3.481	18.592
Pier 6	4626	3.005	19.996	3.717	25.160
Pier 7	5412	2.992	21.043	3.730	26.200

Figure 2 graphically depicts the calculated flow regimes of each pier, identified as 1-7 beginning at the bank on the Kentucky side of the Ohio River. The SMS/FESWMS output can be found in the SECTION 5 - Hydraulic and Scour Analysis Report, dated September 20, 2011. The hydraulic flow parameters are summarized in **Table 2**, and with the geotechnical particle sizes for each pier in **Appendix B, Pier Drag Report** chart.

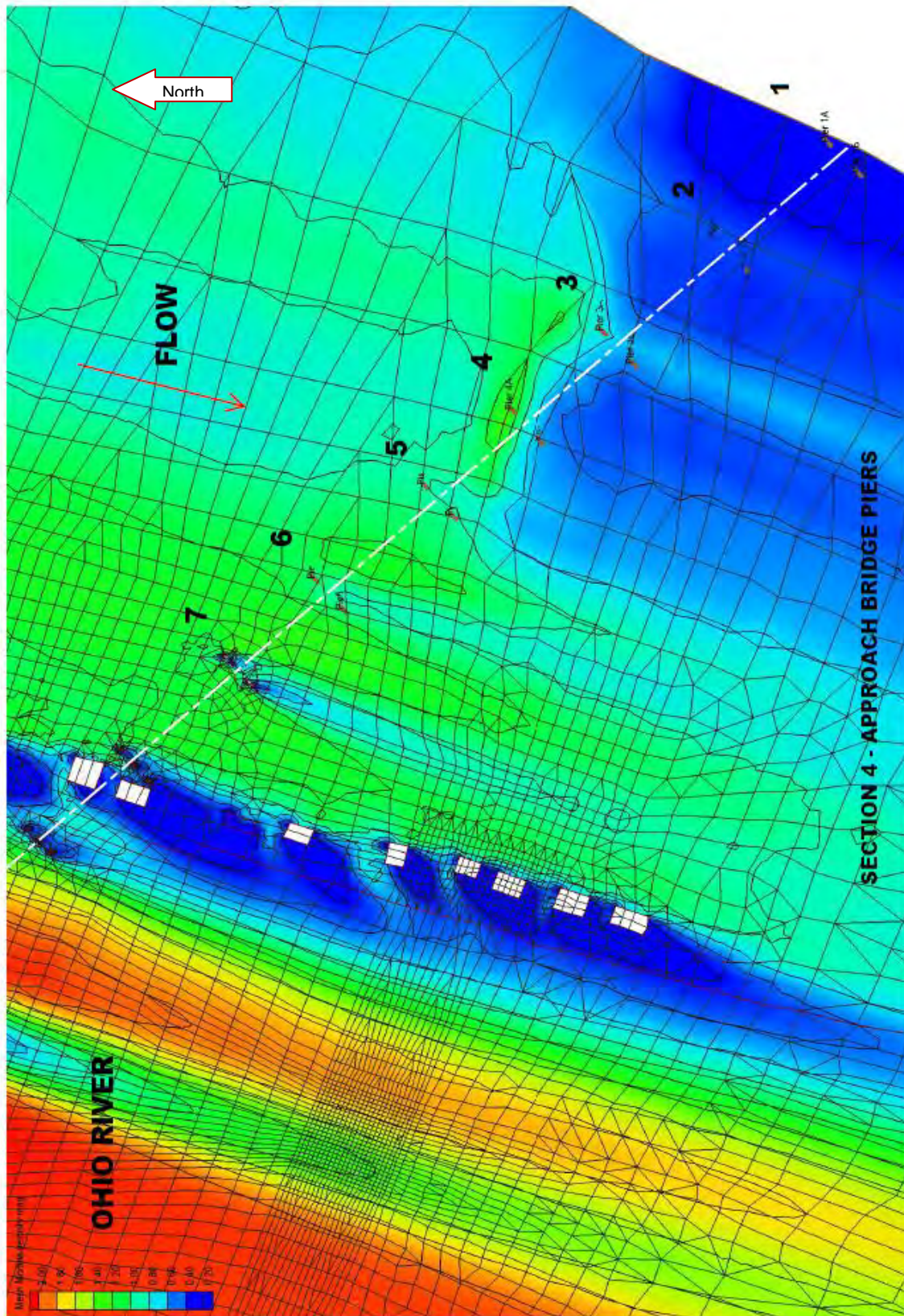


Figure 2: Velocity Magnitude, 1% Storm Event (451.7 ft NAVD88)

3.0 SCOUR ANALYSIS

3.1 Methodology

The scour analysis was performed in accordance with the methodology outlined in the Federal Highway Administration (FHWA) Hydraulic Engineering Circular (HEC) No. 18. These methods were used to develop predictions of long-term, contraction, and local scour at piers and abutments. In accordance with general practice, the 100-yr (1%) storm was used as the scour analysis design event.

For this supplemental report, scour analysis was only performed for the Section 4 (Kentucky Approach) piers.

Hydraulic parameters necessary to complete the scour analysis were taken from the SMS/FESWMS model results. The FESWMS node numbers used to perform the scour analysis are shown in **Appendix C** in the Total Scour Inputs (from FESWMS) table.

3.2 Theoretical Predicted Scour

Total scour depths shown in **Tables 3 and 4** are composed of three scour components: long-term, contraction, and local (which is composed of the pier scour). The total scour depths for the 100-year event are shown graphically in the scour plot in **Figure 3**. The detailed scour calculations are provided in **Appendix C**.

3.2.1 Long-term Scour Component

The USACE Louisville, Kentucky District was contacted to obtain available historical data documenting changes to streambed geometry and elevations over time. Both the Water Management Section of the Engineering Division and the Navigation Information Section of the Operations Division were consulted. Both offices indicated that historical streambed data is only available for a few specific sections of the Ohio River in the vicinity of the locks and dams where scour problems have occurred, and that none of those data sets would be applicable to the river reach requested.

A review of area mapping indicates that the nearest existing bridge crossing is the I-65 JFK Memorial Bridge at River Station 602.9. Due to the proximity of the I-65 bridge to the McAlpine dam and any influences caused by the dam structure, and since this crossing is 7.4 miles downstream, historical streambed surveys and inspection reports for that structure cannot be relied upon to provide a conclusion regarding streambed variations over time at the proposed bridge location. The nearest highway crossing upstream from the study site is in Trimble County which is much too far upstream to be used for guidance in making assumptions.

A level-three (HEC-20) analysis involving sediment routing within the contributing watersheds can sometimes be useful to predict sediment transport, but establishing a mathematical model would require a substantial historical data set as well as substantial analysis time. Lack of data coupled with the knowledge that the downstream control in place at the McAlpine Lock maintains the normal pool condition of the Ohio River a steady, slow velocity (less than 2.0 fps) led to the conclusion that aggradation or degradation of the streambed is likely negligible in comparison to other scour components, and that not enough data is available to develop a prediction based on mathematical analysis. For these reasons, the long-term scour component of the overall scour estimate was assumed to be zero feet.

Table 3: Theoretical Scour Depths (ft) for the 100-yr Storm by Pier

	Pier						
	1	2	3	4	5	6	7
Boring Number	B54	B61	B64	B66	B70	B77	B79
Modeled Bed Elevations***	430.5	427.9	432.3	434.3	437.6	434	430.2
Bed Rock Elevation***	330.5	331.4	333.8	333.3	334.3	335.8	334.7
Competent Rock Elevation***	329.7	331.4	331.1	333.1	334.3	335.5	334.5
Long-term (ft)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Contraction (ft)	0.4	0.4	0.4	0.4	0.4	0.4	0.4
Local (ft)	2.5	10.7	14.5	15.9	15.8	17.3	11.7
Total Scour (ft)	3.0	11.1	14.9	16.3	16.3	17.7	12.1
Total Scour Elevation	427.5	416.8	417.4	418.0	421.3	416.3	418.1

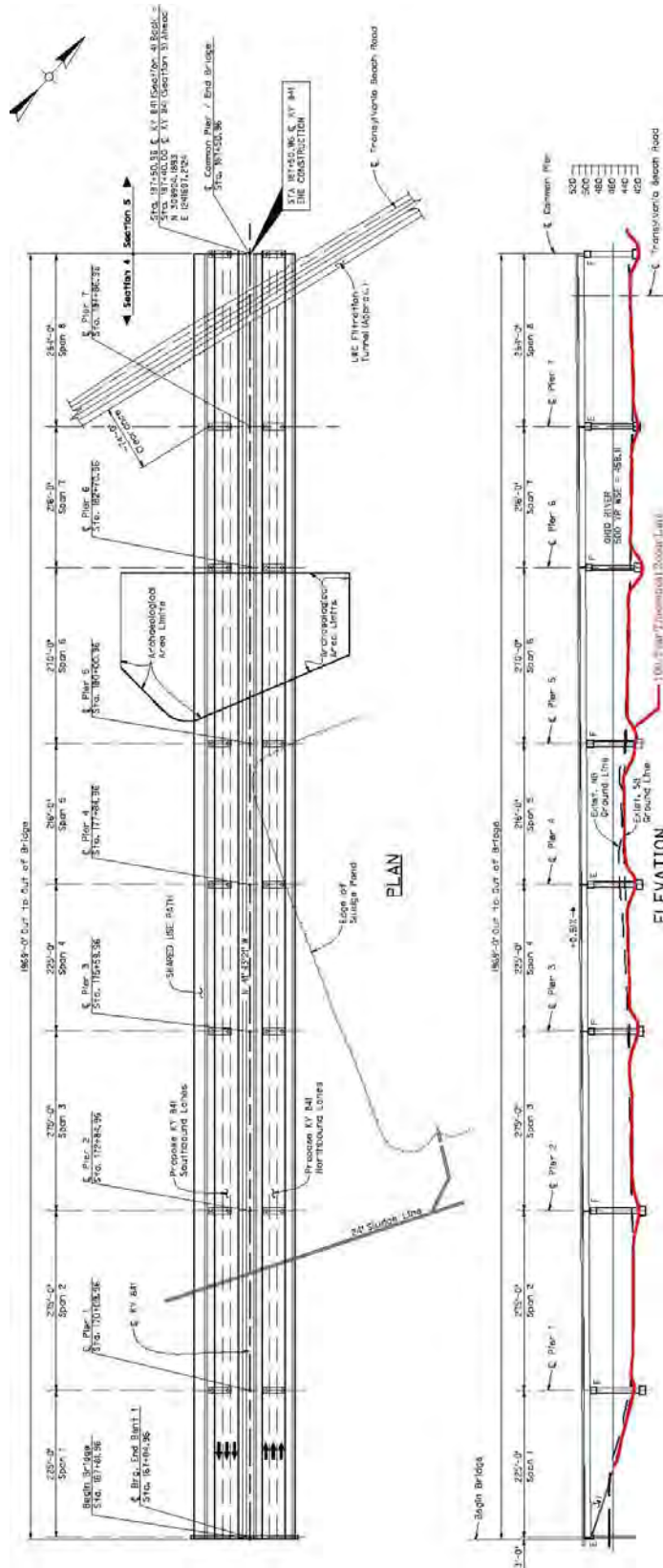
*** from geotechnical data

Table 4: Theoretical Scour Depths (ft) for the 500-yr Storm by Pier

	Pier						
	1	2	3	4	5	6	7
Boring Number	B54	B61	B64	B66	B70	B77	B79
Modeled Bed Elevations***	430.5	427.9	432.3	434.3	437.6	434	430.2
Bed Rock Elevation***	330.5	331.4	333.8	333.3	334.3	335.8	334.7
Competent Rock Elevation***	329.7	331.4	331.1	333.1	334.3	335.5	334.5
Long-term (ft)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Contraction (ft)	1.9	1.9	1.9	1.9	1.9	1.9	1.9
Local (ft)	11.0	14.5	17.4	18.8	18.4	19.6	13.3
Total Scour (ft)	12.9	16.3	19.2	20.6	20.3	21.4	15.1
Total Scour Elevation	417.6	411.6	413.1	413.7	417.3	412.6	415.1

*** from geotechnical data

Figure 3: Theoretical Scour for the 100-year Storm



3.2.2 Contraction Scour Component

Contraction scour was computed by spreadsheet using Laursen's equations as recommended in HEC-18. Initially, critical velocity was completed using hydraulic properties for the bridge and approach sections from HEC-RAS output to determine whether the live-bed or clear-water scour equation was appropriate. The live-bed scour equation was determined to be applicable for the left overbank (Piers 1 - 7). The proposed bridge configuration spans the entire 100-yr floodplain and therefore does not contain abutment fills, so local abutment scour was not considered separately.

3.2.3 Local Scour Component

For the East End Bridge, local pier scour is clearly the controlling scour mechanism. Piers 1 - 7 each consist of two elliptical columns founded on drilled shafts to bedrock. For this reason, these piers were modeled as "simple" piers under the HEC-18 methodology.

The local pier scour equation recommended by HEC-18 contains correction factors for pier nose shape, angle of attack of flow, bed condition and armoring. The angle of attack for the piers was reported as resultant vectors in the two-dimensional FESWMS model relative to the centerline of the piers (see Pier Drag Report in **Appendix B**). Depending on the distance from the vector to the pier, the measured angle of attack varied from approximately 21 to 31 degrees.

The correction factor for bed condition (K_3) accounts for the fact that maximum scour may be 10 to 30 percent greater than computed by the local pier scour equation. The K_3 factor was set to 1.1, which corresponds to the most typical condition of plane-bed through small dunes (up to 9 m high).

HEC-18 recommends that new bridge piers and foundations be designed to accommodate the predicted scour, rather than attempt to protect the piers with scour countermeasures.

4.0 REFERENCES AND DATA SOURCES

1. Wilbur Smith Associates, Section 5 East End Bridge - Hydraulic and Scour Analysis Report, September 20, 2011.
2. U.S. Department of Transportation, Federal Highway Administration, Evaluating Scour at Bridges, Hydraulic Engineering Circular No. 18, 4th Edition, March 2001.
3. Stantec, Draft Summary Particle Sizes and Boring Logs, October 4, 2011.
4. U.S. Department of Transportation, Federal Highway Administration, User's Manual for FESWMS FST2DH, Publication No. FHWA-RD-03-053, September 2002.
5. HEC-RAS, US Army Corps of Engineers, Version 4.1.0, 2010.
6. Surfacewater Modeling System, Environmental Modeling Systems Incorporated, Version 9.2, 2007.
7. Finite Element Surface-Water Modeling System: Two-Dimensional Flow in a Horizontal Plane, U.S. Federal Highways Administration, Version 3.2.2.
8. U.S. Department of Transportation, Federal Highway Administration, River Engineering for Highway Encroachments – Highways in the River Environment, Hydraulic Design Series Number 6, Publication no. FHWA NHI 10-004, December 2001.

APPENDIX A: BRIDGE AND APPROACH DRAWINGS

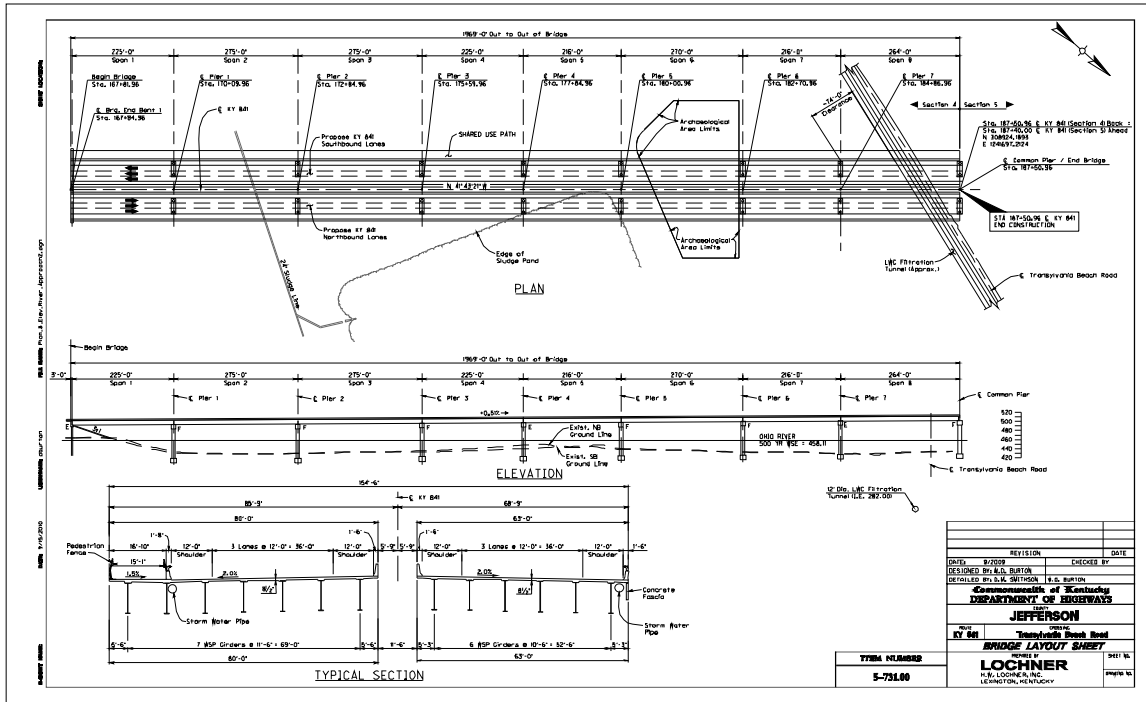
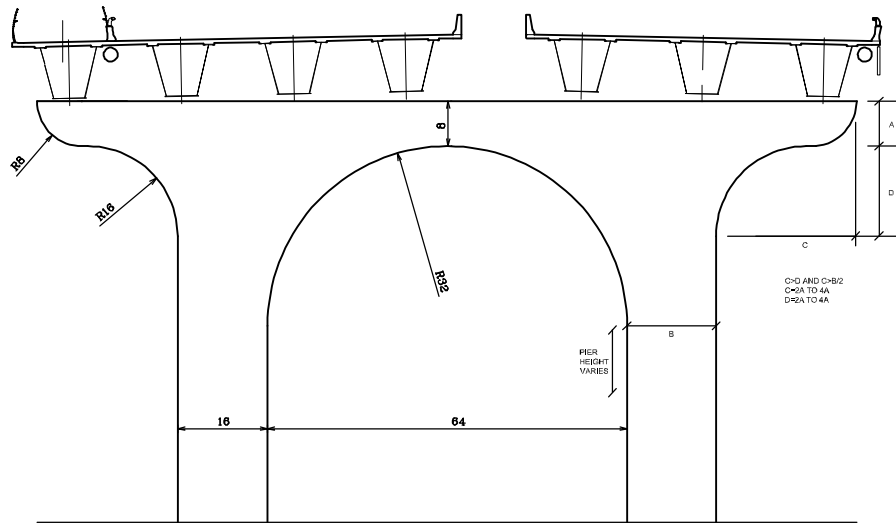


FIGURE 4
REVISED DESIGN OF SECTION 4 KY APPROACH STRUCTURE



RIVER APPROACH NORTH BOUND AND SOUTH BOUND
PIERS 1-6 PROPORTIONAL DESIGN AESTHETICS

APPENDIX B: GEOTECHNICAL DATA

PIER DRAG REPORT

FESWMS			100 year			500 year			Geotech Data			
Pier no.	Pier ID	Elem no.	Approach Flow			Approach Flow			Boring ID	Depth	D50	D95
-----	-----	-----	Vel (m/s)	Depth (m)	Angle (deg)	Vel (m/s)	Depth (m)	Angle (deg)	-----	(m)	(mm)	(mm)
1	Pier 1A	4358	0.098	1.709	28.237	0.419	3.282	24.901	B56	2.1 / 5.0	0.52	18
2	Pier 1B	4605	0.127	2.437	24.615	0.421	4.011	21.832	B54	1.2 / 3.5	0.13	12
3	Pier 2A	4113	0.328	7.524	21.329	0.601	9.099	22.624	B61	3	0.0053	0.26
4	Pier 2B	4357	0.285	7.527	23.92	0.56	9.1	23.38	B59/B62	3	0.0046	0.041
5	Pier 3A	4116	0.653	4.871	28.626	0.922	6.44	28.106	B64	3	0.0065	0.079
6	Pier 3B	4117	0.509	6.416	30.295	0.756	7.991	29.717				
7	Pier 4A	4120	1.086	2.012	27.911	1.331	3.568	28.243				
8	Pier 4B	4367	0.495	4.672	25.462	0.863	6.241	28.893	B66	4.6	0.041	0.31
9	Pier 5A	4369	0.818	3.537	26.507	1.028	5.095	27.505				
10	Pier 5B	4617	0.828	4.116	30.134	1.061	5.667	29.531	B70	3	0.007	0.2
11	Pier 6A	4626	0.916	6.095	29.44	1.133	7.669	29.17	B77	3.6 / 5.0	0.069	0.32
12	Pier 6B	4850	0.837	6.093	27.062	1.092	7.68	27.892	B75	3	0.025	0.32
-	Pier 7A	5412	0.912	6.414	24.92	1.137	7.986	25.42	B79	3	0.0039	0.049
-	Pier 7B	5937	0.813	6.723	24.01	0.974	8.297	26.23	B78	3	0.034	0.04

Data Sources

Hydraulic: SMS/FESWMS 100yr and 500 yr, WSA 9/20/2011

Geotech: Draft Grain Size Summary Table/Boring Logs, Stantec, 10/4/2011

East End Bridge-Section 4 Boring Plan

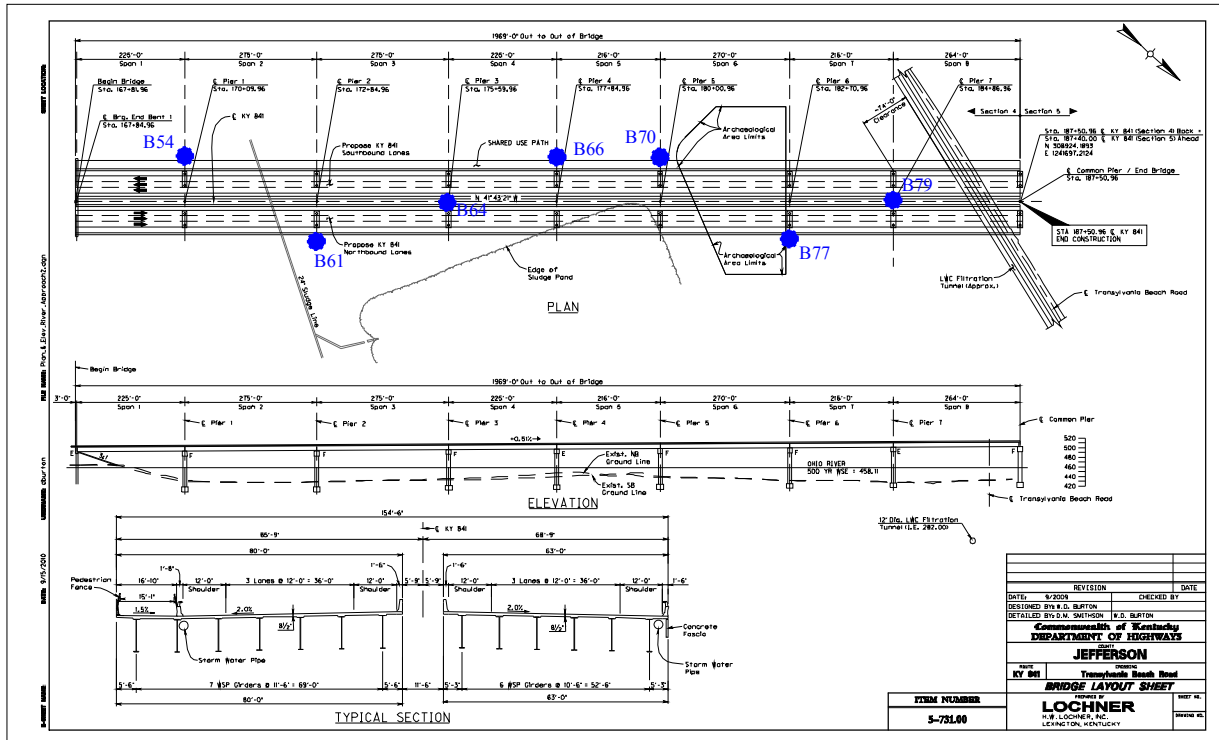


FIGURE 4
REVISED DESIGN OF SECTION 4 KY APPROACH STRUCTURE

BRIDGES		Stantec					
East End Approach - Section 4		Jefferson County, Kentucky					
Item No. 5-731.00		Date: 10/04/2011					
Re: Available Grain Size Information from Borings near Proposed East End Approach							
Boring No.	Station	Location	Onset	Depth Interval (feet)	Classification Unified AASHTO	D ₅₀ (mm)	D ₁₀ (mm)
B-59	172+84.96	6 LT		2.0 - 2.5	CL	0.058	0.56
				2.5 - 3.0	CL	0.058	0.56
				3.0 - 3.5	CL	0.043	0.43
				3.5 - 4.0	CL	0.043	0.43
				4.0 - 4.5	CL	0.043	0.43
				4.5 - 5.0	CL	0.043	0.43
				5.0 - 5.5	CL	0.043	0.43
				5.5 - 6.0	CL	0.043	0.43
				6.0 - 6.5	CL	0.043	0.43
				6.5 - 7.0	CL	0.043	0.43
B-61	172+84.96	69 RT		15.0 - 16.5	CL	0.058	0.56
				16.5 - 18.0	CL	0.058	0.56
				18.0 - 19.5	CL	0.058	0.56
				19.5 - 21.0	CL	0.058	0.56
				21.0 - 22.5	CL	0.058	0.56
				22.5 - 24.0	CL	0.058	0.56
				24.0 - 25.5	CL	0.058	0.56
				25.5 - 27.0	CL	0.058	0.56
				27.0 - 28.5	CL	0.058	0.56
				28.5 - 30.0	CL	0.058	0.56
B-62	172+59.96	86 LT		2.0 - 2.5	CL	0.058	0.56
				2.5 - 3.0	CL	0.058	0.56
				3.0 - 3.5	CL	0.058	0.56
				3.5 - 4.0	CL	0.058	0.56
				4.0 - 4.5	CL	0.058	0.56
				4.5 - 5.0	CL	0.058	0.56
				5.0 - 5.5	CL	0.058	0.56
				5.5 - 6.0	CL	0.058	0.56
				6.0 - 6.5	CL	0.058	0.56
				6.5 - 7.0	CL	0.058	0.56
B-64	175+59.96	6 RT		15.0 - 16.5	CL	0.058	0.56
				16.5 - 18.0	CL	0.058	0.56
				18.0 - 19.5	CL	0.058	0.56
				19.5 - 21.0	CL	0.058	0.56
				21.0 - 22.5	CL	0.058	0.56
				22.5 - 24.0	CL	0.058	0.56
				24.0 - 25.5	CL	0.058	0.56
				25.5 - 27.0	CL	0.058	0.56
				27.0 - 28.5	CL	0.058	0.56
				28.5 - 30.0	CL	0.058	0.56

1 N/A -- Represents 200 Wash Gradation, a complete classification was not performed.

BRIDGES		Stantec					
East End Approach - Section 4		Jefferson County, Kentucky					
Item No. 5-731.00		Date: 10/04/2011					
Re: Available Grain Size Information from Borings near Proposed East End Approach							
Boring No.	Station	Location	Onset	Depth Interval (feet)	Classification Unified AASHTO	D ₅₀ (mm)	D ₁₀ (mm)
B-51	167+84.96	6 LT		2.0 - 2.5	CL	0.028	0.42
				2.5 - 3.0	CL	0.028	0.42
				3.0 - 3.5	CL	0.028	0.42
				3.5 - 4.0	CL	0.028	0.42
				4.0 - 4.5	CL	0.028	0.42
				4.5 - 5.0	CL	0.028	0.42
				5.0 - 5.5	CL	0.028	0.42
				5.5 - 6.0	CL	0.028	0.42
				6.0 - 6.5	CL	0.028	0.42
				6.5 - 7.0	CL	0.028	0.42
B-53	167+84.96	74 RT		15.0 - 16.5	CL	0.028	0.42
				16.5 - 18.0	CL	0.028	0.42
				18.0 - 19.5	CL	0.028	0.42
				19.5 - 21.0	CL	0.028	0.42
				21.0 - 22.5	CL	0.028	0.42
				22.5 - 24.0	CL	0.028	0.42
				24.0 - 25.5	CL	0.028	0.42
				25.5 - 27.0	CL	0.028	0.42
				27.0 - 28.5	CL	0.028	0.42
				28.5 - 30.0	CL	0.028	0.42
B-54	170+09.96	86 LT		2.0 - 2.5	CL	0.028	0.42
				2.5 - 3.0	CL	0.028	0.42
				3.0 - 3.5	CL	0.028	0.42
				3.5 - 4.0	CL	0.028	0.42
				4.0 - 4.5	CL	0.028	0.42
				4.5 - 5.0	CL	0.028	0.42
				5.0 - 5.5	CL	0.028	0.42
				5.5 - 6.0	CL	0.028	0.42
				6.0 - 6.5	CL	0.028	0.42
				6.5 - 7.0	CL	0.028	0.42
B-56	170+17.96	6 RT		15.0 - 16.5	CL	0.028	0.42
				16.5 - 18.0	CL	0.028	0.42
				18.0 - 19.5	CL	0.028	0.42
				19.5 - 21.0	CL	0.028	0.42
				21.0 - 22.5	CL	0.028	0.42
				22.5 - 24.0	CL	0.028	0.42
				24.0 - 25.5	CL	0.028	0.42
				25.5 - 27.0	CL	0.028	0.42
				27.0 - 28.5	CL	0.028	0.42
				28.5 - 30.0	CL	0.028	0.42

1 N/A -- Represents 200 Wash Gradation, a complete classification was not performed.

BRIDGES		Stantec					
East End Approach -- Section 4		Jefferson County, Kentucky					
Item No. 5-731.00		Date: 10/04/2011					
Re: Available Grain Size Information from Borings near Proposed East End Approach							
Boring No.	Station	Location	Onset	Depth Interval (feet)	Classification Unified / AASHTO	D ₅₀ (mm)	D ₁₀ (mm)
B-76	184+86.96	86 LT		2.0 - 2.5	CL	0.0071	0.09
				2.5 - 5.0	A-6	0.0071	0.09
				5.0 - 10.5	CH	0.0071	0.09
				10.5 - 15.5	CL	0.0071	0.09
				15.5 - 20.5	CL	0.0071	0.09
				20.5 - 21.5	CL	0.0071	0.09
				21.5 - 26.5	CL	0.0071	0.09
				26.5 - 30.5	CL	0.0071	0.09
				30.5 - 35.0	CL	0.0071	0.09
				35.0 - 41.5	CL	0.0071	0.09
B-79	184+86.96	6 LT		2.0 - 2.5	CL	0.0071	0.09
				2.5 - 5.0	A-6	0.0071	0.09
				5.0 - 10.5	CH	0.0071	0.09
				10.5 - 15.5	CL	0.0071	0.09
				15.5 - 20.5	CL	0.0071	0.09
				20.5 - 21.5	CL	0.0071	0.09
				21.5 - 26.5	CL	0.0071	0.09
				26.5 - 30.5	CL	0.0071	0.09
				30.5 - 35.0	CL	0.0071	0.09
				35.0 - 41.5	CL	0.0071	0.09

BRIDGES		Stantec					
East End Approach -- Section 4		Jefferson County, Kentucky					
Item No. 5-731.00		Date: 10/04/2011					
Re: Available Grain Size Information from Borings near Proposed East End Approach							
Boring No.	Station	Location	Onset	Depth Interval (feet)	Classification Unified / AASHTO	D ₅₀ (mm)	D ₁₀ (mm)
B-66	177+84.96	86 LT		2.0 - 2.5	CL	0.0038	0.13
				2.5 - 5.0	A-6	0.0038	0.13
				5.0 - 10.5	CH	0.0038	0.13
				10.5 - 15.5	CL	0.0038	0.13
				15.5 - 20.5	CL	0.0038	0.13
				20.5 - 21.5	CL	0.0038	0.13
				21.5 - 26.5	CL	0.0038	0.13
				26.5 - 30.5	CL	0.0038	0.13
				30.5 - 35.0	CL	0.0038	0.13
				35.0 - 41.5	CL	0.0038	0.13
B-70	180+00.96	86 LT		2.0 - 2.5	CL	0.0113	0.17
				2.5 - 5.0	A-6	0.0113	0.17
				5.0 - 10.5	CH	0.0113	0.17
				10.5 - 15.5	CL	0.0113	0.17
				15.5 - 18.5	SM	0.007	0.2
				18.5 - 20.5	SM	0.009	0.33
				20.5 - 21.5	CL	0.009	0.33
				21.5 - 26.5	CL	0.004	0.31
				26.5 - 30.5	CL	0.004	0.31
				30.5 - 35.0	CL	0.004	0.31
B-75	182+70.96	6 LT		2.0 - 2.5	CL	0.0039	0.12
				2.5 - 5.0	A-6	0.0039	0.12
				5.0 - 10.5	CH	0.0039	0.12
				10.5 - 15.5	CL	0.0039	0.12
				15.0 - 16.5	SM	0.009	0.34
				16.5 - 20.5	SM	0.009	0.34
				20.5 - 21.5	SM	0.009	0.34
				21.5 - 26.5	SM	0.009	0.34
				26.5 - 30.5	SM	0.009	0.34
				30.5 - 31.5	SM	0.009	0.34
B-77	182+70.96	69 RT		2.0 - 2.5	CL	0.0098	0.32
				2.5 - 5.0	A-6	0.0098	0.32
				5.0 - 10.5	CH	0.0098	0.32
				10.5 - 15.5	CL	0.0098	0.32
				15.0 - 16.5	SM	0.009	0.28
				16.5 - 20.5	SM	0.009	0.28
				20.5 - 21.5	SM	0.009	0.28
				21.5 - 26.5	SM	0.009	0.28
				26.5 - 30.5	SM	0.009	0.28
				30.5 - 31.5	SM	0.009	0.28

Immediate Water Depth	32.2 (08/31/11)	Static Water Depth	NA	Driller	Danny Jesse	Geologist	James Adams
Surface Elevation	463.6'	Total Depth	166.0'	Location	167+84.96 86.0' Lt.	Overburden	

Hole Number	50	Sample No.	57 / 50	Depth (ft)	10.0	Rec. (ft)	10.0	SPT Blows	100	Sample Type		Remarks	
Surface Elevation	463.6'	Stiffly RQD		Run (ft)		Rec. (ft)		Rec (%)		SDI (JS)			

Lithology		Description		Overburden		Rock Core		Elevation	165
Depth									160

Lithology		Description		Overburden		Rock Core		Elevation	105
Depth									110

DRAFT									
Limestone & Shale: Limestone (70%), gray, fine grained, hard, thin to medium bedded, shale streaks, stringers and partings. fossiliferous. Shale (30%), gray, silty.									
Elevation	165								165

DRAFT									
Overburden overburden.									
Elevation	115								115

Elevation	120								120
Elevation	125								125

Elevation	130								130
Elevation	135								135

Elevation	140								140
Elevation	145								145

Elevation	150								150
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Top of Rock = 128.0' Base Weathered Rock = 128.0' RDZ = 128.0' Elevation = 335.6'									
Louisville Limestone Laurel Dolomite									

Top of Rock = 128.0' Base Weathered Rock = 128.0' RDZ = 128.0' Elevation = 335.6'									
Louisville Limestone Laurel Dolomite									

Immediate Water Depth	32.2 (08/31/11)	Static Water Depth	NA	Driller	Danny Jesse	Geologist	James Adams
Surface Elevation	463.6'	Total Depth	166.0'	Location	167+84.96 86.0' Lt.	Overburden	

Hole Number	50	Sample No.	57 / 50	Depth (ft)	10.0	Rec. (ft)	10.0	SPT Blows	100	Sample Type		Remarks	
Surface Elevation	463.6'	Stiffly RQD		Run (ft)		Rec. (ft)		Rec (%)		SDI (JS)			

Lithology		Description		Overburden		Rock Core		Elevation	165
Depth									160

Lithology		Description		Overburden		Rock Core		Elevation	105
Depth									110

DRAFT									
Limestone & Shale: Limestone (70%), gray, fine grained, hard, thin to medium bedded, shale streaks, stringers and partings. fossiliferous. Shale (30%), gray, silty.									
Elevation	165								165

DRAFT									
Overburden overburden.									
Elevation	115								115

Elevation	120								120
Elevation	125								125

Elevation	130								130
Elevation	135								135

Elevation	140								140
Elevation	145								145

Elevation	150								150
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Top of Rock = 128.0' Base Weathered Rock = 128.0' RDZ = 128.0' Elevation = 335.6'									
Louisville Limestone Laurel Dolomite									

Top of Rock = 128.0' Base Weathered Rock = 128.0' RDZ = 128.0' Elevation = 335.6'									
Louisville Limestone Laurel Dolomite									

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Hole Type <u>core</u>		Remarks
							Start Date <u>09/06/2011</u>	End Date <u>09/07/2011</u>	
Elevation	Depth	Rock Core	Sticky ROD	Run (ft)	Rec (ft)	Rec (%)	SDI (JS)	Rig Number <u>1050.ATV</u>	
105									
110									
115									
120									
125									
336.9	128.6	(Begin Core)							
130			12 / 12	9.4	8.5	90			
135									
140		Limestone & Shale: Limestone (60%) gray, fine grained, hard, thin to medium bedded, shale streaks, stringers and partings. Shale (40%), gray, silty.					138.0		
145			42 / 35	10.0	10.0	100			
150			46 / 46	11.3	11.3	100			RCS-1 @ 146.1-146.7 148.0
Top of Rock = 128.6' Base Weathered Rock = 131.2' RDZ = 131.2' Elevation = 334.3' Elevation = 334.3'									

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Hole Type <u>core</u>		Remarks
							Start Date <u>09/06/2011</u>	End Date <u>09/07/2011</u>	
Elevation	Depth	Rock Core	Sticky ROD	Run (ft)	Rec (ft)	Rec (%)	SDI (JS)	Rig Number <u>1050.ATV</u>	
155			46 / 46	11.3	11.3	100			RCS-2 @ 153.2-154.2 155
306.2	159.3								
160		DRAFT (Bottom of Hole 159.3')							153.3
165									
170									
175									
180									
185									
190									
195									
200									
Top of Rock = 128.6' Base Weathered Rock = 131.2' RDZ = 131.2' Elevation = 336.9' Elevation = 334.3'									

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Hole Type <u>core</u>		Remarks
							Start Date <u>09/06/2011</u>	End Date <u>09/07/2011</u>	
Elevation	Depth	Rock Core	Sticky ROD	Run (ft)	Rec (ft)	Rec (%)	SDI (JS)	Rig Number <u>1050.ATV</u>	
155			46 / 46	11.3	11.3	100			RCS-2 @ 153.2-154.2 155
306.2	159.3								
160		DRAFT (Bottom of Hole 159.3')							153.3
165									
170									
175									
180									
185									
190									
195									
200									
Top of Rock = 128.6' Base Weathered Rock = 131.2' RDZ = 131.2' Elevation = 336.9' Elevation = 334.3'									

Drilling Firm: Stantec
For: Division of Structural Design
Geotechnical Branch

Drilling Firm: Stantec
For: Division of Structural Design
Geotechnical Branch

GEOLOGIST'S SUBSURFACE LOG

GEOLOGIST'S SUBSURFACE LOG

Lithology		Description		Overburden		Sample No.		Depth (ft)		Rec. (ft)		SPT Blows		Sample Type		Remarks	
Elevation	Depth			Stkfy RCD	Run (ft)	Run (ft)	Stkfy RCD	Run (ft)	Run (ft)	Rec (ft)	Rec (%)	Run (ft)	Rec (%)	SDI (JS)			
100363.2	75.0																
75.386.2	75.0																
80.0																	
85.0																	
90.0																	
95.0																	
100363.2	100.0																
Top of Rock = 127.2' Elevation = 336.0' Base Weathered Rock = 127.2' RCD = 127.2' Elevation = 336.0' Louisville Limestone Laurel Dolomite																	

Lithology		Description		Overburden		Sample No.		Depth (ft)		Rec. (ft)		SPT Blows		Sample Type		Remarks	
Elevation	Depth			Stkfy RCD	Run (ft)	Run (ft)	Stkfy RCD	Run (ft)	Run (ft)	Rec (ft)	Rec (%)	Run (ft)	Rec (%)	SDI (JS)			
50.0	50.0																
45.0																	
40.0																	
35.0																	
30.0																	
25.0																	
20.0																	
15.0	15.0																
10.0	10.0																
5.0																	
Top of Rock = 127.2' Elevation = 336.0' Base Weathered Rock = 127.2' RCD = 127.2' Elevation = 336.0' Louisville Limestone Laurel Dolomite																	

Project ID: **UNKNOWN**
Item Number: **5-731.00**
Project Type: **Structure State Bridge**
Project Manager: **Mark Litkenhus**

Project ID: **UNKNOWN**
Item Number: **5-731.00**
Project Type: **Structure State Bridge**
Project Manager: **Mark Litkenhus**

Hole Number: **53**
Surface Elevation: **463.2'**
Total Depth: **167.9'**
Location: **167+84.96 74.0' RL**

Immediate Water Depth: **51.2 (09/07/11)**
Static Water Depth: **NA**
Driller: **Danny Jessie**
Geologist: **James Adams**

Start Date: **09/07/2011**
End Date: **09/07/2011**
Latitude (83): **38.336206**
Longitude (83): **-85.633386**

Hole Type: **core and sample**
Rig Number: **1050.ATV**
GC-1211
Jeffersonville, New Albany, and Charlestown

Hole Number: **53**
Surface Elevation: **463.2'**
Total Depth: **167.9'**
Location: **167+84.96 74.0' RL**

Immediate Water Depth: **51.2 (09/07/11)**
Static Water Depth: **NA**
Driller: **Danny Jessie**
Geologist: **James Adams**

Start Date: **09/07/2011**
End Date: **09/07/2011**
Latitude (83): **38.336206**
Longitude (83): **-85.633386**

Hole Type: **core and sample**
Rig Number: **1050.ATV**
GC-1211
Jeffersonville, New Albany, and Charlestown

DRAFT

DRAFT

Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense to dense, poorly graded, some gravel.

Overburden: Clay, brown, moist, some sand.

Boring advanced with 4.25" augers @ 1' intervals. No casing utilized to sample bedrock @ 3'.

Boring moved 5.0' RT to drill off of driveway @ 9'

Overburden: Silt, light brown, wet, fine grained, hard, some sand.

Overburden: Sand, brown, moist, fine grain, very loose to dense, some silt, some gravel.

Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense to very dense, poorly graded, some gravel.

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Elevation
Hole Number <u>55</u> Surface Elevation <u>438.3'</u> Total Depth <u>136.2'</u> Location <u>T70+17.86 6.0' Lt.</u>	Immediate Water Depth <u>NA</u> Static Water Depth <u>NA</u> Driller <u>Danny Jessie</u> Geologist <u>James Adams</u>			Start Date <u>09/12/2011</u> End Date <u>09/12/2011</u> Latitude (83) <u>38.336521</u> Longitude (83) <u>-85.636110</u>			Hole Type <u>core</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>		
	DRAFT							Boring advanced with 4.25" augers @ 1' intervals. No coring utilized to bedrock @ 3' depth. Boring was staked on slope, dozer work was required to clear path to stake. Boring moved ahead station 8.0' from stake to provide working area. Drilling mud added to the boring at a depth of 30.0 ft below the ground surface @ 30	5
									10
									15
									20
									25
									30
									35
									40
									45
									50
Top of Rock = 105.2' Base Weathered Rock = 105.2' RDZ = 105.2' Elevation = 333.1'									Louisville Limestone Laurel Dolomite

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Elevation
Hole Number <u>54</u> Surface Elevation <u>430.5'</u> Total Depth <u>140.7'</u> Location <u>T70+09.96 86.0' Lt.</u>	Immediate Water Depth <u>17.8 (09/13/11)</u> Static Water Depth <u>NA</u> Driller <u>Danny Jessie</u> Geologist <u>James Adams</u>			Start Date <u>09/12/2011</u> End Date <u>09/13/2011</u> Latitude (83) <u>38.336373</u> Longitude (83) <u>-85.636315</u>			Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>		
	DRAFT								105
									110
									115
									120
									125
									130
									135
									140
									145
									150
Top of Rock = 100.0' Base Weathered Rock = 100.8' RDZ = 100.8' Elevation = 329.7'									Louisville Limestone Laurel Dolomite

DRAFT

Limestone & Shale: Limestone (60%), gray, fine grained, hard, thin to medium bedded, shale streaks, stringers and partings, fossiliferous. Shale (40%), gray, silty.

(Bottom of Hole 140.7)

Overburden: overburden.

GEOLOGIST'S SUBSURFACE LOG

Printed: 9/30/11
Page 2 of 3

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> Latitude (83) <u>38.337077</u> Longitude (83) <u>-85.636758</u>
			8	50.0-51.5	1.5	10-17-15	SPT		
	55	DRAFT	9	55.0-56.5	1.5	7-8-10	SPT		
	60	Overburden: Sand, brown and gray, wet, fine to medium grained, very loose to dense, some clay.	10	60.0-61.5	1.5	15-13-14	SPT		
	65		11	65.0-66.5	1.5	16-11-8	SPT		
	70		12	70.0-71.5	1.5	0-0-4	SPT		
	75	Overburden: Clay, dark gray, wet, soft to hard, some silt.	13	75.0-76.5	1.5	5-17-17	SPT		
	80		14	80.0-81.5	1.5	8-12-13	SPT		
	85	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense to very dense, poorly graded, some gravel.	15	85.0-86.5	1.2	8-8-11	SPT		
	90		16	90.0-91.5	0.8	7-11-19	SPT		
	95		17	95.0-95.6	0.6	15-50.0.10'	SPT	Rock in bottom of split spoon @ 95-96 weathered @ 95.6-96.6	
	100	Top of Rock = 95.6' Elevation = 332.7	24 / 22	10.0	10.0	100			
Base Weathered Rock = 96.6' RDZ = 96.6' Elevation = 331.7									Louisville Limestone Laurel Dolomite

GEOLOGIST'S SUBSURFACE LOG

Printed: 9/30/11
Page 1 of 3

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> Latitude (83) <u>38.337077</u> Longitude (83) <u>-85.636758</u>
	427.3	DRAFT	1	2.0-4.0	2.0		ST	Boring advanced with 4.25" augers @ 1' intervals. No casing utilized to sample bedrock @ 3'	
	5		2	5.0-7.0	2.0		ST		
	10	Overburden: Clay, brown, moist, some sand.	3	10.0-12.0	2.0		ST		
	15		4	15.0-17.0	0.5		ST		
	411.3		1	17.0-18.5	0.6	0-0-0	SPT	Drilling mud added to the boring at a depth of 17.0 ft below the ground surface @ 17'	
	20	Overburden: Clay, brown, moist, very soft to stiff, some sand.	2	20.0-21.5	0.8	2-4-6	SPT		
	25		3	25.0-26.5	0.8	2-5-9	SPT		
	30		4	30.0-31.5	1.0	5-9-11	SPT		
	35	Overburden: Sand, brown and gray, wet, fine to medium grained, very loose to dense, some clay.	5	35.0-36.5	1.5	7-11-12	SPT		
	40		6	40.0-41.5	1.5	6-8-10	SPT		
	45		7	45.0-46.5	1.5	7-5-7	SPT		
	50	Top of Rock = 95.6' Elevation = 332.7							
Base Weathered Rock = 96.6' RDZ = 96.6' Elevation = 331.7									Louisville Limestone Laurel Dolomite

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>
	428.9	Overburden: topsoil.							
	55	DRAFT	1	2.0-4.0	2.0		ST	Boring advanced with 4.25" augers @ 1' intervals. No casing utilized to sample bedrock @ 3'	
	60		2	5.0-7.0	2.0		ST		
	65		3	10.0-12.0	2.0		ST		
	70		4	15.0-17.0	0.5		ST		
	75	Overburden: Sand, brown, wet, fine to medium grained, very loose, clayey.	1	20.0-21.5	0.7	0-0-1	SPT	Drilling mud added to the boring at a depth of 20.0 ft below the ground surface @ 20'	
	80		2	25.0-26.5	0.8	4-5-7	SPT		
	85	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense, poorly graded, some gravel.	3	30.0-31.5	0.7	7-10-12	SPT		
	90		4	35.0-36.5	1.3	5-6-7	SPT		
	95		5	40.0-41.5	1.5	4-5-8	SPT		
	100	Overburden: Sand, brown, wet, fine grained, loose to medium dense.	6	45.0-46.5	1.5	5-10-8	SPT		
	331.4	Top of Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							
	351.7	Base Weathered Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>
	428.9	Overburden: topsoil.							
	55	DRAFT	1	2.0-4.0	2.0		ST	Boring advanced with 4.25" augers @ 1' intervals. No casing utilized to sample bedrock @ 3'	
	60		2	5.0-7.0	2.0		ST		
	65		3	10.0-12.0	2.0		ST		
	70		4	15.0-17.0	0.5		ST		
	75	Overburden: Sand, brown, wet, fine to medium grained, very loose, clayey.	1	20.0-21.5	0.7	0-0-1	SPT	Drilling mud added to the boring at a depth of 20.0 ft below the ground surface @ 20'	
	80		2	25.0-26.5	0.8	4-5-7	SPT		
	85	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense, poorly graded, some gravel.	3	30.0-31.5	0.7	7-10-12	SPT		
	90		4	35.0-36.5	1.3	5-6-7	SPT		
	95		5	40.0-41.5	1.5	4-5-8	SPT		
	100	Overburden: Sand, brown, wet, fine grained, loose to medium dense.	6	45.0-46.5	1.5	5-10-8	SPT		
	331.4	Top of Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							
	351.7	Base Weathered Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>
	428.9	Overburden: topsoil.							
	55	DRAFT	1	2.0-4.0	2.0		ST	Boring advanced with 4.25" augers @ 1' intervals. No casing utilized to sample bedrock @ 3'	
	60		2	5.0-7.0	2.0		ST		
	65		3	10.0-12.0	2.0		ST		
	70		4	15.0-17.0	0.5		ST		
	75	Overburden: Sand, brown, wet, fine to medium grained, very loose, clayey.	1	20.0-21.5	0.7	0-0-1	SPT	Drilling mud added to the boring at a depth of 20.0 ft below the ground surface @ 20'	
	80		2	25.0-26.5	0.8	4-5-7	SPT		
	85	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense, poorly graded, some gravel.	3	30.0-31.5	0.7	7-10-12	SPT		
	90		4	35.0-36.5	1.3	5-6-7	SPT		
	95		5	40.0-41.5	1.5	4-5-8	SPT		
	100	Overburden: Sand, brown, wet, fine grained, loose to medium dense.	6	45.0-46.5	1.5	5-10-8	SPT		
	331.4	Top of Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							
	351.7	Base Weathered Rock = 96.5' RDZ = 96.5' Elevation = 331.4'							

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Depth	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type: <u>core</u> Rig Number: <u>1050.ATV</u>
433.3	104.6	Overburden: Overburden. DRAFT Limestone & Shale: Limestone (70%), gray, medium grained, hard thin to medium bedded, shale streaks, stringers and partings. Shale (30%), gray, silty.							Immediate Water Depth <u>19.8 (08/25/11)</u> Static Water Depth <u>NA</u> Driller: <u>Danny Jessie</u> Geologist: <u>James Adams</u>
				0/0	3.4	3.2	94		End Date <u>08/25/2011</u> Latitude (83): <u>38.337773</u> Longitude (83): <u>-85.637215</u>
				36/19	10.0	10.0	100		108.0 soft zone @ 108-108.2
									115 soft zone @ 115.6-116.4
									120 RCS-1 @ 119.2-120
				65/63	10.0	10.0	100		125
									130 128.0
				85/81	10.0	10.0	100		135
				89/89	7.6	7.6	100		140 RCS-2 @ 139.3-140.1
									145 146.8
								150 (Bottom of Hole 145.6')	
Top of Rock = 104.6' Elevation = 333.1'		Base Weathered Rock = 104.9' RDZ = 104.9' Elevation = 332.8'		Louisville Limestone Laurel Dolomite		Louisville Limestone Laurel Dolomite			

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>			
Lithology	Depth	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type: <u>core and sample</u> Rig Number: <u>750.ATV</u>
433.3	1.0	Overburden: Topsoil. DRAFT Overburden: Clay, brown, moist, sandy.							Immediate Water Depth <u>18.1 (08/23/11)</u> Static Water Depth <u>NA</u> Driller: <u>Mark Martin</u> Geologist: <u>James Adams</u>
				1	2.0-4.0	2.0	2.0	ST	End Date <u>08/23/2011</u> Latitude (83): <u>38.337940</u> Longitude (83): <u>-85.638143</u>
				2	5.0-7.0	2.0	2.0	ST	GC-1211 Jeffersonville, New Albany, and Charlestown
				3	10.0-12.0	2.0	2.0	ST	
				4	15.0-17.0	2.0	2.0	ST	
				1	20.0-21.5	1.5	1-0-1	SPT	Drilling mud added to the boring at a depth of 20.0 ft below the ground surface @ 20
				2	25.0-26.5	1.5	3-5-5	SPT	
				3	30.0-31.5	0.5	5-5-2	SPT	
				4	35.0-36.5	1.2	2-4-6	SPT	
				5	40.0-41.5	1.0	7-8-9	SPT	
			6	45.0-46.5	1.1	4-5-8	SPT		
								45	
								50	
Top of Rock = 101.0' Elevation = 333.3		Base Weathered Rock = 101.2' RDZ = 101.2' Elevation = 333.1'		Louisville Limestone Laurel Dolomite		Louisville Limestone Laurel Dolomite			

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Elevation
Hole Number 6Z Surface Elevation 438.8' Total Depth 134.5' Location 177+84.96 31.0' Lt.	Immediate Water Depth 19.3 (08/25/11) Static Water Depth NA Driller Mark Martin Geologist James Adams						Hole Type core Rig Number 750 ATV GC-1211 Jeffersonville, New Albany, and Charlestown		
	DRAFT								55
									60
									65
									70
									75
									80
									85
									90
									95
									100
Top of Rock = 104.5' Base Weathered Rock = 105.0' RDZ = 105.0' Elevation = 334.3' Elevation = 333.8'									Louisville Limestone Laurel Dolomite

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Elevation
Hole Number 6Z Surface Elevation 438.8' Total Depth 134.5' Location 177+84.96 31.0' Lt.	Immediate Water Depth 19.3 (08/25/11) Static Water Depth NA Driller Mark Martin Geologist James Adams						Hole Type core Rig Number 750 ATV GC-1211 Jeffersonville, New Albany, and Charlestown		
	DRAFT							Boring advanced with 4.25" augers @ 1' intervals. No coring utilized to sample bedrock @ 3'	5
									10
									15
									20
								Drilling mud added to the boring at a depth of 20.0 ft below the ground surface. An additional 25.0' left of centerline to avoid toe of embankment @ 20'	25
									30
									35
									40
									45
									50
Top of Rock = 104.5' Base Weathered Rock = 105.0' RDZ = 105.0' Elevation = 334.3' Elevation = 333.8'									Louisville Limestone Laurel Dolomite

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>					
Lithology	Elevation	Depth	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	
											Stiffly ROD
	105		DRAFT		39 / 29	8.0	8.0	100		soft zone @ 99-99.2	
	110				57 / 57	10.0	10.0	100		soft zone @ 111.4-111.7	
	115									RCS-1 @ 116.2	
	120					43 / 37	10.0	10.0	100	soft zone @ 119.1-119.4	
	125									126.2	
	130					77 / 75	10.0	10.0	100	RCS 2 @ 131.2-132.1	
	135									136.2	
	140					29 / 29	2.0	2.0	100	138.2	
	145									138.2	
	150										
(Bottom of Hole 138.2)											
Top of Rock = 98.2' Base Weathered Rock = 99.2' RDZ = 99.2' Elevation = 334.5' Elevation = 333.5'											

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u> Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Project Type: <u>Structure State Bridge</u> Project Manager: <u>Mark Litkenhus</u>					
Lithology	Elevation	Depth	Description	Overburden	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	
											Stiffly ROD
	55		DRAFT		39 / 29	8.0	8.0	100		weathered @ 98.2-99.2	
	60										
	65										
	70										
	75										
	80										
	85										
	90										
	95										
	334.5	98.2			(Begin Core)						
Top of Rock = 98.2' Base Weathered Rock = 99.2' RDZ = 99.2' Elevation = 334.5' Elevation = 333.5'											

GEOLOGIST'S SUBSURFACE LOG

Project ID: UNKNOWN Item Number: 5-731.00		Jefferson - LSIORB East End Approach				Project Type: Structure State Bridge Project Manager: Mark Litkenhus			
Lithology	Elevation	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	Hole Type <u>core and sample</u> Rig Number <u>1050.ATV</u> GC-1211 <u>Jeffersonville, New Albany, and Charlestown</u>
	433.0	Overburden: Topsoil							
	435.0	DRAFT	1	2.0-4.0	2.0		ST	Boring advanced with 4.25" NQ casing utilized to sample bedrock @ 3	
	442.0	Overburden: Clay, brown, moist, some sand.	2	5.0-7.0	1.8		ST		
	455.0		3	10.0-12.0	0.0		ST		
	465.0		1	12.0-13.5	1.5	0-0-0	SPT		
	475.0		2	15.0-16.5	1.5	0-1-0	SPT		
	480.0	Overburden: Clay, brown, wet, very soft, some sand.	3	20.0-21.5	1.5	0-1-0	SPT	Drilling mud added to the boring at a depth of 20.0 ft below the ground surface @ 20	
	490.0		4	25.0-26.5	1.5	1-1-1	SPT		
	500.0	Overburden: Clay, gray, wet, soft, some sand.	5	30.0-31.5	0.4	3-10-14	SPT		
	510.0	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense, poorly graded, some gravel.	6	35.0-36.5	0.8	9-10-15	SPT		
	520.0	Overburden: Clay, gray, wet, very stiff, some sand.	7	40.0-41.5	1.0	8-15-15	SPT		
	530.0	Overburden: Sand, brown to gray, wet, fine to medium grained, medium dense, poorly graded, some gravel.	8	45.0-46.5	0.5	6-13-16	SPT		
	540.0								
	550.0								
	560.0								
	570.0								
	580.0								
	590.0								
	600.0								
	610.0								
	620.0								
	630.0								
	640.0								
	650.0								
	660.0								
	670.0								
	680.0								
	690.0								
	700.0								
	710.0								
	720.0								
	730.0								
	740.0								
	750.0								
	760.0								
	770.0								
	780.0								
	790.0								
	800.0								
	810.0								
	820.0								
	830.0								
	840.0								
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	960.0								
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	980.0								
	990.0								
	1000.0								
	1010.0								
	1020.0								
	1030.0								
	1040.0								
	1050.0								
	1060.0								
	1070.0								
	1080.0								
	1090.0								
	1100.0								
	1110.0								
	1120.0								
	1130.0								
	1140.0								
	1150.0								
	1160.0								
	1170.0								
	1180.0								
	1190.0								
	1200.0								
	1210.0								
	1220.0								
	1230.0								
	1240.0								
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	1270.0								
	1280.0								
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	1460.0								
	1470.0								
	1480.0								
	1490.0								
	1500.0								
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	1600.0								
	1610.0								
	1620.0								
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	1660.0								
	1670.0								
	1680.0								
	1690.0								
	1700.0								
	1710.0								
	1720.0								
	1730.0								
	1740.0								
	1750.0								
	1760.0								
	1770.0								
	1780.0								
	1790.0								
	1800.0								
	1810.0								
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	2100.0								
	2110.0								
	2120.0								
	2130.0								
	2140.0								
	2150.0								
	2160.0								
	2170.0								
	2180.0								
	2190.0								
	2200.0								
	2210.0				</				

GEOLOGIST'S SUBSURFACE LOG

Project ID: <u>UNKNOWN</u>		Project Type: <u>Structure State Bridge</u>				Project Manager: <u>Mark Litkenhus</u>		
Item Number: <u>5-731.00</u>		Jefferson - LSIORB East End Approach				Hole Type: <u>core</u>		
Hole Number: <u>81</u>		Start Date: <u>08/08/2011</u>		Rig Number: <u>750 ATV</u>		End Date: <u>08/08/2011</u>		
Surface Elevation: <u>437.5'</u>		Static Water Depth: <u>NA</u>		Latitude (83): <u>38.339647</u>		Longitude (83): <u>-85.639402</u>		
Total Depth: <u>138.5'</u>		Driller: <u>Murrell Westington</u>		Geologist: <u>James Adams</u>				
Location: <u>184+96.96 69.0' RL</u>		Geotechnical Branch						
Lithology	Description	Sample No.	Depth (ft)	Rec. (ft)	SPT Blows	Sample Type	Remarks	
Elevation	Depth	Stkfy ROD	Run (ft)	Rec (ft)	Rec (%)	SDI (JS)		
	DRAFT	58 / 47	7.4	7.4	100		98-98.3	
105							105	
			11 / 11	10.0	2.8	28		rock not recovered, ground up by core barrel @ 108.1-115.4
110							108.4	
			73 / 64	5.0	5.0	100		115
115								116.4
			100 / 90	5.0	5.0	100		RCS-1 @ 116.8-117.5
120								120
			92 / 84	10.0	10.0	100		128.4
125								RCS-2 @ 128.1-129
			0 / 0	3.1	1.6	52		130
130								135
								rock not recovered, ground up by core barrel @ 137-138.5
135								138.4
293.0		138.5						145
140							145	
145							145	
150							150	
Top of Rock = 98.0' Elevation = 333.5'						Louisville Limestone		
Base Weathered Rock = 99.0' ROD = 99.0' Elevation = 332.5'						Laurel Dolomite		

APPENDIX C: DETAILED SCOUR CALCULATIONS

100 YEAR SIMPLE PIER SCOUR COMPUTATIONS

A **simple** pier is a single shaft, column, or multiple columns exposed to the flow. The pier itself is socketed into rock, and no footing is needed.

Project: Louisville Southern Indiana Ohio River East End Bridge
Configuration: Final
Date: 9/20/2011
Prepared by: LMW Chkd by: PXVG
Methodology: HEC-18, 4th Edition, Section 6.1
Scour Case: Simple
Return Period (yrs): 100
Flow Type: Steady
No. of Piers: 7
Units: English

BED CONDITIONS

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
D ₅₀	Diameter at which 50% of the soil grains pass	4.27E-04	1.74E-05	2.13E-05	1.35E-04	2.30E-05	2.26E-04	1.28E-05	ft
D ₉₅	Diameter at which 95% of the soil grains pass	3.94E-02	8.53E-04	2.59E-04	1.02E-03	6.56E-04	1.05E-03	1.61E-04	ft
BC	Bed Condition (clear-water, plane bed, small dunes, medium dunes, large dunes)	small dunes	small dunes	small dunes	small dunes	small dunes	small dunes	small dunes	

FLOW DATA

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
θ	Angle of attack	25.0	21.0	29.0	28.0	30.0	29.0	25.0	degrees
y ₁	Flow depth directly upstream of the pier	8.0	24.7	16.0	6.6	13.5	20.0	21.0	ft
V ₁	Mean velocity of flow directly upstream of the pier	0.42	1.08	2.14	3.56	2.72	3.01	2.99	ft / s
g	Acceleration of gravity	32.2	32.2	32.2	32.2	32.2	32.2	32.2	ft / s ²

PIER DATA

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
PS	Pier shape (Square, Round, Cylindrical, Group, Sharp)	Group	Group	Group	Group	Group	Group	Group	
a _p	Pier stem width	8.0	8.0	8.0	8.0	8.0	8.0	8.0	ft
L _p	Length of pier stem	16.0	16.0	16.0	16.0	16.0	16.0	16.0	ft
C	Number of columns per bent	2.0	2.0	2.0	2.0	2.0	2.0	3.0	
S	Column spacing	80.0	80.0	80.0	80.0	80.0	80.0	40.0	

SIMPLE PIER SCOUR CALCULATIONS

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
F _{r1}	Froude number directly upstream of the pier	0.03	0.04	0.09	0.24	0.13	0.12	0.11	
K ₁	Correction factor for pier nose shape from Table 6.1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
K ₂	Correction factor for angle of attack of flow	1.44	1.38	1.49	1.48	1.50	1.49	1.20	
K ₃	Correction factor for bed condition from Table 6.3	1.10	1.10	1.10	1.10	1.10	1.10	1.10	
K ₄	Correction factor for armoring by bed material size	0.40	1.00	1.00	1.00	1.00	1.00	1.00	
V _s	Velocity ratio required to calculate K ₄	1.00	1.00	28.27	19.64	1.00	6.99	42.49	
V _{cd50}	Approach velocity required to initiate scour of the D ₅₀ at the pier	0.46	0.16	0.16	0.28	0.16	0.42	0.14	ft / s
V _{cd95}	Approach velocity required to initiate scour of the D ₉₅ at the pier	2.62	0.72	0.42	0.62	0.59	0.75	0.37	ft / s
V _{ic50}	Critical velocity for incipient motion of the D ₅₀	1.19	0.49	0.49	0.78	0.49	1.12	0.43	ft / s
V _{ic95}	Critical velocity for incipient motion of the D ₉₅	5.37	1.81	1.13	1.54	1.50	1.87	1.01	ft / s

K _v	Correction factor for very wide piers	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
K _u	Units correction factor for calculating V _{cd50}	11.17	11.17	11.17	11.17	11.17	11.17	11.17	
y _s	Single Column Pier Scour	2.1	8.9	12.1	13.3	13.2	14.4	11.7	ft
y _{sm1}	Multiple Column Pier Scour - Raudkivi Method	2.5	10.7	14.5	15.9	15.8	17.3	N/A	
y _{sm2}	Multiple Column Pier Scour - Equivalent Pier Method	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
y _{sm3}	Multiple Column Pier Scour - K ₂ Method	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
y _{sm}	Multiple Column Pier Scour	2.5	10.7	14.5	15.9	15.8	17.3	N/A	ft

EQUATIONS

$$F_{r1} = \frac{V_1}{(\sqrt{y_1 g})}$$

$$K_2 = (\cos \theta + \left(\frac{L_p}{a}\right) \sin \theta)^{0.65}$$

where $(L_p/a)_{max} = 12$

$$K_4 = 0.4(V_R)^{0.15} \text{ where } V_R > 0$$

$$V_R = \frac{V_1 - V_{cd50}}{V_{cd95} - V_{cd50}} > 0$$

$$V_{cd50} = K_u y_1^{1/6} D_{50}^{1/3}$$

$$V_{ic50} = 0.645 \left(\frac{D_{50}}{a_p}\right)^{0.55} V_{cd50}$$

$$K_w = 2.58 \left(\frac{y_1}{a_p}\right)^{0.34} (F_{r1})^{0.65} \text{ for } \frac{V_1}{V_{cd50}} < 1$$

$$K_w = 2 \left(\frac{y_1}{a}\right)^{0.13} (F_{r1})^{0.25} \text{ for } \frac{V_1}{V_{cd50}} \geq 1$$

$$y_s = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_p}{y_1}\right)^{0.65} F_{r1}^{0.43}$$

Raudkivi Method: If $s > 5d$,
 $y_{sm1} = 1.2 y_s$

Equivalent Pier Method: If $s < 5d$,
 $y_{sm2} = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_m}{y_1}\right)^{0.65} F_{r1}^{0.43}$
 where $K_2 = 1$ and $a_m = a_p C \sin \theta + a_p \cos \theta$

K₂ Method: If $s < 5d$,
 $y_{sm3} = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_p}{y_1}\right)^{0.65} F_{r1}^{0.43}$
 where $K_2 = (\cos \theta + C \sin \theta)^{0.65}$

TABLES

Table 6.1. Correction (K₁) for Pier Nose Shape

Pier Nose Shape	K ₁
Square	1.1
Round	1.0
Cylindrical	1.0
Group	1.0
Sharp	0.9

Table 6.3. Increase in Equilibrium Pier Scour Depths (K₃) for Bed Condition

Bed Condition	Height (m)	K ₃
Clear-Water	N/A	1.1
Plane Bed	N/A	1.1
Small Dunes	3 > H >= 0.6	1.1
Medium Dunes	θ > H >= 3	1.2
Large Dunes	H >= 9	1.3

100-YR CONTRACTION SCOUR

US Enter SI for Metric or US for English Units

Fill in highlighted cells

Determine if Live-Bed or Clear-Water Contraction Scour is occurring.
 Critical Velocity (V_c) is the velocity of the approach flow that will begin to move the bed material.
 If $V_c < V$, then Live - Bed Contraction Scour is occurring.
 If Live - Bed Contraction Scour is not occurring, use Clear-Water Contraction Scour equation.

Critical Velocity -

	LOB	CHAN	ROB		
$V_1 =$	1.89	6.30	1.29	V_1 - Velocity of the flow at the upstream approach, (m/s, ft/s)	Approach Section
$y_1 =$	9.27	66.88	9.03	y_1 - Average depth of flow upstream of the bridge, (m, ft)	Approach Section
$D_{50} =$	0.00003	0.00230	0.00079	D_{50} - Particle size in a mixture of which 50 percent are smaller, (m, ft)	soil borings
$K_U =$	11.17	11.17	11.17	K_U - (Metric Units = 6.19, English Units = 11.17)	
$D_c =$	0.0016	0.0219	0.0005	D_c - Critical Diameter for the flow that you are using, (m, ft)	Calculated
$V_c =$	0.48	2.97	1.49	V_c - Critical velocity above which bed material of size D_c and smaller will be transported (m/s, ft/s)	Calculated

$V_c > V_1$

Live	Live	Clear
------	------	-------

Source:

Sta. 595

Live-Bed Contraction Scour -

	LOB	CHAN	ROB		
$y_1 =$	9.27	66.88	9.03	y_1 - Average depth in the upstream main channel, (m, ft)	Approach Section
$y_0 =$	14.71	60.85		y_0 - Existing depth in the contracted section before scour, (m, ft)	Section BR U
$Q_1 =$	35822.20	775530.30		Q_1 - Flow in the upstream channel transporting sediment, (m ³ /s, ft ³ /s)	Approach Section
$Q_2 =$	54614.55	755438.4		Q_2 - Flow in the contracted channel, (m ³ /s, ft ³ /s)	Section BR U
$W_1 =$	2047.9	1840		W_1 - Bottom (or top) width of the upstream main channel, (m, ft)	Approach Section
$W_2 =$	1699.88	2005.93		W_2 - Bottom (or top) width of main channel in the contracted section less the pier width(s), (m, ft)	Section BR U
$D_{50} =$	0.007925	0.70104	0.240792	D_{50} - Median diameter of the bed material, (mm)	soil borings
$\omega =$	0.0010	0.1000	0.0350	ω - Median fall velocity of the bed material based on the D_{50} , (m/s, ft/s)	HEC-18 Figure 5.8
$S_1 =$	0.000068	0.000068		S_1 - Slope of energy grade line of main channel, (m/m, ft/ft)	Approach Section
$g =$	32.2	32.2	32.2	g - Acceleration of gravity (Metric = 9.81m/s ² , English = 32.2ft/s ²)	
$V^* =$	0.142	0.383	0.000	$V^* = (r/\rho)^{1/2} = (g y_1 S_1)^{1/2}$ shear velocity in the upstream section, (m/s, ft/s)	Calculated
$V/\omega =$	142.47	3.83	0.00		Calculated
$k_1 =$	0.69	0.69	0.59	k_1 - Exponents determined on the mode of bed material transport	HEC-18 Table
$y_2 =$	15.13	61.61	0.00	$y_2 = y_1 [(Q_2/Q_1)^{6/7} (W_1/W_2)^{1/3}]$	

$y_s =$

0.42	0.76	N/A
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Feet $y_s = y_2 - y_0 =$ Scour Depth Below Bed

Clear-Water Contraction Scour -

	LOB	CHAN	ROB		
$Q =$			1947.04	Q - Discharge through the bridge or on the set-back overbank area at the bridge associated with W	Section BR U
$D_{50} =$			0.00079	D_{50} - Median diameter of the bed material, (m, ft) (1m=1000mm, 1ft=304.8mm)	soil borings
$W =$			71.69	W - Bottom (or top) width of the contracted section less pier width, (m, ft)	Section BR U
$y_0 =$			12.04	y_0 - Average existing depth in the contracted section, (m, ft)	Section BR U
$D_m =$	0	0	0.000988	D_m - Diameter of the smallest nontransportable particle in the bed material ($1.25D_{50}$), contracted section (m,ft)	Calculated
$K_U =$	0.0077	0.0077	0.0077	K_U - (Metric = 0.025, English = 0.0077)	Constant
$y_2 =$	0.00	0.00	15.21	$y_2 = [K_U Q^2 D_m^{2/3} W_2^{-1/3}]^{3/7}$	

$y_s =$

N/A	N/A	3.17
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Feet $y_s = y_2 - y_0 =$ Scour Depth Below Bed

TOTAL 100 YEAR SCOUR COMPUTATIONS

	Pier						
	1	2	3	4	5	6	7
Boring Number	B54	B61	B64	B66	B70	B77	B79
Modeled Bed Elevations***	430.5	427.9	432.3	434.3	437.6	434	430.2
Bed Rock Elevation***	330.5	331.4	333.8	333.3	334.3	335.8	334.7
Competent Rock Elevation***	329.7	331.4	331.1	333.1	334.3	335.5	334.5
Long-term (ft)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Contraction (ft)	0.4	0.4	0.4	0.4	0.4	0.4	0.4
Local (ft)	2.5	10.7	14.5	15.9	15.8	17.3	11.7
Total Scour (ft)	3.0	11.1	14.9	16.3	16.3	17.7	12.1
Total Scour Elevation	427.5	416.8	417.4	418.0	421.3	416.3	418.1

*** from geotechnical data

TOTAL 100 YEAR SCOUR INPUTS (From FESWMS)

Pier #	1	2	3	4	5	6	7
Node #	4605	4113	4116	4120	4617	4626	5412
Velocity (m/s)	0.1	0.3	0.7	1.1	0.8	0.9	0.9
Velocity (ft/s)	0.4	1.1	2.1	3.6	2.7	3.0	3.0
Surface Elevation (m)	138.4	138.4	138.4	138.4	138.4	138.4	138.4
Surface Elevation (ft)	454.1	454.1	454.1	454.1	454.1	454.1	454.1
Depth (m)	2.4	7.5	4.9	2.0	4.1	6.1	6.4
Depth (ft)	8.0	24.7	16.0	6.6	13.5	20.0	21.0

500 YEAR SIMPLE PIER SCOUR COMPUTATIONS

A **simple** pier is a single shaft, column, or multiple columns exposed to the flow. The pier itself is socketed into rock, and no footing is needed.

Project: Louisville Southern Indiana Ohio River East End Bridge
Configuration: Final
Date: 9/20/2011
Prepared by: LMW Chkd by: PXVG
Methodology: HEC-18, 4th Edition, Section 6.1
Scour Case: Simple
Return Period (yrs): 500
Flow Type: Steady
No. of Piers: 7
Units: English

BED CONDITIONS

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
D ₅₀	Diameter at which 50% of the soil grains pass	4.27E-04	1.74E-05	2.13E-05	1.35E-04	2.30E-05	2.26E-04	1.28E-05	ft
D ₈₅	Diameter at which 95% of the soil grains pass	3.94E-02	8.53E-04	2.59E-04	1.02E-03	6.56E-04	1.05E-03	1.61E-04	ft
BC	Bed Condition (clear-water, plane bed, small dunes, medium dunes, large dunes)	small dunes	small dunes	small dunes	small dunes	small dunes	small dunes	small dunes	

FLOW DATA

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
θ	Angle of attack	22.0	23.0	28.0	28.0	30.0	29.0	25.0	degrees
y ₁	Flow depth directly upstream of the pier	13.2	29.9	21.1	11.7	18.6	25.2	26.2	ft
V ₁	Mean velocity of flow directly upstream of the pier	1.38	1.97	3.02	4.37	3.48	3.72	3.73	ft / s
g	Acceleration of gravity	32.2	32.2	32.2	32.2	32.2	32.2	32.2	ft / s ²

PIER DATA

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
PS	Pier shape (Square, Round, Cylindrical, Group, Sharp)	Group	Group	Group	Group	Group	Group	Group	Group
a _p	Pier stem width	8.0	8.0	8.0	8.0	8.0	8.0	8.0	ft
L _p	Length of pier stem	16.0	16.0	16.0	16.0	16.0	16.0	16.0	ft
C	Number of columns per bent	2.0	2.0	2.0	2.0	2.0	2.0	3.0	
S	Column spacing	80.0	80.0	80.0	80.0	80.0	80.0	40.0	

SIMPLE PIER SCOUR CALCULATIONS

Variable	Description	Pier 1	Pier 2	Pier 3	Pier 4	Pier 5	Pier 6	Pier 7	Unit
F _{r1}	Froude number directly upstream of the pier	0.07	0.06	0.12	0.22	0.14	0.13	0.13	
K ₁	Correction factor for pier nose shape from Table 6.1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
K ₂	Correction factor for angle of attack of flow	1.40	1.41	1.48	1.48	1.50	1.49	1.20	
K ₃	Correction factor for bed condition from Table 6.3	1.10	1.10	1.10	1.10	1.10	1.10	1.10	
K ₄	Correction factor for armoring by bed material size	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
V _s	Velocity ratio required to calculate K ₄	1.00	1.00	38.90	22.08	1.00	8.54	51.49	
V _{cd50}	Approach velocity required to initiate scour of the D ₅₀ at the pier	0.49	0.16	0.17	0.31	0.17	0.43	0.14	ft / s
V _{cd85}	Approach velocity required to initiate scour of the D ₈₅ at the pier	2.84	0.74	0.44	0.68	0.62	0.78	0.38	ft / s
V _{ic50}	Critical velocity for incipient motion of the D ₅₀	1.29	0.51	0.52	0.86	0.52	1.17	0.45	ft / s
V _{ic85}	Critical velocity for incipient motion of the D ₈₅	5.84	1.87	1.18	1.69	1.58	1.94	1.05	ft / s

K _v	Correction factor for very wide piers	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
K _u	Units correction factor for calculating V _{cd50}	11.17	11.17	11.17	11.17	11.17	11.17	11.17	
y _s	Single Column Pier Scour	9.2	12.1	14.5	15.6	15.3	16.3	13.3	ft
y _{sm1}	Multiple Column Pier Scour - Raudkivi Method	11.0	14.5	17.4	18.8	18.4	19.6	N/A	
y _{sm2}	Multiple Column Pier Scour - Equivalent Pier Method	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
y _{sm3}	Multiple Column Pier Scour - K ₂ Method	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
y _{sm}	Multiple Column Pier Scour	11.0	14.5	17.4	18.8	18.4	19.6	N/A	ft

EQUATIONS

$$F_{r1} = \frac{V_1}{(\sqrt{y_1 g})}$$

$$K_2 = (\cos \theta + \left(\frac{L_p}{a_p}\right) \sin \theta)^{0.65}$$

$$K_4 = 0.4(V_R)^{0.15} \text{ where } V_R > 0$$

$$V_R = \frac{V_1 - V_{icD_{50}}}{V_{icD_{85}} - V_{icD_{50}}} > 0$$

$$V_{cd_{50}} = K_u y_1^{1/6} D_{50}^{1/3}$$

$$V_{icD_{50}} = 0.645 \left(\frac{D_{50}}{a_p}\right)^{0.55} V_{cd_{50}}$$

$$K_w = 2.58 \left(\frac{y_1}{a_p}\right)^{0.34} (F_{r1})^{0.65} \text{ for } \frac{V_1}{V_{cd_{50}}} < 1$$

$$K_w = 2 \left(\frac{y_1}{a_p}\right)^{0.13} (F_{r1})^{0.25} \text{ for } \frac{V_1}{V_{cd_{50}}} \geq 1$$

$$y_s = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_p}{y_1}\right)^{0.65} F_{r1}^{0.43}$$

Raudkivi Method: If $s > 5d$,
 $y_{sm1} = 1.2 y_s$

Equivalent Pier Method: If $s < 5d$,
 $y_{sm2} = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_m}{y_1}\right)^{0.65} F_{r1}^{0.43}$
 where $K_2 = 1$ and $a_m = a_p C \sin \theta + a_p \cos \theta$

K₂ Method: If $s < 5d$,
 $y_{sm3} = 2 y_1 K_1 K_2 K_3 K_4 K_w \left(\frac{a_p}{y_1}\right)^{0.65} F_{r1}^{0.43}$
 where $K_2 = (\cos \theta + C \sin \theta)^{0.65}$

TABLES

Table 6.1. Correction (K₁) for Pier Nose Shape

Pier Nose Shape	K ₁
Square	1.1
Round	1.0
Cylindrical	1.0
Group	1.0
Sharp	0.9

Table 6.3. Increase in Equilibrium Pier Scour Depths (K₃) for Bed Condition

Bed Condition	Height (m)	K ₃
Clear-Water	N/A	1.1
Plane Bed	N/A	1.1
Small Dunes	3 > H >= 0.6	1.1
Medium Dunes	θ > H >= 3	1.2
Large Dunes	H >= 9	1.3

500-YR CONTRACTION SCOUR

US Enter SI for Metric or US for English Units

Fill in highlighted cells

Determine if Live-Bed or Clear-Water Contraction Scour is occurring.
 Critical Velocity (V_c) is the velocity of the approach flow that will begin to move the bed material.
 If $V_c < V$, then Live - Bed Contraction Scour is occurring.
 If Live - Bed Contraction Scour is not occurring, use Clear-Water Contraction Scour equation.

Critical Velocity -

	LOB	CHAN	ROB		
V_1	2.13	6.69	1.54	V_1 - Velocity of the flow at the upstream approach, (m/s, ft/s)	Approach Section
y_1	14.41	72.11	11.64	y_1 - Average depth of flow upstream of the bridge, (m, ft)	Approach Section
D_{50}	0.00003	0.00230	0.00079	D_{50} - Particle size in a mixture of which 50 percent are smaller, (m, ft)	soil borings
K_U	11.17	11.17	11.17	K_U - (Metric Units = 6.19, English Units = 11.17)	Calculated
D_c	0.0018	0.0253	0.0008	D_c - Critical Diameter for the flow that you are using, (m, ft)	Calculated
V_c	0.52	3.01	1.55	V_c - Critical velocity above which bed material of size D_c and smaller will be transported (m/s, ft/s)	Calculated

$V_c > V_1$

Live	Live	Clear
------	------	-------

Source:

Sta. 595

Live-Bed Contraction Scour -

	LOB	CHAN	ROB		
y_1	14.41	72.11	11.64	y_1 - Average depth in the upstream main channel, (m, ft)	Approach Section
y_0	19.84	66.07		y_0 - Existing depth in the contracted section before scour, (m, ft)	Section BR U
Q_1	63387.94	887324.20		Q_1 - Flow in the upstream channel transporting sediment, (m ³ /s, ft ³ /s)	Approach Section
Q_2	87910.19	860702.3		Q_2 - Flow in the contracted channel, (m ³ /s, ft ³ /s)	Section BR U
W_1	2064.56	1840		W_1 - Bottom (or top) width of the upstream main channel, (m, ft)	Approach Section
W_2	1710.62	2006.5		W_2 - Bottom (or top) width of main channel in the contracted section less the pier width(s), (m, ft)	Section BR U
ω	0.007925	0.70104	0.240792	ω - Median fall velocity of the bed material based on the D_{50} , (m/s, ft/s)	soil borings
S_0	0.0010	0.1000	0.0350	S_0 - Median fall velocity of the bed material based on the D_{50} , (m/s, ft/s)	HEC-18 Figure 5.8
S_1	0.00007	0.00007		S_1 - Slope of energy grade line of main channel, (m/m, ft/ft)	Approach Section
g	32.2	32.2	32.2	g - Acceleration of gravity (Metric = 9.81m/s ² , English = 32.2ft/s ²)	
V^*	0.180	0.403	0.000	$V^* = (\tau_c/\rho)^{1/2} = (g y_1 S_1)^{1/2}$ shear velocity in the upstream section, (m/s, ft/s)	Calculated
V/ω	180.22	4.03	0.00		Calculated
k_1	0.69	0.69	0.59	k_1 - Exponents determined on the mode of bed material transport	HEC-18 Table
y_2	21.71	66.18	0.00	$y_2 = y_1 \{ (Q_2/Q_1)^{0.7} (W_1/W_2)^{0.1} \}$	

$y_s = y_2 - y_0 =$ Scour Depth Below Bed

1.87	0.11	N/A
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Clear-Water Contraction Scour -

	LOB	CHAN	ROB		
Q			3387.47	Q - Discharge through the bridge or on the set-back overbank area at the bridge associated with W	Section BR U
D_{50}			0.00079	D_{50} - Median diameter of the bed material, (m, ft) (1m=1000mm, 1ft=304.8mm)	soil borings
W			80.56	W - Bottom (or top) width of the contracted section less pier width, (m, ft)	Section BR U
y_0			15.66	y_0 - Average existing depth in the contracted section, (m, ft)	Section BR U
D_m	0	0	0.000988	D_m - Diameter of the smallest nontransportable particle in the bed material (1.25 D_{50}), contracted section (m,ft)	Calculated
K_U	0.0077	0.0077	0.0077	K_U - (Metric = 0.025, English = 0.0077)	Constant
y_2	0.00	0.00	22.12	$y_2 = [K_U Q^2 D_m^{2/3} W_2^{-1/3}]^{3/7}$	

$y_s =$

N/A	N/A	6.46
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$y_s = y_2 - y_0 =$ Scour Depth Below Bed

TOTAL 500 YEAR SCOUR COMPUTATIONS

	Pier						
	1	2	3	4	5	6	7
Boring Number	B54	B61	B64	B66	B70	B77	B79
Modeled Bed Elevations***	430.5	427.9	432.3	434.3	437.6	434	430.2
Bed Rock Elevation***	330.5	331.4	333.8	333.3	334.3	335.8	334.7
Competent Rock Elevation***	329.7	331.4	331.1	333.1	334.3	335.5	334.5
Long-term (ft)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Contraction (ft)	1.9	1.9	1.9	1.9	1.9	1.9	1.9
Local (ft)	11.0	14.5	17.4	18.8	18.4	19.6	13.3
Total Scour (ft)	12.9	16.3	19.2	20.6	20.3	21.4	15.1
Total Scour Elevation	417.6	411.6	413.1	413.7	417.3	412.6	415.1

*** from geotechnical data

TOTAL 500 YEAR SCOUR INPUTS (From FESWMS)

Pier #	1	2	3	4	5	6	7
Node #	4605	4113	4116	4120	4617	4626	5412
Velocity (m/s)	0.4	0.6	0.9	1.3	1.1	1.1	1.1
Velocity (ft/s)	1.4	2.0	3.0	4.4	3.5	3.7	3.7
Surface Elevation (m)	140.0	140.0	140.0	140.0	140.0	140.0	140.0
Surface Elevation (ft)	459.3	459.3	459.3	459.3	459.3	459.3	459.3
Depth (m)	4.0	9.1	6.4	3.6	5.7	7.7	8.0
Depth (ft)	13.2	29.9	21.1	11.7	18.6	25.2	26.2